



Coconut Road Traffic Study

From Estero Bay to Three Oaks Parkway



Village of Estero, FL
03/25/2016

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Statement of Certification

I certify that this Traffic Study has been prepared by me or under my immediate supervision and that I have experience and training in the field of Traffic and Transportation Engineering.

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Table of Contents

Project Description 4

Existing Roadway Conditions 5

Traffic Volume Data Collection 7

Traffic Corridor Level of Service Analysis..... 9

Coconut Road and US 41 Intersection Analysis 25

Coconut Road – Safety Review 38

Conclusions and Recommendations..... 41

APPENDICES

Appendix A: Lee County DOT Functional Classification 42

Appendix B: FDOT Federal-Aid Report (Excerpts) 45

Appendix C: Lee County Road Maintenance Map 49

Appendix D: Lee County 2015 Concurrency Report (Excerpts) 51

Appendix E: Lee County 2015 Traffic Count Report (Excerpts) 54

Appendix F: TCS Corridor Volume Counts 59

Appendix G: 2014 FDOT Peak Season Factor Category Report (Excerpt)..... 71

Appendix H: 2013 Lee County Link – Specific Volumes (Excerpt) 73

Appendix I: Lee County 2035 LRTP (Excerpts) 75

Appendix J: Potential Development – ITE Period Analysis Reports..... 79

Appendix K: FDOT Traffic Online – T24 Factors 86

Appendix L: Lee County – Intersection Signal Data 88

Appendix M: Intersection Analysis - HCS 2010 Printouts 99

Appendix N: Intersection Alternative – HCS 2010 Analysis 109

Appendix O: Crash Data – West of US 41 112

Appendix P: Crash Data – East of US 41 114

Project Description

Traffic counts on major roadways in Lee County had dropped significantly around 2007-2008 due to the downturn in the economy. With the improved economy, more development activity has been the result. As such, Lee County roadway traffic has been climbing up towards pre-recession levels.

Many roadways in the Village of Estero are faced with increased traffic congestion and delays. The Coconut Road Traffic Study was initiated by the Village of Estero, Department of Community Development, to evaluate the impact of development on this roadway. Coconut Road is an east-west roadway and is located in the southern part of the Village of Estero, within Lee County, Florida (refer to **Fig. 1 – Project Location Map**). Within the Village of Estero the west section of this roadway study runs from the Hyatt Hotel to US 41, a distance of approximately 1.6 miles and the east section of this roadway runs from US 41 to Three Oaks Parkway.

Fig. 1 – Project Location Map

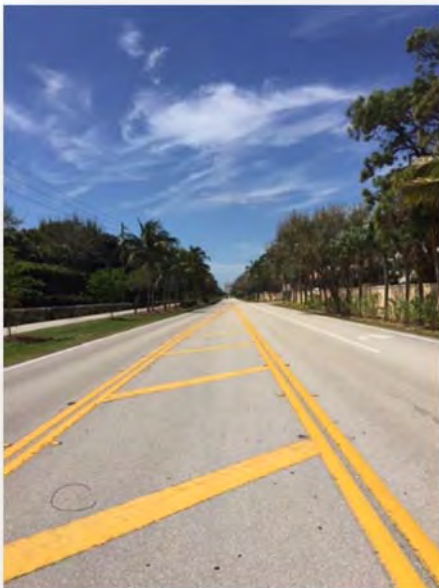


The main objective of this traffic study is to evaluate existing and future traffic conditions and to determine if there are improvement needs for Coconut Road. The study process includes a few steps. The first step involves traffic data collection, to help determine the existing roadway Level of Service (LOS), and estimating future travel demand to evaluate if the existing roadway LOS will be acceptable in the future. The second step includes evaluating corridor improvement needs and potential conceptual alternative improvements, if needed. A safety review of the roadway is provided as well. Finally, conclusions and recommendations are provided.

Existing Roadway Conditions



East-west portion of Coconut Road terminates at the Hyatt Hotel and the road runs north-south for a short length (<500 ft) and then west for another 2,000 ft.



Coconut Road west of US 41 is mainly an undivided 2-lane section of roadway.

Coconut Road is classified as a major collector under the jurisdiction and maintenance of the Lee County Department of Transportation, and it stretches from Estero Bay to the west to just west of Interstate 75 to the east (Pebble Pointe at the Brooks).

Refer to **Appendix A: Lee County DOT Functional Classification**, **Appendix B: FDOT Federal – Aid Road Report (Excerpts)** and **Appendix C: Lee County Road Maintenance Map**.

Coconut Road is an undivided two-lane roadway at its western terminus and has mainly an open drainage system. East of the Hyatt hotel, the roadway has turn lanes at many access points. There is also a sidewalk along the south side of the roadway. The posted speed limit for the east-west section is 40 mph. About 900 ft. west of the intersection with US 41 the roadway changes to curb and gutter with a closed drainage system and sidewalks on both sides of the roadway. East of US 41, the roadway is a four-lane divided curb and gutter facility with a posted speed limit of 45 mph. In the eastern section (from US 41 to Three Oaks Parkway), there are undesignated on-street bicycle lanes on both sides and an asphalt pathway on the south side. Turn lanes are provided as well.

Lee County develops a concurrency report annually that includes an inventory of the maximum utilized and available capacity of public facilities for which minimum Level of Service (LOS) standards are prescribed. The latest report is the October 2015 Concurrency Report. The transportation inventory from the Concurrency Report shows roadway link traffic volumes and its corresponding LOS by “Existing” 2014 100th Highest Hour, a short-term projected “Future” estimated 2015 100th Highest Hour, and Future Forecast Volume.



Coconut Road east of US 41 is a divided 4-lane section of roadway.

Lee County Department of Transportation (Lee DOT) continues to comply with the requirements within the Lee County Comprehensive Land Use Plan (the Lee Plan) by updating the calculations of the maximum service volume for LOS A through LOS E. The maximum service volumes are based on the existing roadway characteristics plus any changes that are part of an improvement that has been programmed for construction in the first three (3) years of the adopted 5-year Lee County Capital Improvement Program (CIP) or the Florida Department of Transportation (FDOT) Work Program.

The existing roadway conditions are extracted from the Lee County October 2015 Concurrency Report and illustrated in **Table 1, Existing Roadways Conditions**.

Table 1
Existing Roadways Conditions

Roadway Link Name	Lee County Link No.	Lee County Roadway Link Location	Exist Roadway ⁽¹⁾	Standard LOS	Standard Volume ⁽²⁾	2014 100 th Highest Hour LOS	2014 100 th Highest Hour Volume ⁽²⁾
Coconut Road	05000	Spring Creek Road to US 41	2LN	E	860	C	366
Coconut Road	05030	US 41 to Three Oaks Parkway	4LD	E	1,790	C	588

Note(s): ⁽¹⁾ 2LN = 2-narrow lanes roadway; 4LD =4-lane divided roadway, respectively;
⁽²⁾ Peak Hour, Peak Season, Peak Direction.

Refer to **Appendix D: Lee County 2015 Concurrency Report (Excerpts)**.

Lee DOT operates traffic count programs on its major roadways to provide traffic characteristics and historical data. Coconut Road traffic count data is provided in **Appendix E: Lee County 2015 Traffic Count Report (Excerpts)**.

Traffic Volume Data Collection

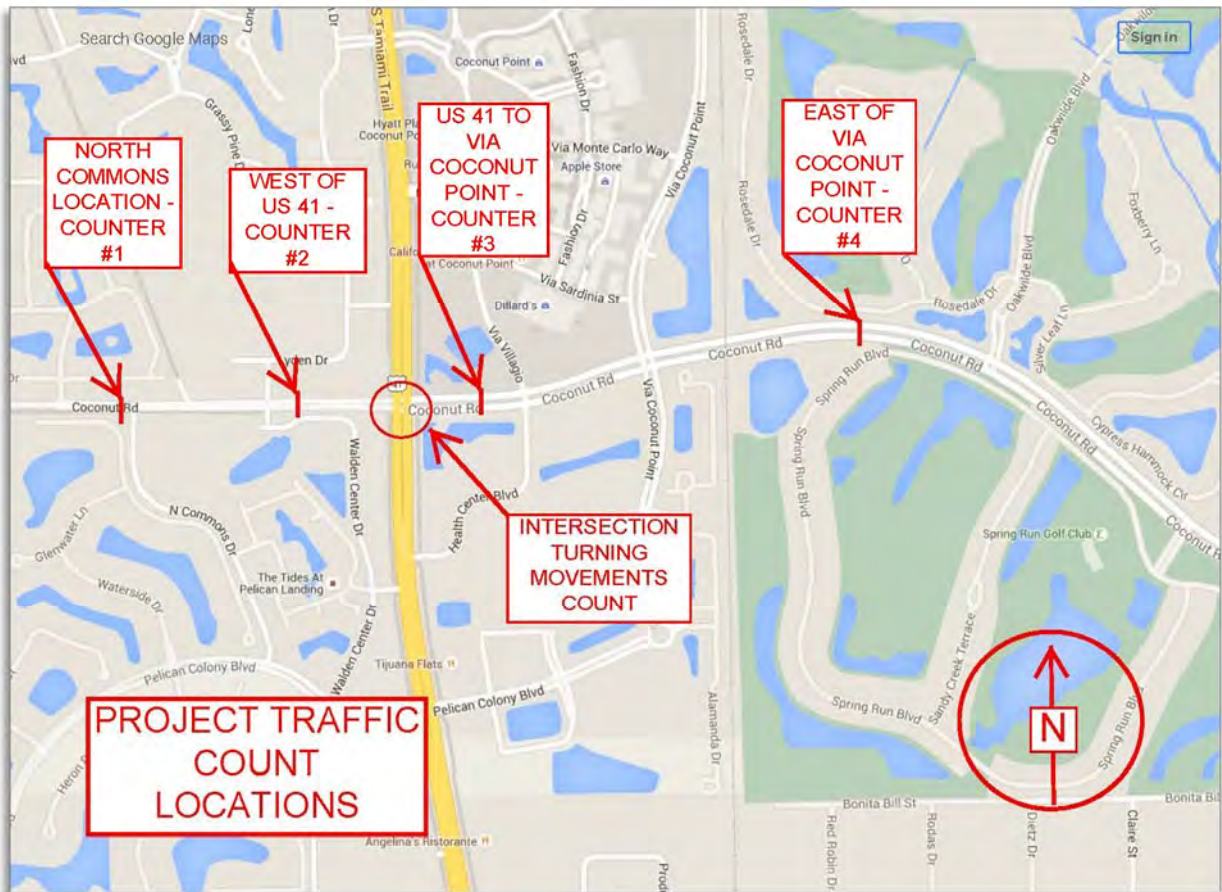
Existing daily and peak hour traffic count data were collected by Trebilcock Consulting Solutions (TCS) for the study corridor. This included traffic counts conducted along the corridor and its intersection with US 41 and count data provided by Lee DOT.

Bi-directional 72-hour machine traffic counts were conducted along the Coconut Road study corridor in February 2016 at four (4) selected locations (refer to **Appendix F: TCS Corridor Volume Counts**).

Selected corridor traffic count locations are illustrated in **Fig. 2** and **Table 2**.

For the purpose of this study, turning movement counts for the intersection of Coconut Road and US 41 were conducted on Wednesday, January 27, 2016, from 7AM to 9AM, and from 4PM to 6PM.

Fig. 2 – Project Traffic Count Locations



**Table 2
TCS Count Locations**

Roadway Link Name	TCS Count Location	TCS Roadway Segment	Lee County Link No.
Coconut Road	1	From Spring Creek Road to US 41	05000
Coconut Road	2	From Spring Creek Road to US 41	05000
Coconut Road	3	From US 41 to Via Coconut Point	05030
Coconut Road	4	From Via Coconut Point to Three Oaks Parkway	05030

Traffic Corridor Level of Service Analysis

Collected daily and peak hour traffic count data was analyzed in accordance with FDOT and Lee DOT procedures.

1. Annual Average Daily Traffic (AADT)

Based on the FDOT procedures outlined in the Project Traffic Forecasting Handbook, the AADT volumes can be determined from short-term traffic count data collected by applying correction factors, such as Weekly Seasonal Correction Factor (SF) and the Axle Correction Factor (AF). The AADT is calculated based on the following formula: $AADT = ADT \times SF \times AF$.

The Average Daily Traffic (ADT) is the total traffic volume during a given time period divided by the number of days in that time period. For the purpose of this report, the 72-hour traffic volume is averaged for Tuesday, Wednesday and Thursday. The calculated 2016 AADT volumes for the field traffic counts are rounded to the nearest hundredths.

Weekly Seasonal Correction Factor (SF) is extracted from the latest published FDOT Peak Season Factor Category Report as shown in **Appendix G: 2014 FDOT Peak Season Factor Category Report (Excerpt)**.

As illustrated in the FDOT Traffic Monitoring Handbook, axle factor categories are more highway-specific than seasonal factor categories. For the purpose of this report, the AADT has not been adjusted with axle correction factor. Projected 2016 AADT volumes are illustrated in **Table 3**.

Table 3
Coconut Road – 2016 AADT

COCONUT ROAD LINK VOLUMES - 2016 AADT						
TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	3 DAY AVERAGE TWO-WAY	FDOT WEEKLY SEASONAL CORRECTION FACTOR (1)	FDOT AXLE CORRECTION FACTOR	2016 AADT TWO-WAY
1	FROM SPRING CREEK RD TO US 41	05000	10,884	0.81	1.0	8,800
2	FROM SPRING CREEK RD TO US 41	05000	11,969	0.82	1.0	9,800
3	FROM US 41 TO VIA COCONUT POINT	05030	15,824	0.82	1.0	13,000
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	17,339	0.82	1.0	14,200

NOTES: (1) 2014 PEAK SEASON FACTOR CATEGORY REPORT - CATEGORY 1252 BONITA SPRINGS AREA

2. Traffic Characteristics and Existing 2016 Level of Service (LOS)

The evaluation of existing and future traffic operating conditions along Coconut Road is determined based on Directional Design Hourly Volumes (DDHV). DDHV are obtained by applying a number of traffic factors such as Design Hour Factor (K) and Directional Factor (D).

Consistent with the information contained within the 2015 Lee County Traffic Count Report and the 2015 Lee County Concurrency Report, the current 2016 peak season, peak hour, peak direction is calculated by factoring the 2016 AADT by the appropriate K-100 and D-100 factors, approximating the 100th highest hour of the year.

The Lee County Traffic Count Report provides traffic characteristics for roadway stations located on all major roadways in Lee County. Monthly, daily and hourly factors are available for all permanent count station. Traffic characteristics of the permanent count stations are used to factor the periodic count location. A review of 2015 Lee County Traffic Count Report – Permanent Count Station 15 (PCS) – was conducted to determine K-100 factor.

The Directional Distribution, D-100 factor is the percentage of the total, two-way design hour traffic (the 100th highest hour of the year) traveling in the peak direction. The D-100 factor is used in Lee County in calculating the level of service for a roadway.

A “D” factor associated with a specific roadway link is derived using permanent traffic counters located throughout Lee County. As illustrated in the FDOT Traffic Monitoring Handbook, Florida values for “D” range between 50% and 80%.

Site-specific “D” factors are calculated for all four (4) TCS traffic count locations during the AM peak hour of 7-9 AM, and PM peak hour of 4-6 PM. It is noted that based on collected site traffic counts, the eastbound was the peak direction movement in the AM and PM for the Coconut Road link west of US 41. For the Coconut Road segment located east of US 41, it was observed that westbound was the peak direction in the AM, while eastbound was the peak direction in the PM. Site-specific “D” factor values are illustrated in **Table 4**.

Table 4
Coconut Road – D Factor Site Specific

THURS 2-11-2016		COCONUT RD AT NORTH COMMONS				
	PK HR	TOTAL VOLUME	DIRECTIONAL VOLUME		DIRECTIONAL FACTOR	
			EB	WB	D FACTOR	DIRECTION
AM	7:15-8:15	611	320	291	52%	EB
PM	4:00-5:00	845	471	374	56%	EB
THURS 2-4-2016		COCONUT RD WEST OF US 41				
	PK HR	TOTAL VOLUME	DIRECTIONAL VOLUME		DIRECTIONAL FACTOR	
			EB	WB	D FACTOR	DIRECTION
AM	7:30-8:30	788	421	367	53%	EB
PM	4:30-5:30	972	601	371	62%	EB
THURS 2-4-2016		COCONUT RD - US 41 TO VIA COCONUT POINT				
	PK HR	TOTAL VOLUME	DIRECTIONAL VOLUME		DIRECTIONAL FACTOR	
			EB	WB	D FACTOR	DIRECTION
AM	8:00-9:00	900	281	619	69%	WB
PM	4:00-5:00	1,536	793	743	52%	EB
THURS 2-4-2016		COCONUT RD - EAST OF VIA COCONUT POINT				
	PK HR	TOTAL VOLUME	DIRECTIONAL VOLUME		DIRECTIONAL FACTOR	
			EB	WB	D FACTOR	DIRECTION
AM	7:45-8:45	1,075	419	656	61%	WB
PM	4:30-5:30	1,462	815	647	56%	EB

As shown in **Table 4** and consistent with the 2015 Lee County Concurrency Report, the design peak hour utilized in this report is the PM peak hour. To better illustrate corridor specific conditions and account for a design 100th peak hour of the year, the estimated “D” factor utilized in this report is calculated by averaging the site-specific “D” factor and the recommended Lee County PCS 15 “D” factor. A comparison analysis between site-specific “D” factor and the Lee County Traffic Count PSC 15 “D” factor is illustrated in **Table 5**.

**Table 5
Coconut Road – Estimated D-100 Factor**

COCONUT ROAD LINK VOLUMES - 2016 D-FACTOR SITE SPECIFIC									
TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	TIME PERIOD	SITE SPECIFIC D-FACTOR		*LEE COUNTY D-FACTOR		ESTIMATED D-FACTOR	
				VALUE	DIRECTION	VALUE	DIRECTION	VALUE	DIRECTION
1	FROM SPRING CREEK RD TO US 41	05000	AM	0.52	EB	0.53	WB	0.53	EB
			PM	0.56	EB	0.52	EB	0.54	EB
2	FROM SPRING CREEK RD TO US 41	05000	AM	0.53	EB	0.53	WB	0.53	EB
			PM	0.62	EB	0.52	EB	0.57	EB
3	FROM US 41 TO VIA COCONUT POINT	05030	AM	0.69	WB	0.53	WB	0.61	WB
			PM	0.52	EB	0.52	EB	0.52	EB
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	AM	0.61	WB	0.53	WB	0.57	WB
			PM	0.56	EB	0.52	EB	0.54	EB

NOTES: *2015 LEE COUNTY TRAFFIC COUNT REPORT - PCS 15

Existing 2016 operating conditions were determined for roadway segments along Coconut Road as follows: from Spring Creek Road to US 41; from US 41 to Via Coconut Point; and from Via Coconut Point to Three Oaks Parkway. Based on our field observations and a review of the Lee County Traffic Count Report data associated with Coconut Road, it is our recommendation to use TCS counter location #2 (west of US 41) to represent data corresponding to Coconut Road segment from Spring Creek to US 41.

In agreement with the Lee DOT level of service calculations, daily volumes are converted to peak hour, peak season, peak directional volumes by applying K-100 factor, the estimated D-100 factor and Lee County PCS 15 day of the week fraction. The estimated Directional Design Hour Volume (DDHV) is illustrated In **Table 6**.

**Table 6
Coconut Road – Estimated Directional Design Hour Volume**

COCONUT ROAD LINK VOLUMES - 2016 PEAK HOUR PEAK DIRECTION PEAK SEASON							
TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	2016 AADT TWO-WAY	*K-100	ESTIMATED D-100	* DAY OF WEEK FRACTION	2016 PEAK HOUR PEAK DIRECTION PEAK SEASON
2	FROM SPRING CREEK RD TO US 41	05000	9,800	0.100	0.57	1.07	522
3	FROM US 41 TO VIA COCONUT POINT	05030	13,000	0.100	0.52	1.07	632
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	14,200	0.100	0.54	1.07	717

NOTES: * 2015 LEE COUNTY TRAFFIC COUNT REPORT - PCS 15

The estimated 2016 DDHV is then compared to the directional capacities obtained from 2015 Lee County Concurrency Report and the Lee County Generalized Peak-Hour Directional Service Values. The LOS is reflected in **Table 7**.

**Table 7
Coconut Road – Estimated 2016 LOS**

COCONUT ROAD LINK VOLUMES - CAPACITY ANALYSIS										
TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	2016 PK HR, PK DIR VOLUME	ROAD TYPE (1)	PERFORMANCE STANDARD (1)		2013 LINK SPECIFIC SERVICE VOLUMES, PK HR, PK DIR LOS (2)			PROJECTED 2016 LOS
					LOS	CAPACITY	C	D	E	
2	FROM SPRING CREEK RD TO US 41	05000	522	2LN	E	860	560	860	860	C
3	FROM US 41 TO VIA COCONUT POINT	05030	632	4LD	E	1,790	1,310	1,790	1,790	C
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	717	4LD	E	1,790	1,310	1,790	1,790	C
NOTES: (1) 2015 LEE COUNTY CONCURRENCY REPORT (2) REFER TO 2013 LEE COUNTY LINK SPECIFIC SERVICE VOLUMES										

Consistent with operation performance standard of LOS E for Coconut Road roadway segments as illustrated in the 2015 Lee County Concurrency Report, no level of service deficiencies were identified for existing 2016 peak hour, peak season, peak direction background traffic conditions.

Lee County service volumes for peak hour and peak direction are provided in **Appendix H: 2013 Lee County Link – Specific Service Volumes (Excerpt)**.

3. Future Area Growth

The Coconut Road region has been experiencing significant growth, not only in population but also in economic activities.

The historical traffic data was obtained from the 2015 Lee County Traffic Count Report (excerpts are included in **Appendix E**) and is summarized in **Table 8**.

**Table 8
Coconut Road – Historical Traffic Data**

ROADWAY SEGMENT	FROM	TO	NUMBER OF LANES	AADT*										GROWTH RATE
				2006	2007	2008	2009	2010	2011	2012	2013	2014	2015	
Coconut Road	Spring Creek Road	US 41	2 LN	6000	9300	N/A	N/A	N/A	7800	N/A	7600	N/A	9200	4.85%
	US 41	Three Oaks Pkwy	4 LD	15100	15500	12600	9900	10700	9900	N/A	12200	N/A	12200	-2.35%
US 41	South of Hickory Dr		6 LD	43300	41300	41200	40200	38600	42000	N/A	36600	37700	42500	-0.20%

Note(s): *Refer to 2015 Lee Traffic Count Report

A review of the historical traffic data indicates that the annual growth rate is 4.85% for Coconut Road west of US 41, and it is negative for the US 41 and Coconut Road segment located east of US 41. As coordinated with the Village of Estero Transportation staff, a minimum growth rate of one percent (1%) was utilized for the Coconut Road segment east of US 41.

The exponential growth formula was implemented to calculate estimated annual growth rate as follows: $F = P \times (1+r)^n$ where – F = future volume; P = base year traffic volume; r = growth rate percentage; n = number of years from the base year.

4. Coconut Road Corridor Development Potential

Several government documents were reviewed in an effort to determine the consistency with future transportation demand and develop strategies for potential solutions along the corridor. As part of this report, the following documents were reviewed: Lee County Transportation Improvement Program, Fiscal Year 2015/2016 to Fiscal Year 2019/2020 (as adopted September 15, 2015); 2015 Lee County Concurrency Report; and the Lee County Metropolitan Planning Organization (MPO) 2035 Long-Range Transportation Plan (LRTP).

The 2035 LRTP is a long-range plan for transportation needs developed by the Lee County MPO. The 2035 LRTP recommends major transportation projects, systems, strategies, and policies in order to maintain and improve the current transportation system with the objective of meeting future travel demand.

Based on the review of these documents, there are no major improvements depicted in the 2035 LRTP Needs Plan. As such, the existing Coconut Road corridor conditions are consistent with the adopted 2035 LRTP.

The latest Lee County Draft Highway Needs Plan (roadway improvements for Needs Plan) and Lee County Existing and Committed Network are provided in **Appendix I: Lee County 2035 LRTP (Excerpts)**.

Although the economic down-turn since 2007-2008 has decelerated the pace of planned projects from this area, several developments approved by either Lee County staff or the City of Bonita Springs Planning staff are still moving forward. The result of this localized growth and implementation of the approved developments are impacting roadway conditions. In addition to the zoning approved developments, there are undeveloped parcels that may be considered for future construction permitting.

For the purpose of this report, zoning approved developments are expected to be completed within the next five (5) years, while the potential future permitting is expected to develop within the next ten (10) or more years.

The projected Coconut Road development potential is summarized in **Table 9** and **Fig. 3 – Coconut Road Development Potential Map**.

Table 9
Coconut Road – Estimated Development Potential

ID #	STRAP #	PARCEL NAME	DEVELOPMENT PARAMETERS	ZONING APPROVALS NOT YET CONSTRUCTED	POTENTIAL FUTURE CONSTRUCTION PERMITTING
1	07-47-25-B2-00004.00CE	Pelican Landing Community Association	Marina – 20 berths; Restaurant – 3,000sf		X
2A	07-47-25-B2-00003.0370	Estero Bay Marine LLC (Weeks Fish Camp)	Residential – Multi Family – 360 dwelling units – high-rise buildings		X
2B	07-47-25-B2-00003.0000				
3	07-47-25-B2-00000.0010	WCI/Pelican Landing DRI (Raptor Bay)	Residential – Multi Family – 360 dwelling units – high-rise buildings		X
4	06-47-25-00-00002.0030	Pelican Landing Timeshare Ventures LP	Timeshare – Rental Townhouses – 267 dwelling units	X	
5	17-47-25-B1-00001.012A	WCI Communities Inc (Altaira High Rise)	High-Rise Residential Condominium/ Townhouse – 76 dwelling units	X	
6	17-47-25-B1-U1681.1891	WCI Communities Inc (Two Future High Rises)	Residential – Multi Family – 150 dwelling units – high-rise buildings	X	
7	08-47-25-01 +	Eldorado Acres Subdivision	Residential – Single Family – 98 dwelling units – platted lots	X	
8	08-47-25-00-00003.0030	John T. Watson	Residential – Single Family – 2 dwelling units	X	
9	08-47-25-01-00016.0000	Judy K. Doyle	Residential – Single Family – 15 dwelling units	X	
10	08-47-25-00-00004.0000	Dhaliwal + J/T	Residential – Single Family – 3 dwelling units	X	
11	08-47-25-E2-U1757.2005	Dhaliwal + J/T		X	
12	09-47-25-E1-U1823.2024	Dewane/Docese of Venice	Residential – Multi Family – 93 dwelling units		X
13	09-47-25-E4-U1882.1994	Coconut Road Associates LLC	General Office Building – 122,484sf	X	
14	09-47-25-E1-U1874.2023	HG Coconut LLC	Shopping Center – 210,000sf Medical Office Building – 40,000sf Multi-Family Residential – 525 dwelling units Assisted Living – 152 beds Hotel – 130 Rooms	X	
15A	09-47-25-E1-U1877.2039	OBE Florida CRE Holdings LLC			
15B	09-47-25-E2-U1900.2033				
15C	09-47-25-E2-U1902.2012				
16	09-47-25-E3-31000.0050	Allsee Investment LP	General Office Building – 27,500sf	X	
17	09-47-25-E3-31000.0010	Naples Diagnostic Imaging	General Office Building – 15,000sf	X	
22A	09-47-25-E3-373A1.0000	Lee Memorial Health System	Acute Care Hospital – 160 beds Shopping Center – 60,000sf Medical Office Building – 198,000sf General Office Building – 102,000sf	X	
22B	09-47-25-E3-373A2.0000				
22C	09-47-25-E3-373A3.0010				

Fig. 3 – Coconut Road Development Potential Map



The trip generation for the potential development influencing Coconut Road was determined by referencing the Institute of Transportation Engineer's (ITE) report, titled Trip Generation Manual, the most current edition (9th Edition). The software program OTISS (Online Traffic Impact Study Software, Version 4.0.1) is used to create the raw unadjusted trip generation for the project. The ITE – OTISS trip generation – period analysis reports are provided in **Appendix J: Potential Development – ITE Period Analysis Reports**. A summary of the estimated trip generation for potential development along Coconut Road is summarized in **Tables 10A, 10B and 10C**.

Table 10A
Trip Generation (Developments with Zoning Approvals – Not Yet Constructed)

Developments		PM Peak Hour		
ID #	Parcel Name	Enter	Exit	Total
4	WCI/Pelican Landing Timeshare Ventures LP	98	94	192
5	WCI Communities Inc. (Altaira High Rise)	18	11	29
6	WCI Communities Inc. (Two Future High Rise)	56	28	84
7	Eldorado Acres Subdivision	21	60	81
8	John T. Watson	1	1	2
9	Judy K. Doyle	12	7	19
10/11	Dhaliwal + J/T	2	1	3
13	Coconut Road Associates LLC	37	179	216
14/15*	HG Coconut LLC/OBE Florida CRE Holdings LLC	627	642	1,269
16	Allsee Investment LP	19	90	109
17	Naples Diagnostic Imaging	16	79	95
22**	Lee Memorial Health System	205	409	614
Total Net External		1,112	1,601	2,713

Note(s): *Per approved Coconut Crossing DCI2014-00019 TIS dated October 16, 2014.

**Maximum allowed external trips per February 9, 2016 memorandum from the Village of Estero.
Directional distribution assumed consistent with greatest traffic generator, LUC 720.

Table 10B
Trip Generation (Developments with Potential Future Construction Permitting)

Developments		PM Peak Hour		
ID #	Parcel Name	Enter	Exit	Total
1	Pelican Landing Community Association	20	14	34
2	Estero Bay Marine LLC (Weeks Fish Camp)	115	57	172
3	WCI/Pelican Landing DRI	115	57	172
12	Dewane/Diocese of Venice	38	19	57
Total Net External		288	147	435

Table 10C
Trip Generation (Total Future Projected Development Potential)

Developments	PM Peak Hour		
	Enter	Exit	Total
Zoning Approvals – Not Yet Constructed	1,112	1,601	2,713
Potential Future Construction Permitting	288	147	435
Total Net External	1,400	1,748	3,148

Projected traffic generated by the future potential development is assigned to the Coconut Road segments using the knowledge of the area, associated approved traffic impact statements, and as coordinated with Village of Estero Transportation Planning staff.

The assignment of proposed trip distribution is illustrated consistent with the peak hour, peak direction as shown in **Tables 11A, 11B and 11C**.

Table 11A
Traffic Distribution* - Developments with Zoning Approvals – Not Yet Constructed

ID #	Parcel Name	Traffic Dist. %	Spring Creek to US 41		Traffic Dist. %	US 41 to Via Coconut Point		Traffic Dist. %	Via Coconut Point to Three Oaks Parkway	
			Enter	Exit		Enter	Exit		Enter	Exit
4	WCI/Pelican Landing Timeshare Ventures LP	100%	WB-98	<u>EB-94</u>	25%	WB-25	<u>EB-24</u>	20%	WB-20	<u>EB-19</u>
5	WCI Communities Inc. (Altaira High Rise)	100%	WB-18	<u>EB-11</u>	20%	WB-4	<u>EB-2</u>	15%	WB-3	<u>EB-2</u>
6	WCI Communities Inc. (Two Future High Rise)	100%	WB-56	<u>EB-28</u>	20%	WB-11	<u>EB-6</u>	15%	WB-8	<u>EB-4</u>
7	Eldorado Acres Subdivision	100%	WB-21	<u>EB-60</u>	20%	WB-4	<u>EB-12</u>	15%	WB-3	<u>EB-9</u>
8	John T. Watson	100%	WB-1	<u>EB-1</u>	20%	WB-0	<u>EB-0</u>	15%	WB-0	<u>EB-0</u>
9	Judy K. Doyle	100%	WB-12	<u>EB-7</u>	20%	WB-2	<u>EB-1</u>	15%	WB-2	<u>EB-1</u>
10/ 11	Dhaliwal + J/T	100%	WB-2	<u>EB-1</u>	20%	WB-0	<u>EB-0</u>	15%	WB-0	<u>EB-0</u>
13	Coconut Road Associates LLC	75%	WB-28	<u>EB-134</u>	20%	WB-7	<u>EB-36</u>	15%	WB-6	<u>EB-27</u>
14/ 15	HG Coconut LLC/OBE Florida CRE Holdings LLC	20%	WB-125	<u>EB-128</u>	30%	WB-188	<u>EB-193</u>	25%	WB-157	<u>EB-161</u>
16	Allsee Investment LP	100%	WB-19	<u>EB-90</u>	20%	WB-4	<u>EB-18</u>	15%	WB-3	<u>EB-14</u>
17	Naples Diagnostic Imaging	100%	WB-16	<u>EB-79</u>	20%	WB-3	<u>EB-16</u>	15%	WB-2	<u>EB-12</u>
22	Lee Memorial Health System	20%	<u>EB-41</u>	WB-82	25%	<u>EB-51</u>	WB-102	20%	<u>EB-41</u>	WB-82
Peak Direction Total			<u>EB – 674</u>			<u>EB – 359</u>			<u>EB – 290</u>	

Note(s): *Peak hour, peak direction traffic volumes are **Bold and Underlined** as applicable.

Table 11B
Traffic Distribution* - Developments with Potential Future Construction Permitting

ID #	Parcel Name	Traffic Dist. %	Spring Creek to US 41		Traffic Dist. %	US 41 to Via Coconut Point		Traffic Dist. %	Via Coconut Point to Three Oaks Parkway	
			Enter	Exit		Enter	Exit		Enter	Exit
1	Pelican Landing Community Association	100%	WB-20	<u>EB-14</u>	20%	WB-4	<u>EB-3</u>	15%	WB-3	<u>EB-2</u>
2	Estero Bay Marine LLC (Weeks Fish Camp)	100%	WB-115	<u>EB-57</u>	25%	WB-29	<u>EB-14</u>	20%	WB-23	<u>EB-11</u>
3	WCI/Pelican Landing DRI	100%	WB-115	<u>EB-57</u>	25%	WB-29	<u>EB-14</u>	20%	WB-23	<u>EB-11</u>
12	Dewane/Dioocese of Venice	100%	WB-38	<u>EB-19</u>	20%	WB-8	<u>EB-4</u>	15%	WB-8	<u>EB-3</u>
Peak Direction Total			<u>EB – 147</u>			<u>EB – 35</u>			<u>EB – 27</u>	

Note(s): *Peak hour, peak direction traffic volumes are **and Underlined** as applicable.

Table 11C
Traffic Distribution* - Total Future Projected Development Potential

Coconut Road Development Potential	Spring Creek to US 41	US 41 to Via Coconut Point	Via Coconut Point to Three Oaks Parkway
Development Zoning Approvals	EB – 674	EB – 359	EB – 290
Development Potential Future Construction Permitting	EB – 147	EB – 35	EB – 27
Total Peak Hour Traffic Volume	<u>EB – 821</u>	<u>EB – 394</u>	<u>EB – 317</u>

Note(s): *Peak hour, peak direction traffic volumes are **and Underlined** as applicable.

For the purpose of this report, estimated future Coconut Road demand is analyzed under 2021 and 2026 traffic conditions. Future projected background traffic volumes are calculated based on historic growth rates calculated from annual counts illustrated in the Lee County 2015 Traffic Count Report (as previously summarized in **Table 8 – Coconut Road – Historical Traffic Data**).

A review of the historical traffic data indicates that the annual growth rate is negative for the Coconut Road segments located east of US 41. As such, a minimum growth rate of one percent (1%) was utilized for Coconut Road segments east of US 41.

For the purpose of this report, the future traffic growth for the Coconut Road segment located west of US 41 is considered associated with the future projected development

potential. As such, no future growth is applied towards the background traffic for this roadway segment.

The peak hour, peak season, peak direction 2016 100th Highest Hour traffic volume is used as determined in **Table 6 – Coconut Road – Estimated Directional Design Hour Volume** of this report. Future 2021 and 2026 background traffic conditions are illustrated in **Table 12**.

Table 12
Coconut Road – Future Background Traffic

Roadway Link	Roadway Link Location	2016 100 th Highest Hour Volume* (trips/hr)	Projected Traffic Annual Growth Rate** (%/yr)	Growth Factor**	Future 2021 Background Pk Hr, Pk Dir Vol*** (trips/hr)	Future 2026 Background Pk Hr, Pk Dir Vol**** (trips/hr)
Coconut Road	From Spring Creek Rd to US 41	522	0.0%	1.000	522	522
Coconut Road	From US 41 to Via Coconut Point	632	1.0%	1.0510, 1.1046	664	698
Coconut Road	From Via Coconut Point to Three Oaks Parkway	717	1.0%	1.0510, 1.1046	754	792

Note(s): *Refer to **Table 6** of this report.

**1% minimum or historical growth rate; Growth Factor = $(1 + \text{Annual Growth Rate})^P$, P is the number of years from 2016 for that period.

***2021 Projected Volume= 2014 100th Highest Hour Volume x Growth Factor with P = 5.

****2026 Projected Volume= 2014 100th Highest Hour Volume x Growth Factor with P = 10.

5. Coconut Road – Projected Future Level of Service

Future 2021 and 2026 projected traffic capacity and level of service were analyzed for Coconut Road segments. Roadway improvements that are currently under construction or are scheduled to be constructed as depicted within Lee County Transportation Improvement Program, Fiscal Year 2015/2016 to Fiscal Year 2019/2020 (as adopted September 15, 2015), are considered to be committed improvements for the purpose of this study. As no such improvements were identified, the evaluated roadway segments are anticipated to remain as such thru project build out.

The development potential with zoning approvals but not yet constructed is considered within the projected future 2021 traffic conditions. The overall future development potential (to include potential future construction permitting) is included in the future 2026 traffic conditions analysis.

The estimated future 2021 and 2026 peak season, peak hour, peak direction traffic volumes were compared to the to the Level of Service thresholds volumes obtained from 2015 Lee County Concurrency Report and the Lee County Generalized Peak-Hour Directional Service Values. The future projected LOS for Coconut Road segments are reflected in **Table 13** and **Table 14**.

Table 13
Coconut Road – Estimated Future 2021 LOS

COCONUT ROAD LINK VOLUMES - CAPACITY ANALYSIS											
TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	2021 BACKGROUND TRAFFIC (1)	2021 ESTIMATED DEVELOPED TRAFFIC (2)	2021 TOTAL ESTIMATED TRAFFIC	PERFORMANCE STANDARD (3)		2013 LINK SPECIFIC SERVICE VOLUMES, PK HR, PK DIR LOS (4)			PROJECTED 2021 LOS
						LOS	CAPACITY	C	D	E	
2	FROM SPRING CREEK RD TO US 41	05000	522	674	1,196	E	860	580	910	910	F
3	FROM US 41 TO VIA COCONUT POINT	05030	664	359	1,023	E	1,790	1,310	1,790	1,790	C
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	754	290	1,044	E	1,790	1,310	1,790	1,790	C
NOTES: (1) SEE TABLE 12 OF THIS REPORT (2) SEE TABLE 11 OF THIS REPORT (3) 2015 LEE COUNTY CONCURRENCY REPORT (4) REFER TO 2013 LEE COUNTY LINK SPECIFIC SERVICE VOLUMES											

Table 14
Coconut Road – Estimated Future 2026 LOS

COCONUT ROAD LINK VOLUMES - CAPACITY ANALYSIS											
TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	2026 BACKGROUND TRAFFIC (1)	2026 ESTIMATED DEVELOPED TRAFFIC (2)	2026 TOTAL ESTIMATED TRAFFIC	PERFORMANCE STANDARD (3)		2013 LINK SPECIFIC SERVICE VOLUMES, PK HR, PK DIR LOS (4)			PROJECTED 2021 LOS
						LOS	CAPACITY	C	D	E	
2	FROM SPRING CREEK RD TO US 41	05000	522	821	1,343	E	860	580	910	910	F
3	FROM US 41 TO VIA COCONUT POINT	05030	698	394	1,092	E	1,790	1,310	1,790	1,790	C
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	792	317	1,109	E	1,790	1,310	1,790	1,790	C
NOTES: (1) SEE TABLE 12 OF THIS REPORT (2) SEE TABLE 11 OF THIS REPORT (3) 2015 LEE COUNTY CONCURRENCY REPORT (4) REFER TO 2013 LEE COUNTY LINK SPECIFIC SERVICE VOLUMES											

Based upon the results illustrated in the level of service analysis, the Coconut Road segment located west of US 41 is anticipated to be overcapacity in the year 2021 and 2026 future conditions. All other analyzed segments are projected to operate within the adopted level of service standard at future conditions.

- **Coconut Road – Segment west of US 41 – No build Alternative**

This option would maintain the existing two-lane geometry for the entire section of Coconut Road, west of US 41. As more developments may be approved for construction as shown within this report, significant congestion and delays will increase with generated traffic along this roadway segment.

- **Coconut Road – Segment west of US 41 – Four-Lane Alternative**



Existing two lane portion of Coconut Rd west of US 41 is ROW constrained in many areas as depicted.

A four-lane divided Coconut Road segment west of US 41 would provide sufficient capacity to allow for future area development. However, due to Right-of-way (ROW) constraints and given that this improvement has not been planned/programmed another alternative may be more cost effective. **Coconut Road – Segment west of US 41 – Two-Lane Enhanced Alternative**

Two-lane geometry with enhancements or safety-related improvements may provide sufficient capacity, and result in a

safer roadway with more efficient traffic operations. These

improvements may include mainly intersection improvements, e.g., roundabouts and/or signal optimization timings. Excluding ROW costs reconstructing this 1.58 mile section of roadway to a 4-lane urban section would be in the \$6.6 million dollar range as compared to an estimated \$2.6 million dollars to create a series of roundabouts with significantly less ROW impact anticipated (note costs are conceptual).

Over the past several years, roundabouts have become more popular for intersection solutions as they offer several advantages over other traffic controls, they may cost less to install, can accommodate a series of U-turns and left-turn lanes and reduce delay. They may have lower operation and maintenance costs as compared to signal alternatives. Roundabouts can improve safety by simplifying conflicts, reducing vehicle speeds and providing a clearer indication of the driver’s right-of way compared to other forms of channelization. They also provide an opportunity to improve aesthetics of an intersection with landscaping in connection with community enhancement projects. Roundabouts are particularly suited at intersections on local roads where it is not desirable to give priority to either road or where overall traffic calming is desired, or needed.

It is our recommendation that a series of roundabouts along this segment should be considered for the roadway west of US 41.



Coconut Road and US 41 Intersection Analysis

1. Existing 2016 Conditions

The intersection of US 41 and Coconut Road is a major four-legged signalized intersection. For the purpose of this report, US 41 is considered as the Major Street, while Coconut Road is analyzed as the Minor Street.

US 41 (SR 45) is a principal arterial roadway which runs generally north-south and provides connectivity to Naples to the south and Fort Myers to the north. At this location, its typical cross section is a suburban six-lane divided roadway with dedicated bicycle lanes, curbed median and a paved shoulder (not curbed). The posted legal speed limit is 50 mph in the vicinity of the intersection.

The north approach has three (3) through lanes, one (1) right-turn lane and double dedicated left turn lanes. The south approach has three (3) through lanes, one (1) dedicated left-turn lane and one (1) right turn lane.

Sidewalks are provided on both sides of the roadway. There are overhead power lines along the west side of the roadway. Intersection street lighting is provided.

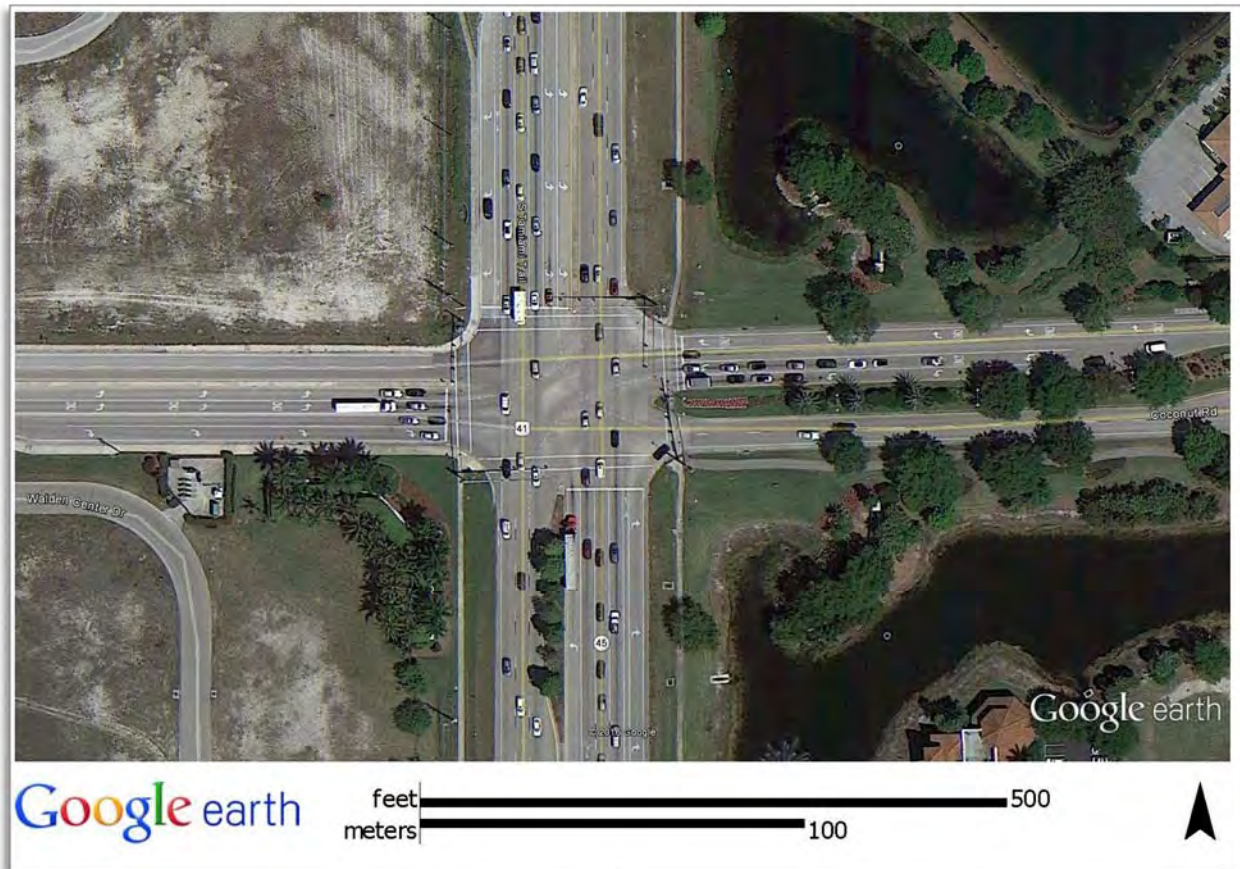
Coconut Road is a major collector roadway. The west approach is a curb and gutter facility with a closed drainage system and no dedicated bicycle lanes. The posted legal speed limit for this approach is 40 mph in the vicinity of the intersection. The east approach typical cross section is a four-lane divided roadway with dedicated bicycle lanes, curb and gutter, and a closed drainage system. The posted legal speed limit for this approach is 45 mph in the vicinity of the intersection.

The west approach has one through lane, one right-turn lane and double dedicated left-turn lanes. The east approach has one through lane, one right-turn lane and double dedicated left-turn lanes.

Sidewalks are provided on both sides of the roadway on the west approach, and on the south side for the east approach. There are overhead power lines along the south side of the roadway on the west approach.

The existing intersection lane configuration is illustrated in **Fig. 4**.

Fig. 4 – Existing Intersection Lane Configuration



2. Intersection Turning Movement Traffic Volumes

For the purpose of this study, turning movement counts for the intersection were conducted on-site on Wednesday, January 27, 2016, from 7AM to 9AM, and from 4PM to 6PM.

Traffic count data is adjusted for peak season by applying a peak season conversion factor to the turning movements. Peak Season Conversion Factor (PSCF) is extracted from the latest published FDOT Peak Season Factor Category Report as shown in **Appendix G: 2014 FDOT Peak Season Factor Category Report (Excerpt)**.

For the purpose of this report, intersection operational analysis was completed following three scenarios: (1) Existing 2016 Conditions; (2) Future 2021 background conditions plus estimated development potential with zoning approvals not yet constructed; and (3) Future 2026 background conditions plus estimated future development potential (to include potential future construction permitting).

In addition, the future traffic growth for the Coconut Road segment located west of US 41 is considered associated with the future projected development potential. As such, no future growth is applied towards the background traffic for this roadway segment.

The historical traffic data was obtained from the 2015 Lee County Traffic Count Report (excerpts are included in **Appendix E**) and it was previously analyzed in this report (refer to **Table 8**). A review of the historical traffic data indicates that the annual growth rate is negative for US 41 and Coconut Road, for the segment located east of US 41. As coordinated with Village of Estero Transportation staff, a minimum growth rate of one percent (1%) was utilized for the Coconut Road segment east of US 41.

The exponential growth formula was implemented to calculate future traffic volumes for the intersection, as follows: $F = P \times (1+r)^n$ where: F = future volume; P = base year traffic volume; r = growth rate percentage; n = number of years from the base year.

A summary of the intersection turning movement count for peak season 2016 existing conditions is illustrated in **Fig. 5A** and **Fig. 5B**.

Fig. 5A – Intersection Count Summary – Existing 2016 Conditions – AM Peak Hour

PROJECT - INTERSECTION EVALUATION REPORT
 INTERSECTION - US 41 AND COCONUT ROAD
 COUNT DATA - DATE - 1-27-2016
 COUNT DATA - TIME - 7.00 AM - 9.00 AM

TRAFFIC COUNTS - 15 MINUTE SUMMARY																			
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL	
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL		
7.00	7.15	13	157	24	194	13	328	31	372	15	14	21	50	60	4	6	70		686
7.15	7.30	11	191	20	222	13	542	31	586	23	15	31	69	68	13	7	88		965
7.30	7.45	16	204	44	264	12	520	40	572	27	23	31	81	104	18	9	131		1,048
7.45	8.00	21	213	53	287	22	535	40	597	33	24	36	93	107	36	12	155		1,132
8.00	8.15	15	233	41	289	18	469	47	534	45	18	35	98	101	25	14	140		1,061
8.15	8.30	22	206	45	273	17	480	65	562	36	26	30	92	103	40	20	163		1,090
8.30	8.45	21	238	45	304	16	474	41	531	44	24	37	105	107	21	12	140		1,080
8.45	9.00	22	215	62	299	31	491	43	565	47	28	45	120	100	29	19	148		1,132

TRAFFIC COUNTS - HOURLY SUMMARY																			
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL	
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL		
7.00	8.00	61	765	141	967	60	1,925	142	2,127	98	76	119	293	339	71	34	444		3,831
7.15	8.15	63	841	158	1,062	65	2,066	158	2,289	128	80	133	341	380	92	42	514		4,206
7.30	8.30	74	856	183	1,113	69	2,004	192	2,265	141	91	132	364	415	119	55	589		4,331
7.45	8.45	79	890	184	1,153	73	1,958	193	2,224	158	92	138	388	418	122	58	598		4,363
8.00	9.00	80	892	193	1,165	82	1,914	196	2,192	172	96	147	415	411	115	65	591		4,363

TRAFFIC COUNTS - PEAK HOUR SUMMARY - UNADJUSTED																					
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL			
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND							
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL				
7.45	8.45	79	890	184	1,153	73	1,958	193	2,224	158	92	138	388	418	122	58	598		4,363		
		PHF				0.95	PHF				0.94	PHF				0.93	PHF				0.92
INTERSECTION PHF										0.97											

TRAFFIC COUNTS - PEAK HOUR SUMMARY - ADJUSTED																				
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL		
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND						
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL			
7.45	8.45	79	890	184	1,153	73	1,958	193	2,224	158	92	138	388	418	122	58	598		4,363	
		PSCF				1.02	1.02				1.02	1.02				1.02	1.02			
		81	908	188	1,176	74	1,997	197	2,268	161	94	141	396	426	124	59	610			

Fig. 5B – Intersection Count Summary – Existing 2016 Conditions – PM Peak Hour

PROJECT - INTERSECTION EVALUATION REPORT
 INTERSECTION - US 41 AND COCONUT ROAD
 COUNT DATA - DATE - 1-27-2016
 COUNT DATA - TIME - 4.00 PM - 6.00 PM

TRAFFIC COUNTS - 15 MINUTE SUMMARY																			
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL	
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL		
4.00	4.15	27	326	110	463	23	350	34	407	54	25	24	103	110	31	23	164	1,137	
4.15	4.30	25	334	113	472	25	342	26	393	54	26	16	96	122	33	14	169	1,130	
4.30	4.45	37	335	107	479	15	304	37	356	52	33	35	120	95	31	19	145	1,100	
4.45	5.00	30	306	131	467	9	298	39	346	56	27	29	112	101	28	17	146	1,071	
5.00	5.15	25	337	128	490	17	325	38	380	96	40	21	157	79	23	14	116	1,143	
5.15	5.30	16	340	138	494	14	285	32	331	51	38	27	116	100	27	18	145	1,086	
5.30	5.45	17	348	104	469	11	278	47	336	51	35	25	111	86	27	8	121	1,037	
5.45	6.00	24	306	95	425	11	274	42	327	41	31	16	88	92	28	18	138	978	

TRAFFIC COUNTS - HOURLY SUMMARY																			
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL	
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL		
4.00	5.00	119	1,301	461	1,881	72	1,294	136	1,502	216	111	104	431	428	123	73	624	4,438	
4.15	5.15	117	1,312	479	1,908	66	1,269	140	1,475	258	126	101	485	397	115	64	576	4,444	
4.30	5.30	108	1,318	504	1,930	55	1,212	146	1,413	255	138	112	505	375	109	68	552	4,400	
4.45	5.45	88	1,331	501	1,920	51	1,186	156	1,393	254	140	102	496	366	105	57	528	4,337	
5.00	6.00	82	1,331	465	1,878	53	1,162	159	1,374	239	144	89	472	357	105	58	520	4,244	

TRAFFIC COUNTS - PEAK HOUR SUMMARY - UNADJUSTED																						
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL				
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND								
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL					
4.15	5.15	117	1,312	479	1,908	66	1,269	140	1,475	258	126	101	485	397	115	64	576	4,444				
		PHF				0.98	PHF				0.94	PHF				0.78	PHF				0.86	
		INTERSECTION PHF																		0.98		

TRAFFIC COUNTS - PEAK HOUR SUMMARY - ADJUSTED																			
TIME		US 41								COCONUT ROAD								INTERSECTION TOTAL	
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND					
FROM	TO	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL		
4.15	5.15	117	1,312	479	1,908	66	1,269	140	1,475	258	126	101	485	397	115	64	576	4,444	
	PSCF	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	1.02	
	ADJUSTED	119	1,338	489	1,946	67	1,294	143	1,505	263	129	103	495	405	117	65	588		

Based on the traffic count data, the AM and PM peak hours for the intersection are determined to be 7.45 – 8.45 AM, and 4.15 – 5.15 PM, respectively.

Peak hour factor (PHF) is the hourly volume during the peak hour divided by the peak 15-min flow rate within the peak hour Highway Capacity Manual 2010 (HCM 2010). As illustrated in the 2014 FDOT Project Traffic Forecasting Handbook, PHF is a measure of traffic demand fluctuation within the analysis design hour. For the purpose of this analysis, PHF is determined to be 0.98 for AM and PM peak hour.

Future background traffic is analyzed based on peak season, peak hour traffic for roadway. As such, future 2021 and 2026 future background conditions are illustrated for PM peak hour (refer to Fig. 6).

Fig. 6 – Turning Movement Summary – 2021 and 2026 Conditions – PM Peak Hour

PM PEAK HOUR FUTURE TRAFFIC																
	US 41								COCONUT ROAD							
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL
TMCs	117	1,312	479	1,908	66	1,269	140	1,475	258	126	101	485	397	115	64	576
PSCF	1.02	1.02	1.02		1.02	1.02	1.02		1.02	1.02	1.02		1.02	1.02	1.02	
2016 PEAK SEASON VOLUME	119	1,338	489	1,946	67	1,294	143	1,504	263	129	103	495	405	117	65	587
ANNUAL GROWTH RATE (AGR)	1.0%	1.0%	1.0%		1.0%	1.0%	1.0%		0.0%	0.0%	0.0%		1.0%	1.0%	1.0%	
YEARS FROM 2016 TO 2021	5	5	5		5	5	5		5	5	5		5	5	5	
2021 BACKGROUND *	126	1,407	514	2,047	71	1,361	151	1,583	263	129	103	495	426	123	69	618
YEARS FROM 2016 TO 2026	10	10	10		10	10	10		10	10	10		10	10	10	
2026 BACKGROUND*	132	1,478	541	2,151	75	1,430	158	1,663	263	129	103	495	448	130	72	650

Note(s): * Future Projected Volume = Base Volume*(1+AGR)ⁿ; n = number of years from the base year.

3. Intersection Analysis – General Description

This is an operational analysis that considers demand volumes, intersection signalization, intersection geometric design and the delay to analyze the quality of operations. The intersection is analyzed for capacity adequacy and level of service provided.

The capacity condition for an intersection is defined by a composite volume/capacity ratio for the critical lane groups for the intersection.

The delay incurred by drivers is used to define the level of service for signalized intersection since it reflects driver's discomfort, frustration, energy consumption and travel time. The level of service is based on the average stopped delay per vehicle in the peak 15 minutes is the criterion used for the traffic movements in the intersection.

Turn lanes are used at intersections to accommodate speed changes and maneuvering of turning traffic, and to increase capacity through an intersection. The length of turn lanes consists of deceleration length (to include entering taper) and storage length. Turn lanes should comply with FDOT Design Standards, Index 301 to the extent practical. The available queue length provided should be based on a traffic study.

The blockage of the turning traffic by the through vehicles should be avoided.

- **Left Turn Lanes**

Left turn lanes are probably the single item having the most influence on intersection operations. Intersection capacity analysis procedures (as consistent with the most current Highway Capacity Manual – HCM) are used to determine the number and use of the left turn lanes.

In agreement with FDOT Plan Preparation Manual (PPM), Volume 1, where left turn volumes exceed 300 vehicles per hour (vph), a double left-turn lane should be considered. Fully protected signal phasing is required for double left turns.

- **Right Turn Lanes**

Exclusive right-turn lanes are less critical in terms of safety than left-turn lanes. As illustrated in FDOT Plan Preparation Manual (PPM), Volume 1, right-turns are generally made more efficient than left-turns. Right-turn storage lanes should be considered when right-turn volume exceeds 300 vph and the adjacent through volume also exceeds 300 vehicles per hour per lane (vphpl).

Right-turn lanes can significantly improve the level of service of signalized intersection by providing means of deceleration and stacking for right turn traffic.

4. Level of Service Analysis

An assessment of the level of service (LOS) of the analyzed intersection was conducted based on existing (current 2016) traffic conditions, future year 2021 (to account for estimated development potential with zoning approvals not yet constructed) and future year 2026 (to include potential future construction permitting).

The intersection was analyzed for Level of Service (LOS) using the Highway Capacity Software 2010 (HCS 2010) computer modeling software, most current version (Release 6.80). The HCS 2010 analyzes signalized intersections by implementing the HCM 2010 procedures.

According to HCM 2010, the level of service criterion for signalized intersections is shown in **Table 15**.

Table 15
Level of Service for Signalized Intersections

HCM-Based Level of Service and Delay Ranges		
Average Delay (seconds / vehicle)		LOS
Signalized Intersections	Unsignalized intersections	
< 10.0	< 10.0	A
> 10.0 to < 20.0	> 10.0 to < 15.0	B
> 20.0 to < 35.0	> 15.0 to < 25.0	C
> 35.0 to < 55.0	> 25.0 to < 35.0	D
> 55.0 to < 80.0	> 35.0 to < 50.0	E
> 80.0	> 50.0	F

Source: HCM 2010

The HCS 2010 percent heavy vehicle is assumed the design hour truck (DHT) – the percent of trucks expected to use the roadway segment during the design hour of the design year. DHT is determined as half of T24 (annual 24-hour percentage of trucks), as illustrated in 2014 FDOT Project Traffic Forecasting Handbook. The T24 factor was extracted from FDOT Florida Traffic Online (T24 = 3.1 for all approaches) and illustrated in **Appendix K: FDOT Traffic Online – T24 Factors**. Conservatively, a 2% heavy vehicle factor was assumed for all movements for the purpose of this analysis.

The existing signal timings/ phasing as obtained from Lee County Department of Transportation was utilized in this analysis. The approved signalization plan, signal pattern plan and signal timing plan are illustrated in **Appendix L: Lee County – Intersection Signal Data**.

The level of service standard for Coconut Road and US 41 is E, as illustrated in the Lee County Concurrency Report. Each approach was analyzed to ensure that vehicles do not experience excessive delay.

The results of the traffic HCS 2010 intersection analysis for Existing 2016 Conditions, Future 2021 background conditions plus estimated development potential with zoning approvals not yet constructed, and Future 2026 background conditions plus estimated future development potential (to include potential future construction permitting) are summarized in **Table 13A**, **Table 13B** and **Table 13C**. Based on the existing 2016 conditions data, the weekday PM peak hour had higher intersection volumes than the weekday AM peak hour.

As such, the PM peak hour characteristics were selected for use in establishing the future background weekday design hour traffic.

Future background traffic with potential development is illustrated in Fig. 7.

Fig. 7 – 2021 and 2026 Background Traffic with Potential Development – PM Peak Hour

PM PEAK HOUR FUTURE TRAFFIC																
	US 41								COCONUT ROAD							
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL
TMCs	117	1,312	479	1,908	66	1,269	140	1,475	258	126	101	485	397	115	64	576
PSCF	1.02	1.02	1.02		1.02	1.02	1.02		1.02	1.02	1.02		1.02	1.02	1.02	
2016 PEAK SEASON VOLUME	119	1,338	489	1,946	67	1,294	143	1,504	263	129	103	495	405	117	65	587
ANNUAL GROWTH RATE (AGR)	1.0%	1.0%	1.0%		1.0%	1.0%	1.0%		0.0%	0.0%	0.0%		1.0%	1.0%	1.0%	
YEARS FROM 2016 TO 2021	5	5	5		5	5	5		5	5	5		5	5	5	
2021 BACKGROUND *	126	1,407	514	2,047	71	1,361	151	1,583	263	129	103	495	426	123	69	618
2021 POTENTIAL DEVELOPMENT	94	104	10	208	85	85	145	315	240	210	135	585	20	185	104	309
FUTURE 2021 TRAFFIC WITH POTENTIAL DEVELOPMENT	240	1,511	524	2,275	156	1,446	296	1,898	503	339	238	1,080	446	308	173	927
YEARS FROM 2016 TO 2026	10	10	10		10	10	10		10	10	10		10	10	10	
2026 BACKGROUND*	132	1,478	541	2,151	75	1,430	158	1,663	263	129	103	495	448	130	72	650
2026 POTENTIAL DEVELOPMENT	169	104	10	283	85	85	268	438	304	246	174	724	20	256	104	380
FUTURE 2026 TRAFFIC WITH POTENTIAL DEVELOPMENT	301	1,582	551	2,434	160	1,515	426	2,101	567	375	277	1,219	468	386	176	1,030

Note(s): * Future Projected Volume = Base Volume*(1+AGR)ⁿ; n = number of years from the base year.

The HCS intersection worksheets and future projected total traffic are provided in **Appendix M: Intersection Analysis – HCS 2010 Printouts**.

Table 13A
Intersection LOS – Existing 2016 Traffic

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS
AM Peak Hour					
Existing 2016	63.7/E	64.8/E	24.2/C	32.8/C	37.7/D
PM Peak Hour					
Existing 2016	85.7/F	85.0/F	24.3/C	22.7/C	38.4/D

NOTE(S) *Approach Delay in s/veh.

Table 13B
Intersection LOS – Future Background Traffic

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS
PM Peak Hour					
Future 2021	85.4/F	85.3/F	25.4/C	24.0/C	39.0/D
PM Peak Hour					
Future 2026	85.4/F	85.8/F	26.6/C	25.5/C	39.9/D

NOTE(S) *Approach Delay in s/veh.

Table 13C
Intersection LOS – Future Background with Potential Development Traffic

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS
PM Peak Hour					
Future 2021	156.9/F	148.4/F	49.6/D	40.2/D	80.3/F
PM Peak Hour					
Future 2026	219.5/F	209.9/F	68.0/E	41.3/D	108.5/F

NOTE(S) *Approach Delay in s/veh.

The intersection HCS 2010 analysis for the existing 2016 conditions shows that eastbound and westbound approaches operate at Level of Service (LOS) F. In addition, the overall intersection background traffic will exhibit an acceptable LOS D for future 2026 conditions.

The intersection HCS 2010 analysis for future 2021 and 2026 conditions with projected potential development shows an overall intersection LOS F. As illustrated in **Appendix M**, queue spillover from turn lanes is present (Queue Storage Ratio exceeds 1.0). Additionally, the extended queue generated from downstream intersections can spill back into the upstream intersection and diminish the performance of the upstream treatment. This phenomenon is frequently observed in large urban areas where the traffic volume is heavy, intersection spacing is short and cycle length is long. As such, intersection treatments such as adding/extending turning lanes, turning movement restrictions that are traditionally applied to improve intersection capacity may not realize the expected benefits of relieving congestion and reducing delay.

5. Volume to Capacity Ratio Analysis

Failure of an intersection is also dependent on the volume over capacity ratio (v/c) being greater than the value of 1 on any approach of the intersection. The outputs provided by HCS (refer to **Appendix M: Intersection Analysis – HCS 2010 Printouts**) include volume over capacity ratios for each approach. The results of the traffic HCS 2010 volume over capacity analysis for Existing 2016 Conditions, Future 2021 background conditions plus estimated development potential with zoning approvals not yet constructed, and Future 2026 background conditions plus estimated future development potential (to include potential future construction permitting) are summarized in **Table 14A**, **Table 14B** and **Table 14C**.

Table 14A
Intersection V/C Ratio – Existing 2016 Traffic

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio
AM Peak Hour				
Existing 2016	LT – 0.753	LT – 0.899	LT – 0.796	LT – 0.436
	TH – 0.484	TH – 0.370	TH – 0.374	TH – 0.837
	RT – 0.840	RT – 0.208	RT – 0.271	RT – 0.265
PM Peak Hour				
Existing 2016	LT – 0.864	LT – 0.902	LT – 0.861	LT – 0.528
	TH – 0.837	TH – 0.620	TH – 0.440	TH – 0.435
	RT – 0.773	RT – 0.406	RT – 0.563	RT – 0.154

Table 14B
Intersection v/c ratio – Future Background Traffic

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio
PM Peak Hour				
Future 2021	LT – 0.862	LT – 0.907	LT – 0.868	LT – 0.556
	TH – 0.836	TH – 0.607	TH – 0.467	TH – 0.465
	RT – 0.773	RT – 0.402	RT – 0.598	RT – 0.166
PM Peak Hour				
Future 2026	LT – 0.862	LT – 0.912	LT – 0.873	LT – 0.584
	TH – 0.836	TH – 0.607	TH – 0.496	TH – 0.497
	RT – 0.773	RT – 0.397	RT – 0.636	RT – 0.177

Table 14C
Intersection v/c ratio – Future Background with Potential Development Traffic

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio
PM Peak Hour				
Future 2021	LT – 1.031	LT – 0.912	LT – 1.156	LT – 0.813
	TH – 1.251	TH – 1.332	TH – 0.588	TH – 0.602
	RT – 1.016	RT – 0.883	RT – 0.714	RT – 0.396
PM Peak Hour				
Future 2026	LT – 1.163	LT – 0.917	LT – 1.449	LT – 0.820
	TH – 1.445	TH – 1.669	TH – 0.616	TH – 0.631
	RT – 1.235	RT – 0.898	RT – 0.752	RT – 0.570

The threshold value of failure for the background traffic volume over capacity ratio (V/C) is not exceeded by any of the scenario years for the intersection.

However, the threshold (V/C) value is exceeded under future 2021 and 2026 traffic conditions when potential development is considered.

6. Intersection Alternatives

Based on the HCS analysis performed for future projected traffic conditions, a significant increase in vehicle delay is expected and one or more of the following conditions are expected to occur: thru vehicle queues extend back and block access to exclusive left-turn and right-turn lanes, left-turn and right-turn vehicle queues extend back and spill over into the adjacent thru lanes, and vehicles require more than one signal cycle to clear the intersection.

Signal retiming and intersection geometric improvements are recommended to promote safety, decreased vehicle delay and sufficient capacity.

To support future general growth in the area and to provide for improved intersection operations a number of intersection improvements are recommended as follows: intersection signal retiming; on Coconut Road – extend eastbound right-turn lane and extend westbound dual left-turn lanes; on US 41 (SR 45) – extend southbound right-turn lane and provide northbound dual left-turn lanes.

Additional HCS 2010 analyses were conducted to illustrate potential benefits that would be achieved with these improvements. The HCS intersection worksheets and future projected total traffic are provided in **Appendix N: Intersection Alternative – HCS 2010 Analysis**.

The HCS 2010 analysis for year 2026 potential future conditions shows that all approaches operate at Level of Service (LOS) E or better. In addition, all movements exhibit a volume over capacity ratio less than 1.0 for future 2026 estimated traffic.

The results of the traffic HCS 2010 intersection analysis for future 2021 and 2026 estimated overall traffic are summarized in **Table 15A** and **Table 15B**.

Table 15A
Intersection LOS – Potential Alternative

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS
PM Peak Hour					
Future 2021	61.6/E	61.3/E	43.2/D	39.4/D	48.0/D
PM Peak Hour					
Future 2026	59.0/E	63.9/E	56.4/E	51.5/D	56.5/E

NOTE(S) *Approach Delay in s/veh.

Table 15B
Intersection v/c ratio – Potential Alternative

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio
PM Peak Hour				
Future 2021	LT – 0.900	LT – 0.892	LT – 0.834	LT – 0.775
	TH – 0.800	TH – 0.921	TH – 0.732	TH – 0.691
	RT – 0.475	RT – 0.461	RT – 0.655	RT – 0.323
PM Peak Hour				
Future 2026	LT – 0.910	LT – 0.896	LT – 0.859	LT – 0.781
	TH – 0.716	TH – 0.937	TH – 0.902	TH – 0.895
	RT – 0.449	RT – 0.397	RT – 0.765	RT – 0.524

Coconut Road – Safety Review

West of US 41

Accident data was supplied by Lee County Transportation staff for the corridor between the west end of Coconut Road, east to Walden Center Drive. The data covers a four year period (12/31/2011 thru 11/13/2015) of accident report information from the Lee County Sheriff's office and Florida Highway Patrol. During this time there were 11 crashes reported. None were fatal. Four of the crashes occurred at night. Weather conditions were clear for 10 of the 11 crashes. Two of the 11 crashes involved possible injuries. None of the accidents involved bikes, motorcycles, pedestrians, intoxication, speeding, loss of control. Five of the crashes involved aggressive driving, two involved distracted driving. One was an angle collision, two were left turn crashes, one was a head on, one involved a heavy truck, one and involved a teen. None of the accidents required an access management review. The table below provides a summary of the crash locations during the reported period. Based on the crash data there does not appear to be any significant crash conditions along the west corridor. Additional crash detail can be found in Appendix O.

EventID	EventCrashDate	EventCrashTime	EventOnStreet	EventCrossStreet	EventNodeDescription
86102035	11/13/2015	14:17:00	NORTH COMMONS DR	COCONUT RD	COCONUT RD @ NORTH COMMONS DR
86101422	10/22/2015	23:00:00	ELDORADO BLVD	COCONUT RD	COCONUT RD @ EL DORADO BLVD
86100668	9/19/2015	7:42:00	COCONUT ROAD	VIA VENETO AT THE COLONY ENTRANCE	COCONUT RD @ VIA VENETO BLVD
85882501	5/17/2015	8:10:00	EL DORADO BLVD	COCONUT RD	COCONUT RD @ EL DORADO BLVD
85615619	2/7/2015	2:45:00	COCONUT RD	SPRING CREEK DR	COCONUT RD @ SPRING CREEK RD
84995514	10/8/2014	23:00:00	COCONUT RD	EL DORADO BLVDVD	COCONUT RD @ EL DORADO BLVD
84794472	7/22/2014	9:00:00	COCONUT RD	SAND FLY CT	COCONUT RD @ VIA VENETO BLVD
81598581	11/9/2013	2:00:00	ELDORADO BLVDVD	COCONUT RD	COCONUT RD @ EL DORADO BLVD
81580877	2/12/2013	6:55:00	COCONUT RD	OLD MEADOWBROOK CIR	COCONUT RD @ OLDE MEADOWBROOK BLVD
81588327	8/15/2012	18:19:00	COCONUT RD	NORTH COMMONS DR	COCONUT RD @ NORTH COMMONS DR
82814081	12/31/2011	1701	COCONUT RD	OLDE MEADOWBROOK CIRCLE	COCONUT RD @ OLDE MEADOWBROOK CIR

East of US 41

Accident data was supplied by Lee County Transportation staff for the corridor between US 41, east to Three Oaks Parkway. The data covers a three year period (1/1/2013 thru 12/30/2015) of accident report information from the Lee County Sheriff's office and Florida Highway Patrol. During this time there were fifty crashes reported, which is nearly six times more than the west section of the corridor for the same period of time. None were fatal. Eight of the crashes occurred at night. Weather conditions were clear for twenty-nine of the fifty crashes. Six of the fifty crashes involved possible injuries. One crash involved a bicycle. Two crashes involved a motorcycle. Fives crashes involved speeding, or driving too fast for conditions. Two crashes involved pedestrians. None of the accidents involved intoxication, or loss of control. Sixteen of the crashes involved aggressive driving, six involved distracted driving. Eleven were angle collisions, ten were left turn crashes, two a head on, three involved a heavy truck, and ten involved a teen. None of the accidents required an access management review. The following table provides a summary of the crash locations during the reported period. Based on the crash data provided, there does not appear to be any particular systemic crash conditions along the east corridor. Additional crash detail can be found in Appendix P. No crash is provided for the intersection of US 41 and Coconut Road, which is a signalized intersection.

Table of Cashes Along Coconut Road East of US 41 to Three Oaks Parkway

EventID	EventCrashDate	EventCrashTime	EventOnStreet	EventCrossStreet	EventNodeDescription
86103219	12/30/2015	15:34:00	COCONUT RD	IMPERIAL PKWY	THREE OAKS PKWY @ COCONUT RD
86102652	12/6/2015	15:47:00	COCONUT RD	HEALTH CENTER BLVD	COCONUT RD @ VIA VILLAGIO
86102546	12/3/2015	13:54:00	COCONUT RD	VIA VILLAGIO PKWY	COCONUT RD @ VIA VILLAGIO
86102441	11/25/2015	9:22:00	COCONUT RD	IMPERIAL PKWY	THREE OAKS PKWY @ COCONUT RD
86101905	11/9/2015	6:31:00	IMPERIAL BLVD	COCONUT DR	THREE OAKS PKWY @ COCONUT RD
85188983	10/31/2015	11:00:00	THREE OAKS PW	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
86101568	10/28/2015	7:40:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
86101368	10/19/2015	17:35:00	COCONUT RD	OAKWILDE BLVD	COCONUT RD @ OAKWILDE BLVD
86100537	9/14/2015	14:47:00	VIA COCONUT POINT	COCONUT RD	COCONUT RD @ VIA COCONUT POINT
86100546	9/14/2015	19:24:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
86100131	8/28/2015	16:30:00	COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
86099780	8/14/2015	7:50:00	THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85883659	7/7/2015	0:45:00	COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
85883099	6/12/2015	12:40:00	COCONUT RD	VIA COCONUT PT	COCONUT RD @ VIA COCONUT POINT
85882790	5/29/2015	20:44:00	COCONUT RD	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
85118465	5/28/2015	14:01:00	THREE OAKS PW	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85882711	5/26/2015	16:47:00	VIA COCONUT PT	COCONUT RD	COCONUT RD @ VIA COCONUT POINT
85882715	5/26/2015	15:00:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85882351	5/11/2015	10:45:00	THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85776473	4/6/2015	19:00:00	COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
85775612	3/10/2015	12:40:00	THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85775332	3/2/2015	9:13:00	SPRING RUN BLVD	COCONUT RD	COCONUT RD @ SPRING RUN BLVD
85615128	1/20/2015	16:15:00	SANDY CREEK TERRACE	SPRINGRUN BLVD	SPRING RUN BLVD @ SANDYCREEK TER
85614965	1/14/2015	17:29:00	THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85614893	1/12/2015	15:25:00	COCONUT RD	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
85614112	12/15/2014	11:25:00	COCONUT DR	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
84996305	11/12/2014	17:24:00	COCONUT RD	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
84996239	11/10/2014	4:01:00	CR 881	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
84996252	11/10/2014	16:15:00	VIA COCONUT PT	COCONUT RD	COCONUT RD @ VIA COCONUT POINT
84995889	10/26/2014	18:50:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
84995546	10/10/2014	0:08:00	COCONUT RD	VIA COCONUT PT	COCONUT RD @ VIA COCONUT POINT
84794116	7/2/2014	12:14:00	THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
84793592	6/3/2014	18:31:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
84152318	3/4/2014	15:58:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
9482355	1/29/2014	13:50:00	COCONUT RD	VIA COCONUT POINT	COCONUT RD @ VIA COCONUT POINT
81585676	12/9/2013	0:34:00	COCONUT RD	VIA COCONUT RD	COCONUT RD @ VIA COCONUT POINT
81588597	10/3/2013	15:27:00	COCONUT RD	VIA COCONUT PT	COCONUT RD @ VIA COCONUT POINT
81586001	9/24/2013	19:24:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
9506440	7/22/2013	8:30:00	CR 881	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
81593383	6/16/2013	15:45:00	COCONUT BLVD		COCONUT RD @ SPRING RUN BLVD
81590299	5/16/2013	8:50:00	IE IMPERIAL PARKWAY IE THREE OAKS	COCONUT ROAD	THREE OAKS PKWY @ COCONUT RD
9499169	4/28/2013	17:40:00	THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
81589663	3/18/2013	14:58:00	COCONUT RD	HEALTH CENTER BLVD	COCONUT RD @ VIA VILLAGIO
83276176	3/14/2013	16:50:00	COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
81591774	3/12/2013	8:11:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
9502305	2/28/2013	18:04:00	8200 HEALTH CENTER BLVD		COCONUT RD @ VIA VILLAGIO
81590065	2/24/2013	14:08:00	COCONUT RD	VIA VILLAGIO WAY	COCONUT RD @ VIA VILLAGIO
81591908	2/13/2013	19:17:00	COCONUT RD	HEALTH CENTER BLVD	COCONUT RD @ VIA VILLAGIO
81586566	1/16/2013	18:19:00	COCONUT RD	VIA COCONUT POINT	COCONUT RD @ VIA COCONUT POINT
9492704	1/1/2013	19:05:00	VIA COCONUT POINT AND COCONUT RD	VIA COCONUT POINT AND COCONUT RD	COCONUT RD @ VIA COCONUT POINT

Conclusions and Recommendations

The main objective of this traffic study is to evaluate engineering data, document information, and to determine if there are improvement needs for Coconut Road future traffic conditions.

Once forecasts were developed, a roadway segment analyses was performed for future 2021 and 2026 conditions. The Level of Service E standard was utilized as a screening method to evaluate whether a roadway segment was deficient for the future condition. Based upon the results illustrated in corridor segment level of service analysis, the Coconut Road segment located west of US 41 is anticipated to be over capacity in the year 2021 and 2026 future conditions. All other analyzed segments are projected to the adopted level of service standard at future conditions.

A significant benefit to overall mobility on Coconut Road west of US 41 is provided by a series of roundabouts along this segment. We would recommend analyzing this option as a way to meet the future buildout along this corridor.

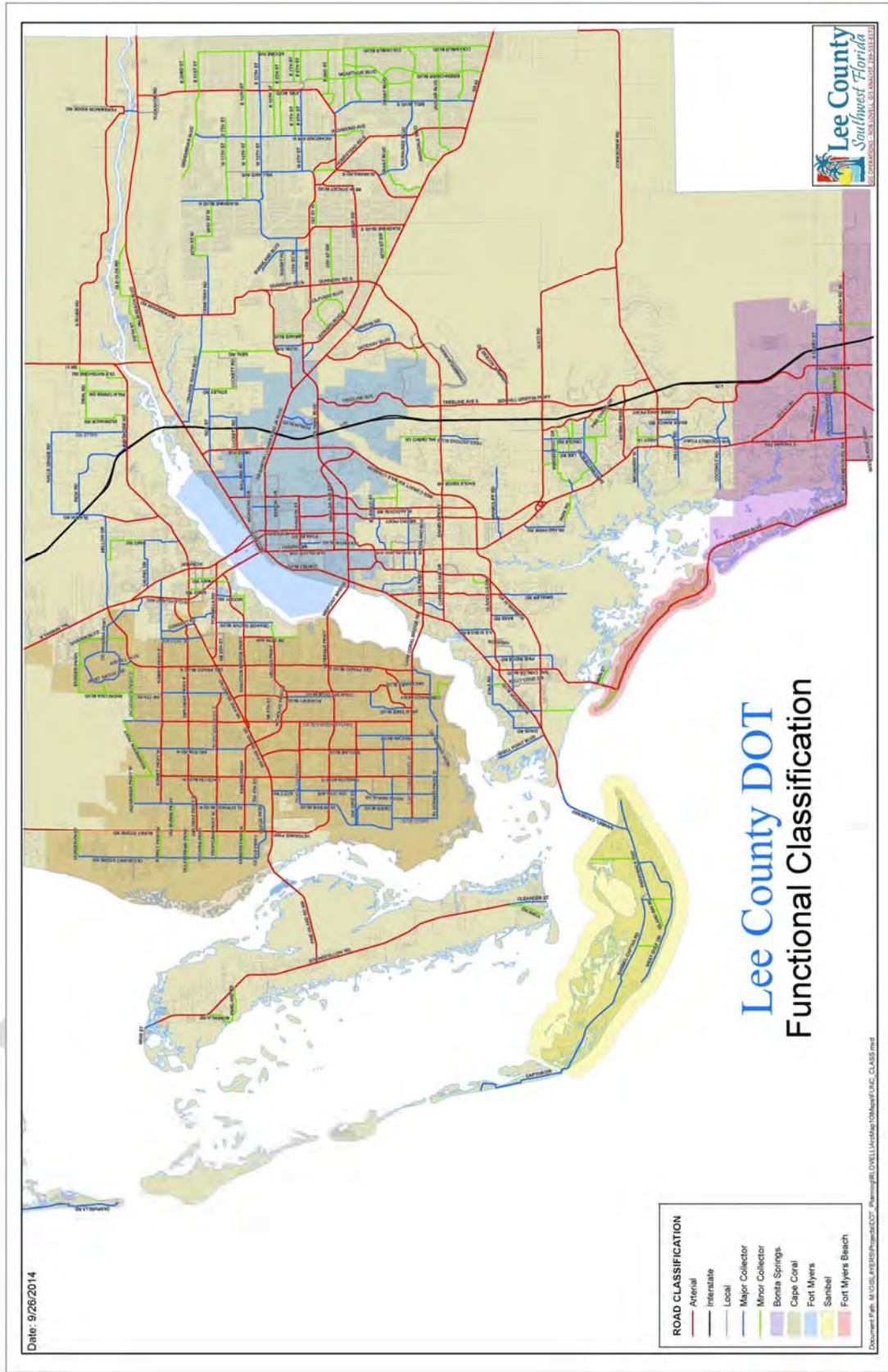
The operational analyses completed for the intersection of Coconut Road and US 41 indicates the need for future intersection improvements to accommodate future volume growth. These improvements include turn lane extensions on Coconut Rd/US 41 and a dual northbound left on US 41.

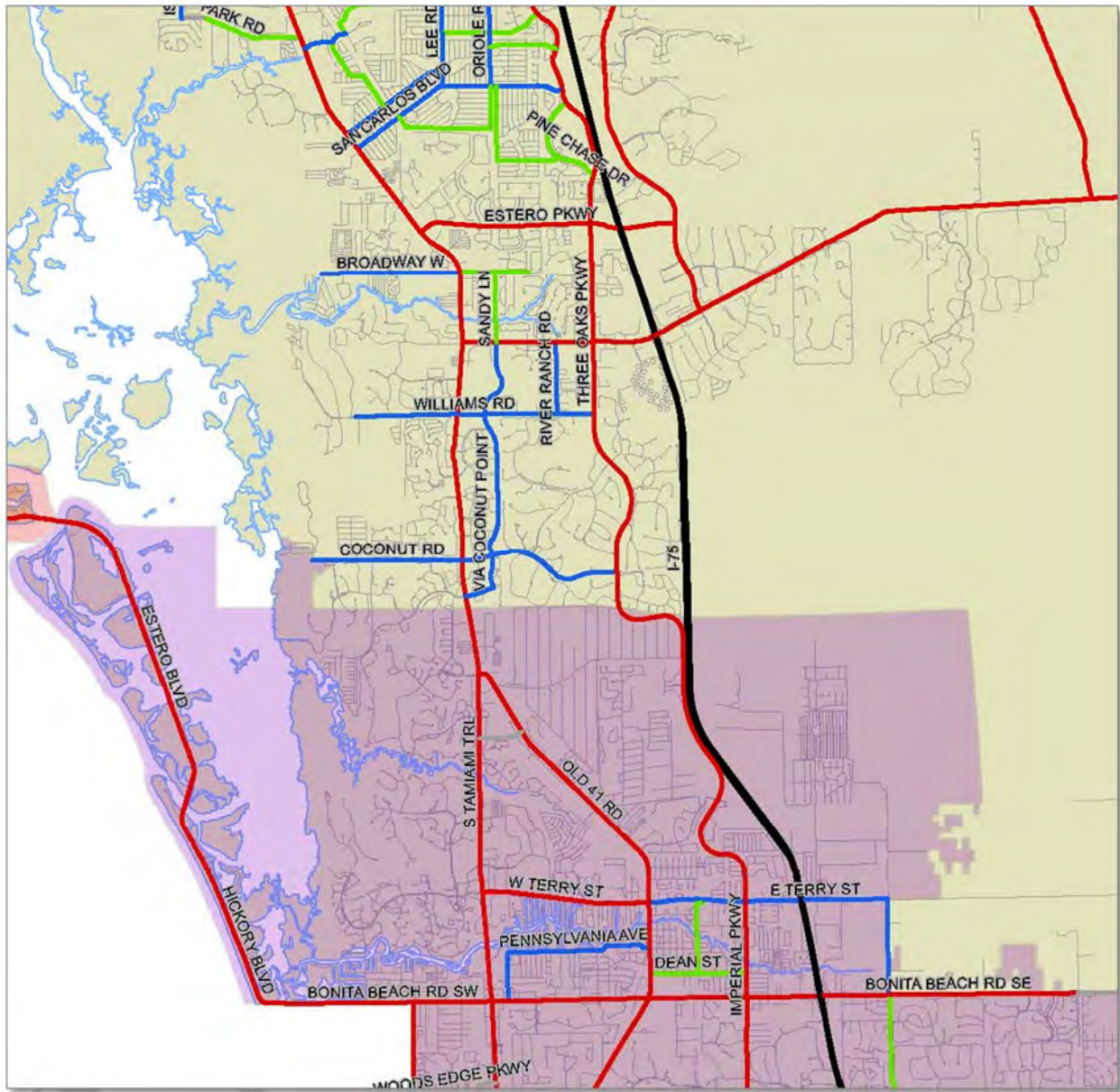
To provide adequate operations through the 2026 future traffic conditions, new intersection lane configuration and signal retiming are recommended to produce lower delays, sufficient capacity and acceptable level of service.

Appendix A: Lee County DOT Functional Classification

(2 Sheets)

DRAFT





Appendix B: FDOT Federal-Aid Report (Excerpts)

(3 Sheets)

DRAFT

Federal-Aid Road Report

This report provides detailed information about all roads on the State Highway System and many additional public roads. It includes the Federal agency responsible for providing federal aid after a disaster, functional classification, local road name, and the name of the road or boundary at each end of each segment identified by FDOT's Roadway ID.

The roads are listed first by county, and then in the order of the Roadway ID. A given Roadway ID may include portions with several different local road names, and a single local road name may include multiple Roadway IDs.

Column	Code	Description
Emergency Mgmt Fund (Emergency Management Fund)	FHWA-ER FEMA	Federal Highway Administration - Emergency Relief Federal Emergency Management Agency
Geographic District	1 thru 7	District Number by County Location
Managing District	1 thru 8	District Number by County Location, and Turnpike ("District 8")
Fed. Fun. Class (Federal Functional Classification)	RPAL RPAO RPAO RMA RMJC RMIC RL UPAI UPAF UPAO UMA UMJC UMIC UL	01 - Rural Principal Arterial - Interstate 02 - Rural Principal Arterial - Other Freeways & Expressways 04 - Rural Principal Arterial - Other 06 - Rural Minor Arterial 07 - Rural Major Collector 08 - Rural Minor Collector 09 - Rural Local 11 - Urban Principal Arterial - Interstate 12 - Urban Principal Arterial - Other Freeways & Expressways 14 - Urban Principal Arterial - Other 16 - Urban Minor Arterial 17 - Urban Major Collector 18 - Urban Minor Collector 19 - Urban Local
Prop. Fun. Class (Proposed Functional Classification)		Codes same as for Fed. Fun Class
Federal System	STP NHS FA-NONE	Surface Transportation Program National Highway System Federal Aid None

Created
Dec. 31, 2015

For More Information

Call Tina Hatcher or Eric Brickner
Local: 850-414-4848
Toll-Free: 800-399-5503

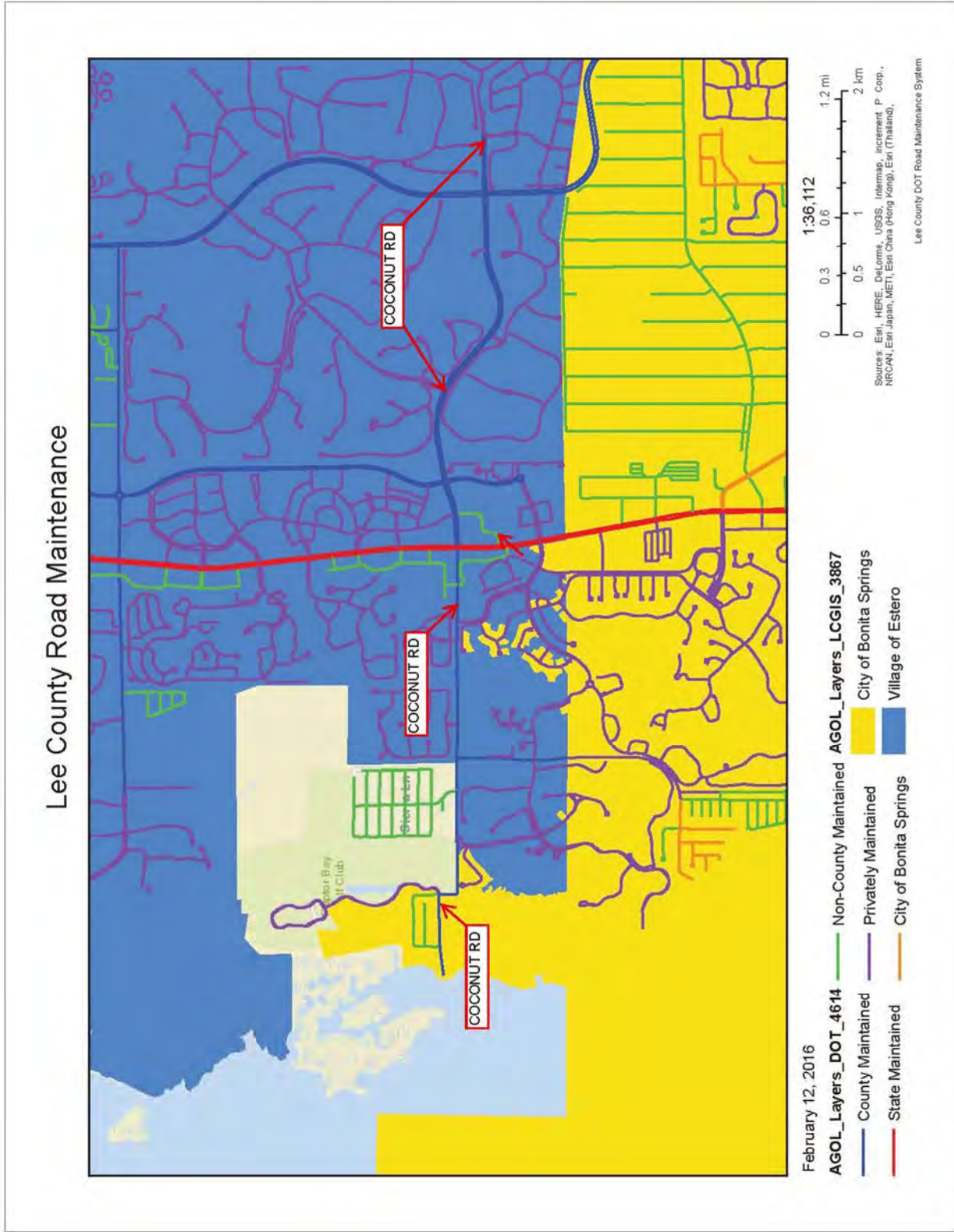
Write Florida Department of Transportation
Transportation Statistics Office
605 Suwannee St, MS 27
Tallahassee, FL 32399-0450

Email transportation@fldot.com
www.fdot.com

Appendix C: Lee County Road Maintenance Map

(1 Sheet)

DRAFT



**Appendix D: Lee County 2015 Concurrency Report
(Excerpts)**
(2 Sheets)

DRAFT

ROADWAY LINK NAME	FROM	TO	ROAD TYPE	PERFORMANCE STANDARD		2014 100th HIGHEST HR		EST 2015 100th HIGHEST HR		FORECAST FUTURE VOL		LINK NO.	NOTES*
				LOS	CAPACITY	LOS	VOLUME	LOS	VOLUME	LOS	VOLUME		
BONITA BEACH RD	IMPERIAL ST	I-75	6LD	E	2,740	C	1,876	C	1,876	C	1,876	02800	
BONITA BEACH RD	I-75	BONITA GRANDE DR	4LD	E	2,000	B	462	B	462	B	462	02900	
BONITA BEACH RD	BONITA GRANDE DR	LOGAN BLVD EXT	4LD	E	2,000	B	462	B	462	B	462	02950	
BOY SCOUT DR	SUMMERLIN RD	US 41	6LD	E	2,550	D	998	D	998	D	998	03200	
BRANTLEY DR*	SUMMERLIN RD	US 41	2LU	E	860	C	116	C	116	C	116	03300	
BRIARCLIFF DR*	US 41	TRIPLE CROWN CT	2LU	E	860	C	203	C	203	C	213	03400	
BROADWAY (ALVA)	PALM BEACH BL (SR 80)	NORTH RIVER RD	2LU	E	860	C	181	C	181	C	181	03500	
BROADWAY (LESTERO)	LOGAN AVE	US 41	2LU	E	860	C	239	C	242	C	262	03600	
BUCKINGHAM RD	IMMOKALEE RD (SR 82)	GUNNERY RD	2LU	E	990	C	350	C	350	C	362	03700	
BUCKINGHAM RD	GUNNERY RD	ORANGE RIVER BL	2LU	E	990	D	529	D	530	D	530	03730	
BUCKINGHAM RD	ORANGE RIVER BL	PALM BEACH BL (SR 80)	2LU	E	990	D	481	D	482	F	1,162	03800	
BURNT STORE RD	PINE ISLAND RD (SR 78)	DIPLOMAT PKWY	2LU	E	1,100	C	664	C	664	D	690	03900	4 Ln construction in FY's 16/17 & 18/19
BURNT STORE RD	DIPLOMAT PKWY	CHARLOTTE COUNTY LINE	2LU	E	1,100	B	356	B	357	C	453	04000	4 Ln construction up to Van Buren in FY 14/15
BUS 41 (SR 739)	FORT MYERS CITY LIMIT	PONDELLA RD	6LD	D	2,740	C	2,051	C	2,051	C	2,148	04200	
BUS 41 (SR 739)	PONDELLA RD	PINE ISLAND RD (SR 78)	6LD	D	2,740	C	1,414	C	1,414	C	1,414	04300	
BUS 41 (SR 739)	PINE ISLAND RD (SR 78)	LITTLETON RD	4LD	D	1,820	B	824	B	824	B	836	04400	
BUS 41 (SR 739)	LITTLETON RD	US 41	2LU	D	1,080	A	380	A	380	A	431	04500	
CAPE CORAL BR RD	DEL PRADO BL	MCGREGOR BL	4L	E	4,000	C	2,616	C	2,616	C	2,616	04600	
CAPTIVA RD*	BLIND PASS	SOUTH SEAS PLANTATION	2LU	E	860	C	267	C	267	C	324	04700	Constrained v/c = 0.31
CEMETERY RD	BUCKINGHAM RD	HIGGINS AVE	2LU	E	860	C	285	C	291	C	291	04800	
CHAMBERLIN PKWY	AIRPORT ENT	DANIELS PKWY	4LD	E	1,790	C	93	C	94	C	94	04900	Port Authority maintained
COCONUT RD*	SPRING CREEK RD	US 41	2LN	E	860	C	366	C	366	C	411	05000	No count since 2007
COCONUT RD*	US 41	THREE OAKS PKWY	4LD	E	1,790	C	588	C	651	C	805	05030	
COLLEGE PKWY*	MCGREGOR BL	WINKLER RD	6LD	E	2,980	D	1,831	C	1,837	C	1,878	05100	
COLLEGE PKWY	WINKLER RD	WHISKEY CREEK DR	6LD	E	2,980	D	1,872	D	1,872	D	1,872	05200	
COLLEGE PKWY*	WHISKEY CREEK DR	SUMMERLIN RD	6LD	E	2,980	D	2,371	C	2,455	C	2,486	05300	
COLLEGE PKWY	SUMMERLIN RD	US 41	6LD	E	2,980	D	1,456	D	1,456	D	1,479	05400	
COLONIAL BL*	MCGREGOR BL	SUMMERLIN RD	6LD	E	2,780	D	2,628	D	2,628	D	2,628	05500	
COLONIAL BL	SUMMERLIN RD	US 41	6LD	E	2,780	F	2,836	F	2,836	F	2,836	05600	Alternatives analysis in FY 18/19
COLONIAL BL (SR 884)	US 41	FOWLER ST	6LD	E	2,780	D	2,535	D	2,535	D	2,535	05700	
COLONIAL BL (SR 884)	FOWLER ST	METRO PKWY	6LD	E	2,780	F	3,125	F	3,125	F	3,125	05800	N. Airport Rd. Ext. current funding for design & construction

ROADWAY LINK NAME	FROM	TO	ROAD TYPE	PERFORMANCE STANDARD		2014 100th HIGHEST HR		EST 2015 100th HIGHEST HR		FORECAST FUTURE VOL		LINK NO.	NOTES*
				LOS	CAPACITY	LOS	VOLUME	LOS	VOLUME	LOS	VOLUME		
DEF													

= Road links within incorporated areas of Fort Myers, Fort Myers Beach, Bonita Springs or Cape Coral

= Road links which do not meet the adopted County or State Level of Service Standard. [Note: Some of these links are constrained.]

* = Road links which no traffic count was performed in 2014, so the 2013 AADT was utilized to develop the link volume.

Note: "Constrained" Roads are as indicated in TABLE 2(a) CONSTRAINED ROADS STATE AND COUNTY ROADS OF "THE LEE PLAN".

Funding is by Lee County unless noted otherwise.

v/c ratio = 2014 100th Highest Hour/Capacity at the Performance Standard.

**Appendix E: Lee County 2015 Traffic Count Report
(Excerpts)**
(4 Sheets)

STREET	LOCATION	Station #	Daily Traffic Volume (AADT)													Area
			2006	2007	2008	2009	2010	2011	2012	2013	2014	2015				
A & W BULB RD	N OF GLADIOLUS DR	215	5800	6100			6400	7700			6800			6600	37	7
ALABAMA RD	N OF IMMOKALEE RD	201	6300	4700	6100	5700	5700							6800	5	5
ALABAMA RD	S OF HOMESTEAD RD	200	9500	9900	8800	9000	9100	8800	11100	9000	9300			10300	6	5
BELL BLVD	N OF IMMOKALEE RD	202	3200	2800	2400	1900									6	5
BELL BLVD	S OF LEELAND HEIGHTS BV	203	8500	9100	7800	7200	7900	7900	9500	8100	8800			9600	6	5
ALICO RD	E OF US 41	204	18900	21200	18100	19500	21400	21800	21700	23400	19900			21900	10	3
ALICO RD	E OF LEE RD	207	22600	22900	20100	19900	22700								10	3
ALICO RD	W OF I - 75	10			28300	26600	26100	25800	27200	29100	38400			41100	3	3
ALICO RD	E OF I - 75	53	11700		12300	20800	25700	26200	26000	26900	28400			25600	3	3
ALICO RD	E OF BEN HILL GRIFFIN PKWAY	205	12600	9400	5800	3800	2800							7500	53	3
ALICO RD	N OF CORKSCREW RD	206	2800	2400	2000	1400	1500								53	3
ARROYAL ST	N OF BONITA BEACH RD	496	6400	5300	4700	4000									42	6
BABCOCK RD	E OF US 41	461	900	700	1400	1300	1200								25	4
BALLARD RD	W OF ORTIZ AV	504	6400	4900	4100	3500	3400								20	3
BARRETT RD	S OF PINE ISLAND RD	509	3100	3200	2600	2300									49	2
BASS RD	N OF SUMMERLIN RD	216	5900	10900	9100	10400	10000	8200	8400					8200	36	7
BAYSHORE RD (SR 78)	E OF BUSINESS 41	218	33400	34000	29500	28900									64	2
BAYSHORE RD (SR 78)	W OF HART RD	104												28600		
BAYSHORE RD (SR 78)	E OF HART RD	219	28500	27700	24800	23500									64	2
BAYSHORE RD (SR 78)	W OF WILLIAMSBURG DR	54			18300	19200	19300	18400	20100	21000	22900					
BAYSHORE RD (SR 78)	E OF NALLE RD	217	13700	11600	8700	10600									64	2
BEN HILL GRIFFIN PKWY	S OF MIDFIELD TERMINAL	60	20900	23700	24000	24100	23500	24000	23700	24800	26300			21100	4	4
BEN HILL GRIFFIN PKWY	S OF ALICO RD	514	20300	26800	21200	15300	18400	18000	16800	26300	29900			22500	60	4
BEN HILL GRIFFIN PKWY	N OF ESTERO PKWY	71								18800	19100			19400		
BEN HILL GRIFFIN PKWY	N OF CORKSCREW RD	517	20500	26200	20300	18800	18000	17300	16200	15100	19500			19600	60	4

BUSINESS 41 (SR 739)	N OF POWELL DR	394	19500	17400	14600	13300						41	2
BUSINESS 41 (SR 739)	N OF LITTLETON RD	76									11500		
BUSINESS 41 (SR 739)	N OF LAUREL DR	396	8300	7700	8500	7300						41	2
CAPE CORAL PKWY	E OF SKYLINE BLVD	13	30100	28200	25900	26800	26200	26700	25000	26400	27700	28800	1
CAPE CORAL PKWY	W OF PALM TREE	56	54000	51000	31900	31800	38500	40800	40100	44800	44100	13	1
CAPE CORAL PKWY	W OF DEL PRADO PKWY											1	1
CAPE CORAL PKWY	W OF CAPE CORAL BR												
CAPE CORAL BRIDGE	W OF BRIDGE	234	48400	47500			39700		45600	51600		13	1
CAPE CORAL BRIDGE	AT TOLL PLAZA	122									44000		
CAPTIVA DR	N OF BLIND PASS BRIDGE	319	6000	6500	6500	4500	4700					36	7
CEMETERY RD	E OF BUCKINGHAM RD	486	5200	5400	4700	4700	5400			5700		11	5
CHALLENGER BLVD	S OF COLONIAL BV	628	1700	1800	1500	1600						18	3
CHAMBERLIN PKWY	S OF DANIELS PKWY	33	2100	1800	1500	1400	1400		1200	1200	1200		
CHIQUITA BLVD	N OF SW 27TH ST	58	19600	22200	16500	16700	16600	16500	22200	17100	17700	16800	
COCONUT RD	W OF US 41	495	6000	9300			7800			7600	9200	15	6
COCONUT RD	E OF US 41	490	15100	15500	12600	9900	10700	9900		12200	12200	15	6
COLUMBUS BLVD	N OF IMMOKALEE RD	473	2400	2200	1900	1500	1800					6	5
CONSTITUTION BLVD	E OF US 41	464	6600	5700	5900	5500	4700					25	4
COLLEGE PKWY	E OF MCGREGOR BLVD	236	38000	36200	32500	31100	32900					43	3
COLLEGE PKWY	W OF SOUTH POINTE BLVD	83									38000		
COLLEGE PKWY	E OF WINKLER RD	43	40500	37900	31400	32700	31600	30400	30400	31700	32300	36100	3
COLLEGE PKWY	W OF SUMMERLIN RD	238	50000	47500	42600							43	3
COLLEGE PKWY	W OF NEW BRITANNY	87									33500		
COLLEGE PKWY	E OF KENWOOD LN	237	36500	33200	25500	29700	28700				26900	43	3
COLONIAL BLVD	E OF MCGREGOR BLVD	243	61400	58800	57400	49300	49600					14	3
COLONIAL BLVD	E OF SUMMERLIN RD	14	60400	57600	53800	51600	51600	51500	51500	52500	53100	54600	3

US 41 (SR 45)	N OF COLLIER CO LINE	23	36400	35100	34400	33900	32000	32700	33000	33900	34800	6
US 41 (SR 45)	N OF BONITA BEACH RD	437	42400	47400	49000	40400	40800					23
US 41 (SR 45)	N OF BONITA BEACH RD	92									42600	6
US 41 (SR 45)	N OF WEST TERRY ST	433	42400	36500	35900	34200						23
US 41 (SR 45)	N OF OLD 41 RD	436	53300	53600	50100	46100	42000					25
US 41 (SR 45)	S OF COCONUT RD	93									46100	6
US 41 (SR 45)	S OF HICKORY DR	25	43300	41300	40200	38600	42000		36600	37700	42500	
US 41 (SR 45)	N OF SANIBEL BLVD	424	45300	41700	37000	37200	33400					25
US 41 (SR 45)	N OF CONSTITUTION BLVD	94									33100	4
US 41 (SR 45)	N OF ALICO RD	420	57900	57700	57800	54600	53400					25
US 41 (SR 45)	N OF ISLAND PARK RD	434	56200	57200	58200	51000	44000					25
US 41 (SR 45)	N OF JAMAICA BAY WEST	435	65300	63400	58800	54700	51200					25
US 41 (SR 45)	N OF SIX MILE CYPRESS PKWY	418	52400	49400	43100	38100	42200					9
US 41 (SR 45)	N OF ANDREA LN	95									40000	4
US 41 (SR 45)	N OF CYPRESS LAKE DR	426	61200	56000	53200	54600	49400					9
US 41 (SR 45)	N OF BRANTLEY RD	9	61000	58000	50400	53300	53800	52400	50700	49100	50500	3
US 41 (SR 45)	N OF SOUTH RD	422	60800	52500	52100	49800	49900					9
US 41 (SR 45)	N OF BOY SCOUT DR	430	45700	42700	38400	36200	32400					9
US 41 (SR 45)	N OF BOY SCOUT DR	96									41100	3
US 41 (SR 45)	N OF N AIRPORT RD	427	50500	49600	43500	38100						9
US 41 (SR 45)	N OF COLONIAL BLVD	432	52000	51600	46800	35500	38800					9
US 41 (SR 45)	N OF WINKLER AVE	429	50800	53000	52100	42000						9
US 41 (SR 45)	N OF WINKLER AVE	97									42500	3
US 41 (SR 45)	N OF HANSON ST	428	46600	46400	43200	40400						9
US 41 (SR 45)	S OF VICTORIA AVE	98									35600	3
US 41 (SR 45)	N OF NORTH KEY DR	1	51900	42500	41800	41200	39700	36700	36400	41300	42400	2
US 41 (SR 45)	N OF HANCOCK BR. PWY	421	39500	32700	29900	32700						1
US 41 (SR 45)	N OF PONDELLA RD	431	32100	26300	24900	26800						1
US 41 (SR 45)	S OF PINE ISLAND RD	99									25700	2
US 41 (SR 45)	N OF PINE ISLAND RD	419	30800	25100	26100	21600						1
US 41 (SR 45)	N OF LITTLETON RD	425	23000	18300	17700	15700						1
US 41 (SR 45)	N OF LITTLETON RD	100									18100	2
US 41 (SR 45)	S OF TARA WOODS BLVD	103									21000	1
US 41 (SR 45)	S OF CHARLOTTE CO.	449	18400	15500	20700	13900						1
VANDERBILT RD	S OF BONITA BEACH RD	491	7800	6100	5700	5100						23
VETERANS PKWY	S OF PINE ISLAND RD	527	13600	14100	12900	13300	12000	12600	12400	12800	14100	50
											15400	1

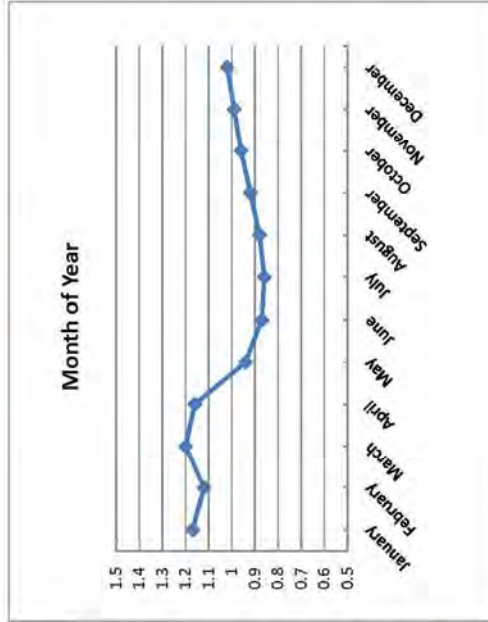
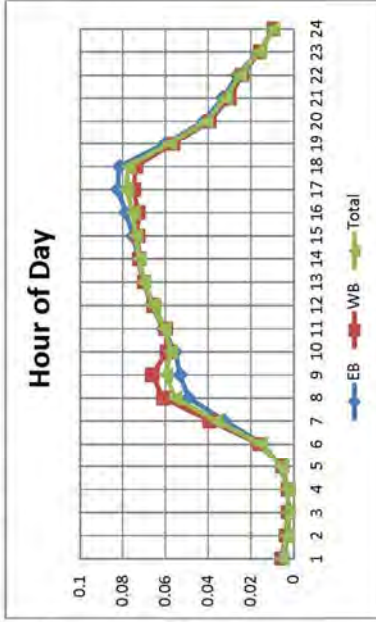
PCS 15 - Corkscrew Rd west of I-75

2015 AADT = 30,900 VPD

Hour	EB	WB	Total
0	0.49%	0.57%	0.53%
1	0.28%	0.36%	0.32%
2	0.22%	0.27%	0.25%
3	0.30%	0.27%	0.29%
4	0.60%	0.54%	0.58%
5	1.45%	1.61%	1.55%
6	3.23%	3.92%	3.61%
7	4.91%	6.11%	5.54%
8	5.31%	6.63%	6.00%
9	5.55%	5.93%	5.75%
10	6.12%	6.00%	6.05%
11	6.64%	6.57%	6.59%
12	7.08%	7.01%	7.02%
13	7.28%	7.23%	7.24%
14	7.52%	7.30%	7.39%
15	7.66%	7.28%	7.53%
16	8.25%	7.48%	7.84%
17	8.16%	7.39%	7.74%
18	6.01%	5.64%	5.82%
19	4.20%	3.84%	4.08%
20	3.33%	3.01%	3.19%
21	2.63%	2.42%	2.52%
22	1.66%	1.56%	1.62%
23	0.64%	0.94%	0.84%

Month of Year	Fraction
January	1.17
February	1.12
March	1.2
April	1.16
May	0.94
June	0.87
July	0.86
August	0.88
September	0.92
October	0.96
November	0.99
December	1.02

Directional Factor	
AM	0.55
PM	0.52
WB	EB



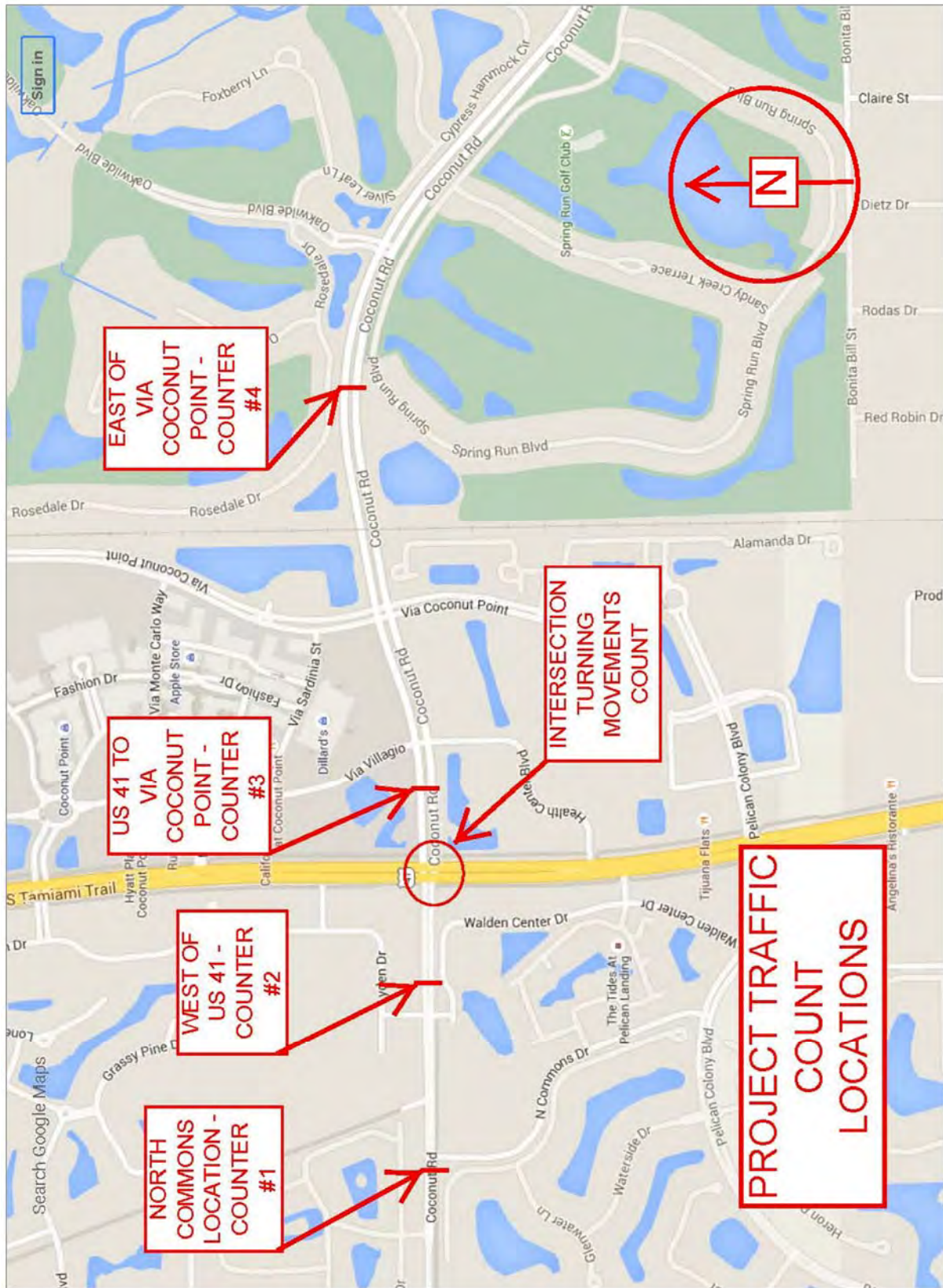
Design Hour Volume	
#	Volume Factor
5	0.114
10	0.111
20	0.108
30	0.106
50	0.104
100	0.100
150	0.098
200	0.096

Day of Week	Fraction
Sunday	0.75
Monday	1.01
Tuesday	1.05
Wednesday	1.07
Thursday	1.07
Friday	1.11
Saturday	0.93

Appendix F: TCS Corridor Volume Counts

(11 Sheets)

DRAFT



Trebilcock Consulting Solutions

1205 Piper Blvd, Suite 202

Naples, Florida 34110

Ph:239 566 9551 Fax:239 566 9553

email: ntrebilcock@trebilcock.biz

Coconut Road Volume Study
North Commons Drive
#29504

Station ID:
Site Code: 029504
Station ID:
North Commons Drive
#29504
Latitude: 0' 0.0000 Undefined

Start Time	08-Feb-16 Mon		09-Feb Tue		Westbound		Eastbound		Combined			
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.		
12:00	*	*	*	*	4	101	11	88	15	189		
12:15	*	*	*	*	7	83	2	88	9	171		
12:30	*	*	*	*	1	96	4	87	5	183		
12:45	*	*	*	*	3	105	3	91	6	196		
01:00	*	*	*	*	2	86	1	121	3	207		
01:15	*	*	*	*	3	102	6	109	9	211		
01:30	*	*	*	*	2	107	2	120	4	227		
01:45	*	*	*	*	2	101	1	102	3	203		
02:00	*	*	*	*	1	105	2	100	3	205		
02:15	*	*	*	*	3	105	0	81	3	186		
02:30	*	*	*	*	2	99	3	98	5	197		
02:45	*	*	*	*	3	111	2	108	5	219		
03:00	*	*	*	*	2	114	0	105	2	219		
03:15	*	*	*	*	1	102	1	104	2	206		
03:30	*	*	*	*	3	102	3	97	6	199		
03:45	*	*	*	*	3	123	2	98	5	221		
04:00	*	*	*	*	2	105	5	119	7	224		
04:15	*	*	*	*	6	112	5	93	11	205		
04:30	*	*	*	*	8	95	8	106	16	201		
04:45	*	*	*	*	13	108	7	90	20	198		
05:00	*	*	*	*	10	108	11	89	21	197		
05:15	*	*	*	*	13	99	10	88	23	187		
05:30	*	*	*	*	18	97	15	84	33	181		
05:45	*	*	*	*	46	107	17	96	63	203		
06:00	*	*	*	*	32	129	23	84	55	213		
06:15	*	118	*	69	39	73	29	62	68	135		
06:30	*	115	*	74	54	82	51	63	105	145		
06:45	*	69	*	51	68	82	47	46	115	128		
07:00	*	65	*	37	53	74	63	49	116	123		
07:15	*	62	*	40	42	58	69	46	111	104		
07:30	*	55	*	33	70	61	101	45	171	106		
07:45	*	44	*	40	86	69	81	42	167	111		
08:00	*	50	*	41	74	52	74	34	148	86		
08:15	*	55	*	28	90	56	78	28	168	84		
08:30	*	55	*	27	74	47	82	35	156	82		
08:45	*	47	*	22	78	49	82	18	160	67		
09:00	*	52	*	17	49	48	91	13	140	61		
09:15	*	31	*	87	65	41	74	18	139	59		
09:30	*	30	*	29	62	47	88	22	150	69		
09:45	*	28	*	27	61	27	101	38	162	65		
10:00	*	21	*	24	52	30	104	33	156	63		
10:15	*	22	*	14	53	22	100	24	153	46		
10:30	*	21	*	12	68	23	86	9	154	32		
10:45	*	19	*	15	96	22	100	18	196	40		
11:00	*	12	*	19	93	7	98	21	191	28		
11:15	*	8	*	7	96	7	107	10	203	17		
11:30	*	11	*	15	90	8	90	8	180	16		
11:45	*	4	*	3	106	8	72	5	178	13		
Total	0	994	0	731	0	1725	1809	3595	2012	3133	3821	6728
Day Total		994		731		1725	5404		5145		10549	
% Total	0.0%	57.6%	0.0%	42.4%			17.1%	34.1%	19.1%	29.7%		
Peak Vol	-	-	06:15	-	06:15	-	11:00	03:30	10:45	01:00	10:45	03:45
P.H.F.	-	-	0.778	-	0.780	-	0.908	0.898	0.923	0.934	0.948	0.950

Trebilcock Consulting Solutions

Coconut Road Volume Study
North Commons Drive
#29504

1205 Piper Blvd, Suite 202
Naples, Florida 34110
Ph:239 566 9551 Fax:239 566 9553
email: ntrebilcock@trebilcock.biz

Station ID:
Site Code: 029504
Station ID:
North Commons Drive
#29504
Latitude: 0' 0.0000 Undefined

Start Time	10-Feb-16 Wed	Westbound		Eastbound		Combined		11-Feb Thu	Westbound		Eastbound		Combined	
		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00		5	87	10	96	15	183		7	111	7	93	14	204
12:15		9	80	3	98	12	178		5	110	5	104	10	214
12:30		4	111	5	89	9	200		3	102	6	104	9	206
12:45		3	112	1	101	4	213		5	100	4	83	9	183
01:00		2	92	3	81	5	173		5	107	2	97	7	204
01:15		4	102	3	86	7	188		4	98	10	99	14	197
01:30		3	87	2	104	5	191		1	105	1	92	2	197
01:45		4	102	2	107	6	209		2	108	0	97	2	205
02:00		1	114	6	103	7	217		0	93	4	109	4	202
02:15		2	97	3	88	5	185		2	101	2	103	4	204
02:30		1	124	5	110	6	234		3	117	0	105	3	222
02:45		2	103	3	109	5	212		0	106	5	109	5	215
03:00		2	88	1	118	3	206		1	100	1	111	2	211
03:15		1	105	3	110	4	215		3	106	2	88	5	194
03:30		0	111	5	89	5	200		2	115	2	102	4	217
03:45		3	115	1	94	4	209		2	117	1	98	3	215
04:00		2	108	2	102	4	210		0	103	0	127	0	230
04:15		4	133	3	107	7	240		7	97	3	116	10	213
04:30		5	116	4	129	9	245		8	82	4	103	12	185
04:45		11	110	2	112	13	222		13	92	3	125	16	217
05:00		13	109	8	117	21	226		12	112	11	90	23	202
05:15		18	96	10	99	28	195		21	82	11	98	32	180
05:30		20	88	20	111	40	199		14	84	16	116	30	200
05:45		47	105	15	82	62	187		46	96	15	82	61	178
06:00		24	110	16	75	40	185		26	81	27	95	53	176
06:15		43	83	27	73	70	156		35	99	29	74	64	173
06:30		60	71	53	78	113	149		50	95	46	81	96	176
06:45		69	88	72	51	141	139		90	111	59	82	149	193
07:00		51	81	63	48	114	129		58	87	71	71	129	158
07:15		50	76	83	46	133	122		62	66	88	48	150	114
07:30		61	57	83	30	144	87		73	72	82	43	155	115
07:45		90	51	102	34	192	85		89	63	80	34	169	97
08:00		75	44	87	22	162	66		67	58	70	29	137	87
08:15		64	55	87	25	151	80		66	64	72	24	138	88
08:30		72	66	89	34	161	100		74	73	80	27	154	100
08:45		61	62	89	20	150	82		75	57	101	20	176	77
09:00		64	54	80	27	144	81		66	67	88	24	154	91
09:15		77	61	82	24	159	85		65	52	98	35	163	87
09:30		71	38	90	27	161	65		59	57	96	34	155	91
09:45		82	46	99	19	181	65		77	45	81	35	158	80
10:00		79	29	118	27	197	56		69	40	90	30	159	70
10:15		73	22	126	31	199	53		73	36	97	17	170	53
10:30		87	19	91	20	178	39		88	26	103	26	191	52
10:45		91	23	100	10	191	33		90	20	104	10	194	30
11:00		78	20	95	30	173	50		89	14	138	17	227	31
11:15		83	13	103	7	186	20		115	6	113	9	228	15
11:30		90	10	100	13	190	23		104	19	102	15	206	34
11:45		102	11	110	11	212	22		110	8	98	11	208	19
Total		1863	3685	2165	3224	4028	6909		1936	3760	2128	3342	4064	7102
Day Total			5548		5389		10937			5696		5470		11166
% Total		17.0%	33.7%	19.8%	29.5%				17.3%	33.7%	19.1%	29.9%		
Peak		11:00	03:45	10:00	04:15	10:00	04:15		11:00	03:15	10:30	04:00	11:00	03:30
Vol		353	472	435	465	765	933		418	441	458	471	869	875
P.H.F.		0.865	0.887	0.863	0.901	0.961	0.952		0.809	0.942	0.830	0.927	0.953	0.951

Trebilcock Consulting Solutions

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Naples, Florida 34110

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Coconut Road Volume Study
West of US 41
#28817

Station ID:
Site Code: #28817

Station ID:
West of US 41

#28817

Latitude: 0' 0.0000 Undefined

Start Time	01-Feb-16 Mon		Channel 2		Combined		02-Feb Tue		Channel 2		Combined			
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.		
12:00	*	75	*	0	*	75	8	84	0	0	8	84		
12:15	*	72	*	0	*	72	3	83	0	0	3	83		
12:30	*	100	*	0	*	100	3	101	0	0	3	101		
12:45	*	83	*	0	*	83	2	79	0	0	2	79		
01:00	*	77	*	0	*	77	3	83	0	0	3	83		
01:15	*	98	*	0	*	98	1	88	0	0	1	88		
01:30	*	132	*	0	*	132	1	109	0	0	1	109		
01:45	*	96	*	0	*	96	0	105	0	0	0	105		
02:00	*	84	*	0	*	84	3	77	0	0	3	77		
02:15	*	97	*	0	*	97	1	91	0	0	1	91		
02:30	*	94	*	0	*	94	0	102	0	0	0	102		
02:45	*	84	*	0	*	84	2	101	0	0	2	101		
03:00	*	79	*	0	*	79	4	100	0	0	4	100		
03:15	*	92	*	0	*	92	3	108	0	0	3	108		
03:30	*	101	*	0	*	101	7	87	0	0	7	87		
03:45	*	90	*	0	*	90	7	106	0	0	7	106		
04:00	*	84	*	0	*	84	4	102	0	0	4	102		
04:15	*	84	*	0	*	84	7	84	0	0	7	84		
04:30	*	92	*	0	*	92	14	96	0	0	14	96		
04:45	*	87	*	0	*	87	12	82	0	0	12	82		
05:00	*	82	*	0	*	82	15	93	0	0	15	93		
05:15	*	82	*	0	*	82	19	76	0	0	19	76		
05:30	*	86	*	0	*	86	29	101	0	0	29	101		
05:45	*	53	*	0	*	53	30	97	0	0	30	97		
06:00	*	69	*	0	*	69	36	73	0	0	36	73		
06:15	*	69	*	0	*	69	47	94	0	0	47	94		
06:30	*	64	*	0	*	64	65	69	0	0	65	69		
06:45	*	47	*	0	*	47	53	54	0	0	53	54		
07:00	*	61	*	0	*	61	58	47	0	0	58	47		
07:15	*	52	*	0	*	52	78	56	0	0	78	56		
07:30	*	48	*	0	*	48	78	61	0	0	78	61		
07:45	*	45	*	0	*	45	77	42	0	0	77	42		
08:00	*	37	*	0	*	37	103	41	0	0	103	41		
08:15	*	39	*	0	*	39	78	51	0	0	78	51		
08:30	*	46	*	0	*	46	98	37	0	0	98	37		
08:45	*	39	*	0	*	39	62	49	0	0	62	49		
09:00	*	35	*	0	*	35	76	37	0	0	76	37		
09:15	*	25	*	0	*	25	70	36	0	0	70	36		
09:30	*	28	*	0	*	28	82	31	0	0	82	31		
09:45	*	29	*	0	*	29	54	21	0	0	54	21		
10:00	*	22	*	0	*	22	89	25	0	0	89	25		
10:15	*	21	*	0	*	21	68	19	0	0	68	19		
10:30	*	21	*	0	*	21	84	10	0	0	84	10		
10:45	*	12	*	0	*	12	71	7	0	0	71	7		
11:00	*	83	7	0	0	83	81	11	0	0	81	11		
11:15	*	82	4	0	0	82	83	5	0	0	83	5		
11:30	*	113	7	0	0	113	94	8	0	0	94	8		
11:45	*	86	2	0	0	86	90	11	0	0	90	11		
Total		364	2933	0	0	364	2933	1953	3130	0	0	1953	3130	
Day Total		3297		0		3297		5083		0		5083		
% Total		11.0%	89.0%	0.0%	0.0%			38.4%	61.6%	0.0%	0.0%			
Peak	-	11:00	01:15	-	-	11:00	01:15	-	07:45	02:30	-	-	07:45	02:30
Vol.	-	364	410	-	-	364	410	-	356	411	-	-	356	411
P.H.F.	-	0.805	0.777	-	-	0.805	0.777	-	0.864	0.951	-	-	0.864	0.951

Trebilcock Consulting Solutions

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Naples, Florida 34110

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Coconut Road Volume Study
West of US 41
#28817

Station ID:
Site Code: #28817

Station ID:
West of US 41

#28817

Latitude: 0' 0.0000 Undefined

Start Time	03-Feb-16 Wed		Westbound		Channel 2		Combined		04-Feb Thu		Westbound		Channel 2		Combined	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	5	94	0	0	5	94	6	100	0	0	6	100				
12:15	2	114	0	0	2	114	9	102	0	0	9	102				
12:30	4	102	0	0	4	102	6	93	0	0	6	93				
12:45	5	101	0	0	5	101	8	99	0	0	8	99				
01:00	2	101	0	0	2	101	1	114	0	0	1	114				
01:15	1	120	0	0	1	120	1	106	0	0	1	106				
01:30	2	124	0	0	2	124	1	130	0	0	1	130				
01:45	1	108	0	0	1	108	2	77	0	0	2	77				
02:00	0	95	0	0	0	95	1	104	0	0	1	104				
02:15	3	82	0	0	3	82	2	97	0	0	2	97				
02:30	1	93	0	0	1	93	1	89	0	0	1	89				
02:45	0	88	0	0	0	88	3	94	0	0	3	94				
03:00	1	95	0	0	1	95	1	106	0	0	1	106				
03:15	0	98	0	0	0	98	3	82	0	0	3	82				
03:30	1	92	0	0	1	92	3	105	0	0	3	105				
03:45	5	78	0	0	5	78	1	98	0	0	1	98				
04:00	3	94	0	0	3	94	7	109	0	0	7	109				
04:15	10	108	0	0	10	108	5	80	0	0	5	80				
04:30	17	103	0	0	17	103	7	107	0	0	7	107				
04:45	16	81	0	0	16	81	14	84	0	0	14	84				
05:00	15	85	0	0	15	85	14	84	0	0	14	84				
05:15	18	109	0	0	18	109	25	96	0	0	25	96				
05:30	34	78	0	0	34	78	35	88	0	0	35	88				
05:45	29	89	0	0	29	89	29	99	0	0	29	99				
06:00	22	86	0	0	22	86	29	88	0	0	29	88				
06:15	59	73	0	0	59	73	53	93	0	0	53	93				
06:30	62	79	0	0	62	79	74	92	0	0	74	92				
06:45	56	46	0	0	56	46	52	68	0	0	52	68				
07:00	55	58	0	0	55	58	58	52	0	0	58	52				
07:15	81	58	0	0	81	58	74	57	0	0	74	57				
07:30	100	59	0	0	100	59	98	72	0	0	98	72				
07:45	92	61	0	0	92	61	100	43	0	0	100	43				
08:00	88	52	0	0	88	52	91	41	0	0	91	41				
08:15	78	57	0	0	78	57	78	53	0	0	78	53				
08:30	81	44	0	0	81	44	81	43	0	0	81	43				
08:45	67	41	0	0	67	41	70	54	0	0	70	54				
09:00	68	37	0	0	68	37	72	56	0	0	72	56				
09:15	80	36	0	0	80	36	69	38	0	0	69	38				
09:30	70	30	0	0	70	30	96	36	0	0	96	36				
09:45	64	32	0	0	64	32	68	34	0	0	68	34				
10:00	80	26	0	0	80	26	79	26	0	0	79	26				
10:15	74	22	0	0	74	22	96	23	0	0	96	23				
10:30	79	11	0	0	79	11	110	16	0	0	110	16				
10:45	80	17	0	0	80	17	85	13	0	0	85	13				
11:00	81	5	0	0	81	5	81	10	0	0	81	10				
11:15	111	7	0	0	111	7	107	11	0	0	107	11				
11:30	95	7	0	0	95	7	98	10	0	0	98	10				
11:45	88	7	0	0	88	7	83	6	0	0	83	6				
Total	1986	3283	0	0	1986	3283	2095	3378	0	0	2095	3378				
Day Total	5269		0		5269		5473		0		5473					
% Total	37.7%	62.3%	0.0%	0.0%	38.3%	61.7%	0.0%	0.0%	0.0%	0.0%	0.870	0.863				
Peak	-	11:00	01:00	-	-	11:00	01:00	-	-	-	-	10:30	00:45			
Vol.	-	375	453	-	-	375	453	-	-	-	-	383	449			
P.H.F.	-	0.845	0.913	-	-	0.845	0.913	-	-	-	-	0.870	0.863			

Trebilcock Consulting Solutions

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Naples, Florida 34110

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Coconut Road Volume Study
West of US 41
#29504

Station ID:
Site Code: #29504
Station ID:
West of US 41
#29504

Latitude: 0' 0.0000 Undefined

Start Time	01-Feb-16 Mon	Eastbound		Channel 2		Combined		02-Feb Tue	Eastbound		Channel 2		Combined	
		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	147	*	0	*	147	*	5	139	0	0	5	139	
12:15	*	109	*	0	*	109	*	5	121	0	0	5	121	
12:30	*	110	*	0	*	110	*	5	129	0	0	5	129	
12:45	*	104	*	0	*	104	*	4	107	0	0	4	107	
01:00	*	106	*	0	*	106	*	3	165	0	0	3	165	
01:15	*	106	*	0	*	106	*	5	124	0	0	5	124	
01:30	*	114	*	0	*	114	*	1	106	0	0	1	106	
01:45	*	111	*	0	*	111	*	1	109	0	0	1	109	
02:00	*	117	*	0	*	117	*	1	128	0	0	1	128	
02:15	*	114	*	0	*	114	*	0	117	0	0	0	117	
02:30	*	122	*	0	*	122	*	1	136	0	0	1	136	
02:45	*	140	*	0	*	140	*	0	127	0	0	0	127	
03:00	*	138	*	0	*	138	*	2	135	0	0	2	135	
03:15	*	87	*	0	*	87	*	4	134	0	0	4	134	
03:30	*	130	*	0	*	130	*	4	128	0	0	4	128	
03:45	*	149	*	0	*	149	*	2	133	0	0	2	133	
04:00	*	137	*	0	*	137	*	4	162	0	0	4	162	
04:15	*	132	*	0	*	132	*	5	132	0	0	5	132	
04:30	*	142	*	0	*	142	*	5	140	0	0	5	140	
04:45	*	129	*	0	*	129	*	9	132	0	0	9	132	
05:00	*	161	*	0	*	161	*	14	174	0	0	14	174	
05:15	*	111	*	0	*	111	*	10	133	0	0	10	133	
05:30	*	124	*	0	*	124	*	23	129	0	0	23	129	
05:45	*	118	*	0	*	118	*	20	138	0	0	20	138	
06:00	*	102	*	0	*	102	*	26	105	0	0	26	105	
06:15	*	77	*	0	*	77	*	32	89	0	0	32	89	
06:30	*	76	*	0	*	76	*	47	82	0	0	47	82	
06:45	*	92	*	0	*	92	*	60	100	0	0	60	100	
07:00	*	54	*	0	*	54	*	68	57	0	0	68	57	
07:15	*	43	*	0	*	43	*	71	49	0	0	71	49	
07:30	*	42	*	0	*	42	*	86	40	0	0	86	40	
07:45	*	40	*	0	*	40	*	90	27	0	0	90	27	
08:00	*	34	*	0	*	34	*	130	37	0	0	130	37	
08:15	*	27	*	0	*	27	*	100	32	0	0	100	32	
08:30	*	24	*	0	*	24	*	114	41	0	0	114	41	
08:45	*	16	*	0	*	16	*	108	43	0	0	108	43	
09:00	*	19	*	0	*	19	*	87	22	0	0	89	22	
09:15	*	15	*	0	*	15	*	104	43	0	0	104	43	
09:30	*	18	*	0	*	18	*	101	25	0	0	101	25	
09:45	*	15	*	0	*	15	*	95	19	0	0	95	19	
10:00	*	17	*	0	*	17	*	113	34	0	0	113	34	
10:15	*	19	*	0	*	19	*	122	15	0	0	122	15	
10:30	*	15	*	0	*	15	*	126	18	0	0	126	18	
10:45	*	14	*	0	*	14	*	131	7	0	0	131	7	
11:00	*	135	17	1	0	136	17	101	15	0	0	101	15	
11:15	*	125	5	0	0	125	5	124	16	0	0	124	16	
11:30	*	119	11	0	0	119	11	116	10	0	0	116	10	
11:45	*	119	3	0	0	119	3	120	2	0	0	120	2	
Total		498	3753	1	0	499	3753	2405	4106	2	0	2407	4106	
Day Total		4251		1		4252		6511		2		6513		
% Total		11.7%	88.3%	0.0%	0.0%			36.9%	63.0%	0.0%	0.0%			
Peak	-	11:00	04:15	11:00	-	11:00	04:15	-	10:00	04:30	08:15	-	10:00	04:30
Vol.	-	498	564	1	-	499	564	-	492	579	2	-	492	579
P.H.F.		0.922	0.876	0.250		0.917	0.876		0.939	0.832	0.250		0.939	0.832

Trebilcock Consulting Solutions

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Naples, Florida 34110

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Coconut Road Volume Study
West of US 41
#29504

Station ID:
Site Code: #29504
Station ID:
West of US 41
#29504

Latitude: 0' 0.0000 Undefined

Start Time	03-Feb-16 Wed		Eastbound		Channel 2		Combined		04-Feb Thu		Eastbound		Channel 2		Combined	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	7	118	0	0	7	118	4	146	0	0	4	146				
12:15	4	112	0	0	4	112	4	145	0	0	4	145				
12:30	8	130	0	0	8	130	10	124	0	0	10	124				
12:45	4	128	0	0	4	128	2	118	0	0	2	118				
01:00	4	114	0	0	4	114	3	113	0	0	3	113				
01:15	9	99	0	0	9	99	1	115	0	0	1	115				
01:30	2	123	0	0	2	123	3	146	0	0	3	146				
01:45	2	125	0	0	2	125	4	89	0	0	4	89				
02:00	0	123	0	0	0	123	9	119	0	0	9	119				
02:15	1	128	0	0	1	128	4	126	0	0	4	126				
02:30	4	136	0	0	4	136	1	139	0	0	1	139				
02:45	1	123	0	0	1	123	1	118	0	0	1	118				
03:00	0	143	0	0	0	143	1	159	0	0	1	159				
03:15	1	103	0	0	1	103	0	145	0	0	0	145				
03:30	5	162	0	0	5	162	3	134	0	0	3	134				
03:45	1	126	0	0	1	126	4	121	0	0	4	121				
04:00	2	165	0	0	2	165	3	149	0	0	3	149				
04:15	4	142	0	0	4	142	5	128	0	0	5	128				
04:30	5	165	0	0	5	165	6	148	0	0	6	148				
04:45	2	154	0	0	2	154	5	160	0	0	5	160				
05:00	11	192	0	0	11	192	11	151	0	0	11	151				
05:15	9	135	0	0	9	135	11	142	0	0	11	142				
05:30	17	137	0	0	17	137	24	141	0	0	24	141				
05:45	20	124	0	0	20	124	14	139	0	0	14	139				
06:00	24	118	0	0	24	118	31	127	0	0	31	127				
06:15	37	112	0	0	37	112	33	92	0	0	33	92				
06:30	49	89	0	0	49	89	42	98	0	0	42	98				
06:45	59	71	0	0	59	71	57	93	0	0	57	93				
07:00	76	66	0	0	76	66	61	70	0	0	61	70				
07:15	87	47	0	0	87	47	76	52	0	0	76	52				
07:30	102	42	0	0	102	42	108	55	0	0	108	55				
07:45	102	23	0	0	102	23	101	51	0	0	101	51				
08:00	110	39	0	0	110	39	99	36	0	0	99	36				
08:15	119	33	0	0	119	33	113	41	0	0	113	41				
08:30	121	36	0	0	121	36	108	40	0	0	108	40				
08:45	109	32	0	0	109	32	116	45	0	0	116	45				
09:00	98	28	0	0	98	28	102	54	0	0	102	54				
09:15	80	66	0	0	80	66	130	22	0	0	130	22				
09:30	118	35	0	0	118	35	125	40	0	0	125	40				
09:45	141	23	0	0	141	23	135	29	0	0	135	29				
10:00	113	18	0	0	113	18	132	21	0	0	132	21				
10:15	113	14	0	0	113	14	138	34	0	0	138	34				
10:30	104	21	0	0	104	21	118	14	0	0	118	14				
10:45	114	10	0	0	114	10	146	23	0	0	146	23				
11:00	114	13	0	0	114	13	104	22	0	0	104	22				
11:15	126	12	0	0	126	12	120	6	0	0	120	6				
11:30	108	9	0	0	108	9	148	12	0	0	148	12				
11:45	133	6	0	0	133	6	132	13	0	0	132	13				
Total	2490	4168	0	0	2490	4168	2608	4305	0	0	2608	4305				
Day Total	6658		0		6658		6913		0		6913					
% Total	37.4%	62.6%	0.0%	0.0%			37.7%	62.3%	0.0%	0.0%						
Peak	-	09:30	04:15	-	-	09:30	04:15	-	10:00	04:30	-	-	10:00	04:30		
Vol.	-	485	653	-	-	485	653	-	534	601	-	-	534	601		
P.H.F.	-	0.860	0.850	-	-	0.860	0.850	-	0.914	0.939	-	-	0.914	0.939		

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Coconut Road Volume Study
41 to Via Vilagio
#29856

Station ID:
Site Code: #29856

Station ID:
41 to Via Vilagio
#29856

Latitude: 0' 0.0000 Undefined

Start Time	01-Feb-16 Mon		Westbound		Eastbound		Combined		02-Feb Tue		Westbound		Eastbound		Combined	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	186	*	182	*	368	0	183	3	122	3	305				
12:15	*	178	*	149	*	327	3	202	1	145	4	347				
12:30	*	168	*	161	*	329	0	169	1	99	1	268				
12:45	*	172	*	168	*	340	1	200	2	117	3	317				
01:00	*	161	*	140	*	301	2	181	3	114	5	295				
01:15	*	183	*	156	*	339	0	205	0	116	0	321				
01:30	*	188	*	148	*	336	2	174	1	119	3	293				
01:45	*	212	*	167	*	379	1	193	0	127	1	320				
02:00	*	168	*	178	*	346	4	202	1	126	5	328				
02:15	*	164	*	155	*	319	1	156	0	138	1	294				
02:30	*	182	*	202	*	384	0	189	2	113	2	302				
02:45	*	186	*	149	*	335	2	196	0	130	2	326				
03:00	*	190	*	170	*	360	0	197	0	96	0	293				
03:15	*	198	*	152	*	350	0	181	1	139	1	320				
03:30	*	186	*	185	*	371	0	186	1	126	1	312				
03:45	*	169	*	185	*	354	0	188	1	142	1	330				
04:00	*	163	*	190	*	353	5	184	2	135	7	319				
04:15	*	170	*	167	*	337	2	190	6	146	8	336				
04:30	*	165	*	197	*	362	4	158	9	141	13	299				
04:45	*	178	*	179	*	357	6	191	8	123	14	314				
05:00	*	189	*	185	*	374	6	190	10	133	16	323				
05:15	*	180	*	177	*	357	7	166	6	133	13	299				
05:30	*	153	*	178	*	331	5	148	19	111	24	259				
05:45	*	147	*	162	*	309	19	148	19	131	38	279				
06:00	*	158	*	143	*	301	18	157	17	107	35	264				
06:15	*	122	*	121	*	243	34	139	29	85	63	224				
06:30	*	136	*	114	*	250	60	133	41	91	101	224				
06:45	*	115	*	104	*	219	73	99	86	88	159	187				
07:00	*	86	*	87	*	173	81	84	52	53	133	137				
07:15	*	84	*	86	*	170	114	67	70	57	184	124				
07:30	*	66	*	75	*	141	143	67	75	38	218	105				
07:45	*	61	*	74	*	135	165	66	85	27	250	93				
08:00	*	52	*	68	*	120	159	54	107	55	266	109				
08:15	*	39	*	57	*	96	164	53	95	43	259	96				
08:30	*	36	*	54	*	90	141	46	120	38	261	84				
08:45	*	43	*	46	*	89	153	74	124	43	277	117				
09:00	*	51	*	44	*	95	119	59	92	27	211	86				
09:15	*	52	*	45	*	97	138	50	91	35	229	85				
09:30	*	32	*	29	*	61	156	50	94	25	250	75				
09:45	*	34	*	27	*	61	154	37	77	26	231	63				
10:00	*	27	*	20	*	47	146	22	92	22	238	44				
10:15	*	22	*	28	*	50	169	23	95	25	264	48				
10:30	*	21	*	15	*	36	164	16	98	12	262	28				
10:45	*	15	*	8	*	23	149	16	100	4	249	20				
11:00	*	12	*	10	*	22	159	14	101	6	260	20				
11:15	*	5	*	4	*	9	156	10	105	4	261	14				
11:30		112	7	75	5	187	184	4	119	5	303	9				
11:45		196	3	190	9	386	190	6	128	3	318	9				
Total		308	5515	265	5355	573	10870	3259	5723	2189	3941	5448	9664			
Day Total			5823		5620		11443		8982		6130		15112			
% Total			2.7%	48.2%	2.3%	46.8%		21.6%	37.9%	14.5%	26.1%					
Peak	-	-	02:45	-	03:45	-	04:30	-	11:00	01:15	11:00	03:45	11:00	03:30		
Vpk	-	-	760	-	739	-	1450	-	689	774	453	564	1142	1297		
P.H.F.			0.960		0.938		0.969		0.907	0.944	0.885	0.966	0.898	0.965		

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Coconut Road Volume Study
41 to Via Vilagio
#29856

Station ID:
Site Code: #29856
Station ID:
41 to Via Vilagio
#29856

Latitude: 0° 0.0000 Undefined

Start Time	03-Feb-16 Wed	Westbound		Eastbound		Combined		04-Feb Thu	Westbound		Eastbound		Combined	
		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	3	191	2	122	5	313		6	196	4	188	10	384	
12:15	6	226	5	98	11	324		2	206	2	186	4	392	
12:30	2	187	6	122	8	309		3	207	1	177	4	384	
12:45	0	197	2	122	2	319		2	198	2	162	4	360	
01:00	2	155	1	96	3	251		3	188	2	183	5	371	
01:15	1	209	1	112	2	321		4	186	0	184	4	370	
01:30	1	193	2	123	3	316		0	216	0	176	0	392	
01:45	1	211	0	132	1	343		0	204	1	171	1	375	
02:00	3	176	0	104	3	280		1	188	1	170	2	358	
02:15	2	170	0	117	2	287		1	180	0	185	1	365	
02:30	1	171	0	115	1	286		2	185	0	166	2	351	
02:45	2	180	2	129	4	309		0	162	0	186	0	348	
03:00	1	181	0	108	1	289		0	152	0	147	0	299	
03:15	1	187	0	134	1	321		0	169	0	204	0	373	
03:30	0	203	0	132	0	335		3	186	0	184	3	370	
03:45	2	185	1	127	3	312		0	170	2	195	2	365	
04:00	4	174	1	138	5	312		4	172	2	194	6	366	
04:15	2	176	2	148	4	324		1	205	5	191	6	396	
04:30	5	189	1	118	6	307		5	180	3	211	8	391	
04:45	3	211	2	142	5	353		3	186	3	197	6	383	
05:00	6	158	8	138	14	296		5	153	6	171	11	324	
05:15	4	171	3	151	7	322		7	181	3	191	10	372	
05:30	14	144	8	129	22	273		9	153	9	169	18	322	
05:45	22	131	7	110	29	241		20	131	13	194	33	325	
06:00	15	159	10	114	25	273		23	154	10	166	33	320	
06:15	32	139	19	98	51	237		26	123	21	122	47	245	
06:30	55	127	29	82	84	209		46	133	32	104	78	237	
06:45	69	109	65	74	134	183		73	131	66	104	139	235	
07:00	87	86	37	64	124	150		72	105	26	96	98	201	
07:15	122	82	44	59	166	141		98	82	30	54	128	136	
07:30	143	58	65	55	208	113		145	80	61	78	206	158	
07:45	167	71	57	42	224	113		158	80	63	59	221	139	
08:00	150	68	69	46	219	114		165	77	75	72	240	149	
08:15	175	61	58	41	234	102		174	67	58	57	232	124	
08:30	161	58	59	31	220	89		133	54	73	53	206	107	
08:45	149	48	74	29	223	77		147	56	75	55	222	111	
09:00	142	51	72	26	214	77		134	58	84	68	218	126	
09:15	135	48	62	31	197	79		145	63	116	45	261	108	
09:30	167	39	73	31	240	70		164	56	138	28	302	84	
09:45	144	31	97	26	241	57		175	36	119	33	294	69	
10:00	156	43	106	19	262	62		146	30	162	31	308	61	
10:15	155	25	104	14	259	39		183	25	140	30	323	55	
10:30	168	24	104	11	272	35		187	13	158	20	345	33	
10:45	180	11	112	4	292	15		204	12	197	13	401	25	
11:00	165	10	108	10	273	20		184	13	200	17	384	30	
11:15	181	8	122	3	303	11		195	4	173	6	368	10	
11:30	199	2	108	2	307	4		190	8	178	6	368	14	
11:45	186	6	143	2	329	8		209	7	189	6	398	13	
Total		3402	5740	1851	3881	5253	9621		3457	5821	2503	5705	5960	11526
Day Total			9142		5732		14874			9278		8208		17486
% Total			22.9%	38.6%	12.4%	26.1%			19.8%	33.3%	14.3%	32.6%		
Peak	-	11:00	12:00	11:00	04:45	11:00	04:00	-	11:00	12:00	10:45	04:00	10:45	04:00
Vol.	-	741	801	481	560	1222	1296	-	778	807	748	793	1521	1536
P.H.F.	-	0.931	0.886	0.841	0.927	0.901	0.918	-	0.931	0.975	0.935	0.940	0.948	0.971

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Coconut Road Volume Study
East of Via Cocnut Point
#29737

Station ID:
Site Code: #29737
Station ID:
East of Via Cocnut Point
#29737
Latitude: 0' 0.0000 Undefined

Start Time	01-Feb-16 Mon		Westbound		Eastbound		Combined		02-Feb Tue		Westbound		Eastbound		Combined	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	*	*	*	*	*	*	*	*	1	198	2	175	3	373		
12:15	*	*	*	*	*	*	*	*	4	178	3	176	7	354		
12:30	*	*	*	*	*	*	*	*	0	162	3	171	3	333		
12:45	*	178	*	153	*	331	*	0	197	4	167	4	364			
01:00	*	153	*	150	*	303	*	2	155	3	179	5	334			
01:15	*	161	*	156	*	317	*	0	194	0	186	0	380			
01:30	*	166	*	140	*	306	*	2	170	1	179	3	349			
01:45	*	176	*	191	*	367	*	2	186	0	189	2	375			
02:00	*	181	*	187	*	368	*	2	176	0	148	2	324			
02:15	*	166	*	170	*	336	*	1	157	0	192	1	349			
02:30	*	170	*	186	*	356	*	1	164	1	168	2	332			
02:45	*	161	*	169	*	330	*	1	175	0	181	1	356			
03:00	*	145	*	199	*	344	*	0	159	0	179	0	338			
03:15	*	149	*	181	*	330	*	0	142	2	190	2	332			
03:30	*	152	*	203	*	355	*	1	139	1	211	2	350			
03:45	*	153	*	188	*	341	*	0	164	0	219	0	383			
04:00	*	156	*	198	*	354	*	3	149	4	218	7	367			
04:15	*	150	*	177	*	327	*	3	160	5	217	8	377			
04:30	*	155	*	226	*	381	*	5	157	10	199	15	356			
04:45	*	174	*	179	*	353	*	8	172	6	178	14	350			
05:00	*	150	*	209	*	359	*	5	162	11	186	16	348			
05:15	*	160	*	183	*	343	*	7	164	6	207	13	371			
05:30	*	154	*	168	*	322	*	5	160	20	183	25	343			
05:45	*	142	*	169	*	311	*	15	156	19	188	34	344			
06:00	*	128	*	153	*	281	*	21	137	11	165	32	302			
06:15	*	135	*	144	*	279	*	34	128	23	127	57	255			
06:30	*	127	*	121	*	248	*	53	134	23	124	76	258			
06:45	*	109	*	114	*	223	*	63	105	42	120	105	225			
07:00	*	86	*	103	*	189	*	69	82	44	97	113	179			
07:15	*	54	*	77	*	131	*	92	64	62	103	154	167			
07:30	*	64	*	110	*	174	*	136	40	76	88	212	128			
07:45	*	33	*	80	*	113	*	168	46	97	83	265	129			
08:00	*	36	*	89	*	125	*	151	28	112	93	263	121			
08:15	*	34	*	82	*	116	*	152	33	98	88	250	121			
08:30	*	21	*	82	*	103	*	156	25	113	84	269	109			
08:45	*	22	*	71	*	93	*	168	42	110	86	278	128			
09:00	*	24	*	65	*	89	*	126	30	94	81	220	111			
09:15	*	25	*	52	*	77	*	152	19	89	74	241	93			
09:30	*	14	*	51	*	65	*	164	20	129	62	293	82			
09:45	*	24	*	43	*	67	*	167	21	122	44	289	65			
10:00	*	20	*	31	*	51	*	177	13	109	48	286	61			
10:15	*	11	*	31	*	42	*	176	9	149	36	325	45			
10:30	*	18	*	18	*	36	*	170	14	149	27	319	41			
10:45	*	9	*	11	*	20	*	174	8	156	10	330	18			
11:00	*	11	*	11	*	22	*	185	8	148	13	333	21			
11:15	*	3	*	5	*	8	*	182	4	175	8	357	12			
11:30	*	5	*	8	*	13	*	174	5	163	8	337	13			
11:45	*	2	*	7	*	9	*	189	5	162	3	351	8			
Total		0	4367	0	5341	0	9708	3367	5016	2557	6158	5924	11174			
Day Total			4367		5341		9708		8383		8715		17098			
% Total			0.0%	45.0%	0.0%	55.0%		19.7%	29.3%	15.0%	36.0%					
Peak			01:45		04:30		04:30		11:00	12:00	11:00	03:30	11:00	03:45		
Vol			693		797		1436		730	735	648	855	1378	1483		
P.H.F			0.957		0.882		0.942		0.966	0.928	0.926	0.987	0.965	0.968		

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Coconut Road Volume Study
East of Via Cocnut Point
#29737

Station ID:
Site Code: #29737
Station ID:
East of Via Cocnut Point
#29737
Latitude: 0° 0.0000 Undefined

Start Time	03-Feb-16 Wed		Westbound		Eastbound		Combined		04-Feb Thu	Westbound		Eastbound		Combined	
	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.		A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00	1	200	4	156	5	356			3	199	3	193	6	392	
12:15	4	195	5	176	9	371			2	186	4	199	6	385	
12:30	1	176	8	158	9	334			2	196	3	171	5	367	
12:45	0	191	4	151	4	342			0	171	1	155	1	326	
01:00	1	133	1	151	2	284			2	194	2	185	4	379	
01:15	0	191	1	146	1	337			5	162	1	193	6	355	
01:30	0	156	1	181	1	337			0	188	0	205	0	393	
01:45	1	159	2	176	3	335			1	175	1	174	2	349	
02:00	1	162	0	178	1	340			0	155	1	177	1	332	
02:15	1	149	0	171	1	320			1	177	1	199	2	376	
02:30	3	158	1	193	4	351			2	164	1	184	3	348	
02:45	0	159	2	200	2	359			0	142	1	206	1	348	
03:00	1	144	1	200	2	344			0	154	0	190	0	344	
03:15	1	172	2	196	3	368			0	149	2	213	2	362	
03:30	0	157	1	212	1	369			4	141	0	214	4	355	
03:45	3	168	1	200	4	368			1	158	2	202	3	360	
04:00	4	145	2	224	6	369			4	162	3	189	7	351	
04:15	2	159	4	190	6	349			0	163	6	194	6	357	
04:30	4	151	5	226	9	377			5	148	6	213	11	361	
04:45	7	182	5	203	12	385			5	173	7	198	12	371	
05:00	6	146	15	219	21	365			5	145	10	200	15	345	
05:15	5	171	4	206	9	377			7	181	4	204	11	385	
05:30	18	158	17	200	35	358			9	173	21	174	30	347	
05:45	20	139	15	157	35	296			20	158	18	189	38	347	
06:00	20	161	17	165	37	326			25	147	17	177	42	324	
06:15	30	121	25	162	55	283			29	135	27	135	56	270	
06:30	53	145	31	117	84	262			39	144	27	134	66	278	
06:45	56	121	45	109	101	230			62	121	38	114	100	235	
07:00	71	78	64	98	135	176			68	74	53	124	121	198	
07:15	106	72	62	125	168	197			88	81	55	85	143	166	
07:30	148	38	86	91	234	129			125	53	90	91	215	144	
07:45	176	39	100	86	276	125			173	65	95	92	268	157	
08:00	138	42	92	92	230	134			165	50	106	109	271	159	
08:15	149	44	97	81	246	125			172	47	102	85	274	132	
08:30	151	28	87	99	238	127			146	35	116	78	262	113	
08:45	171	24	112	61	283	85			145	39	101	88	246	127	
09:00	134	18	95	59	229	77			170	27	113	100	283	127	
09:15	145	23	111	67	256	90			179	27	132	65	311	92	
09:30	172	21	101	53	273	74			205	29	148	49	353	78	
09:45	164	15	127	54	291	69			201	23	118	47	319	70	
10:00	175	13	139	51	314	64			169	21	152	42	321	63	
10:15	139	17	149	21	288	38			196	22	136	30	332	52	
10:30	160	19	115	15	275	34			204	10	157	24	361	34	
10:45	178	6	150	11	328	17			207	9	174	19	381	28	
11:00	171	6	177	10	348	16			181	9	172	17	353	26	
11:15	178	6	152	6	330	12			181	3	181	8	362	11	
11:30	184	3	171	4	355	7			190	4	190	4	380	8	
11:45	193	2	171	5	364	7			194	6	172	5	366	11	
Total		3346	4983	2577	6112	5923	11095		3592	5195	2770	6343	6362	11538	
Day Total		8329		8689		17018			8787		9113		17900		
% Total		19.7%	29.3%	15.1%	35.9%				20.1%	29.0%	15.5%	35.4%			
Peak	-	11:00	12:00	11:00	04:30	11:00	04:30	-	10:15	12:00	10:45	02:45	10:45	01:00	
Vcl.	-	726	762	671	854	1397	1504	-	788	752	717	823	1476	1476	
P.H.F.	-	0.940	0.953	0.948	0.945	0.959	0.977	-	0.952	0.945	0.943	0.961	0.969	0.939	

**Appendix G: 2014 FDOT Peak Season Factor Category
Report (Excerpt)**
(1 Sheet)

2014 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL
 CATEGORY: 1252 BONITA SPRINGS AREA

MOCF: 0.82

WEEK	DATES	SF	PSCF
1	01/01/2014 - 01/04/2014	0.98	1.20
2	01/05/2014 - 01/11/2014	0.92	1.12
* 3	01/12/2014 - 01/18/2014	0.87	1.06
* 4	01/19/2014 - 01/25/2014	0.85	1.04
* 5	01/26/2014 - 02/01/2014	0.84	1.02
* 6	02/02/2014 - 02/08/2014	0.82	1.00
* 7	02/09/2014 - 02/15/2014	0.81	0.99
* 8	02/16/2014 - 02/22/2014	0.79	0.96
* 9	02/23/2014 - 03/01/2014	0.79	0.96
*10	03/02/2014 - 03/08/2014	0.79	0.96
*11	03/09/2014 - 03/15/2014	0.78	0.95
*12	03/16/2014 - 03/22/2014	0.78	0.95
*13	03/23/2014 - 03/29/2014	0.81	0.99
*14	03/30/2014 - 04/05/2014	0.84	1.02
*15	04/06/2014 - 04/12/2014	0.86	1.05
16	04/13/2014 - 04/19/2014	0.89	1.09
17	04/20/2014 - 04/26/2014	0.93	1.13
18	04/27/2014 - 05/03/2014	0.96	1.17
19	05/04/2014 - 05/10/2014	1.00	1.22
20	05/11/2014 - 05/17/2014	1.04	1.27
21	05/18/2014 - 05/24/2014	1.08	1.32
22	05/25/2014 - 05/31/2014	1.10	1.34
23	06/01/2014 - 06/07/2014	1.13	1.38
24	06/08/2014 - 06/14/2014	1.16	1.41
25	06/15/2014 - 06/21/2014	1.19	1.45
26	06/22/2014 - 06/28/2014	1.20	1.46
27	06/29/2014 - 07/05/2014	1.21	1.48
28	07/06/2014 - 07/12/2014	1.21	1.48
29	07/13/2014 - 07/19/2014	1.22	1.49
30	07/20/2014 - 07/26/2014	1.22	1.49
31	07/27/2014 - 08/02/2014	1.23	1.50
32	08/03/2014 - 08/09/2014	1.23	1.50
33	08/10/2014 - 08/16/2014	1.23	1.50
34	08/17/2014 - 08/23/2014	1.24	1.51
35	08/24/2014 - 08/30/2014	1.24	1.51
36	08/31/2014 - 09/06/2014	1.25	1.52
37	09/07/2014 - 09/13/2014	1.25	1.52
38	09/14/2014 - 09/20/2014	1.26	1.54
39	09/21/2014 - 09/27/2014	1.21	1.48
40	09/28/2014 - 10/04/2014	1.16	1.41
41	10/05/2014 - 10/11/2014	1.11	1.35
42	10/12/2014 - 10/18/2014	1.07	1.30
43	10/19/2014 - 10/25/2014	1.04	1.27
44	10/26/2014 - 11/01/2014	1.02	1.24
45	11/02/2014 - 11/08/2014	1.00	1.22
46	11/09/2014 - 11/15/2014	0.97	1.18
47	11/16/2014 - 11/22/2014	0.95	1.16
48	11/23/2014 - 11/29/2014	0.96	1.17
49	11/30/2014 - 12/06/2014	0.97	1.18
50	12/07/2014 - 12/13/2014	0.97	1.18
51	12/14/2014 - 12/20/2014	0.98	1.20
52	12/21/2014 - 12/27/2014	0.92	1.12
53	12/28/2014 - 12/31/2014	0.87	1.06

* PEAK SEASON

09-MAR-2015 16:07:48

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1_1252_PKSEASON.TXT

**Appendix H: 2013 Lee County Link – Specific Volumes
(Excerpt)**
(1 Sheet)

LINK-SPECIFIC SERVICE VOLUMES ON ARTERIALS IN LEE COUNTY (2013 DATA)

May, 2014

ROAD SEGMENT	FROM	TO	TRAFFIC DISTRICT	LENGTH (MILE)	ROAD TYPE	SERVICE VOLUMES (PEAK HOUR PEAK DIRECTION)					SERVICE VOLUMES (PEAK HOUR-BOTH DIRECTIONS)				
						A	B	C	D	E	A	B	C	D	E
VETERANS MEM. PKWY	McGREGOR BLVD	DEL PRADO BLVD	1 & 5	3.5	4LB	1,120	1,900	2,680	3,440	4,000	1,740	2,900	4,120	5,260	6,190
	DEL PRADO BLVD	SANTA BARBARA BLVD	5	2.0	6LD	2,860	3,120	3,120	3,120	3,120	4,770	5,220	5,220	5,220	5,220
	SANTA BARBARA BLVD	SKYLINE BLVD	5	1.0	6LD	2,860	3,120	3,120	3,120	3,120	4,770	5,220	5,220	5,220	5,220
	SKYLINE BLVD	SR 78	5	3.5	4LD	1,830	2,080	2,080	2,080	2,080	3,090	3,460	3,460	3,460	3,460
	WINKLER RD	SUMMERLIN RD		4	0.4	4LD	0	0	0	950	1,520	0	0	1,520	2,560
WINKLER RD	GLADIOLUS DR	BRANDYWINE CIR	4	0.9	2LN	0	780	920	920	920	0	1,340	1,570	1,570	1,570
	BRANDYWINE CIR	CYPRESS LAKE DR	4	0.9	2LN	0	780	920	920	920	0	1,340	1,570	1,570	1,570
	CYPRESS LAKE DR	COLLEGE PKWY	4	0.7	4LD	0	0	530	1,800	1,800	0	0	1,410	3,040	3,040
	COLLEGE PKWY	SUNSET VISTA	4	0.5	2LN	0	810	840	840	840	0	1,380	1,430	1,430	1,430
	SUNSET VISTA	McGREGOR BLVD	4	0.8	2LN	0	810	840	840	840	0	1,380	1,430	1,430	1,430

SERVICE VOLUMES ON COLLECTORS IN LEE COUNTY (2013 DATA)

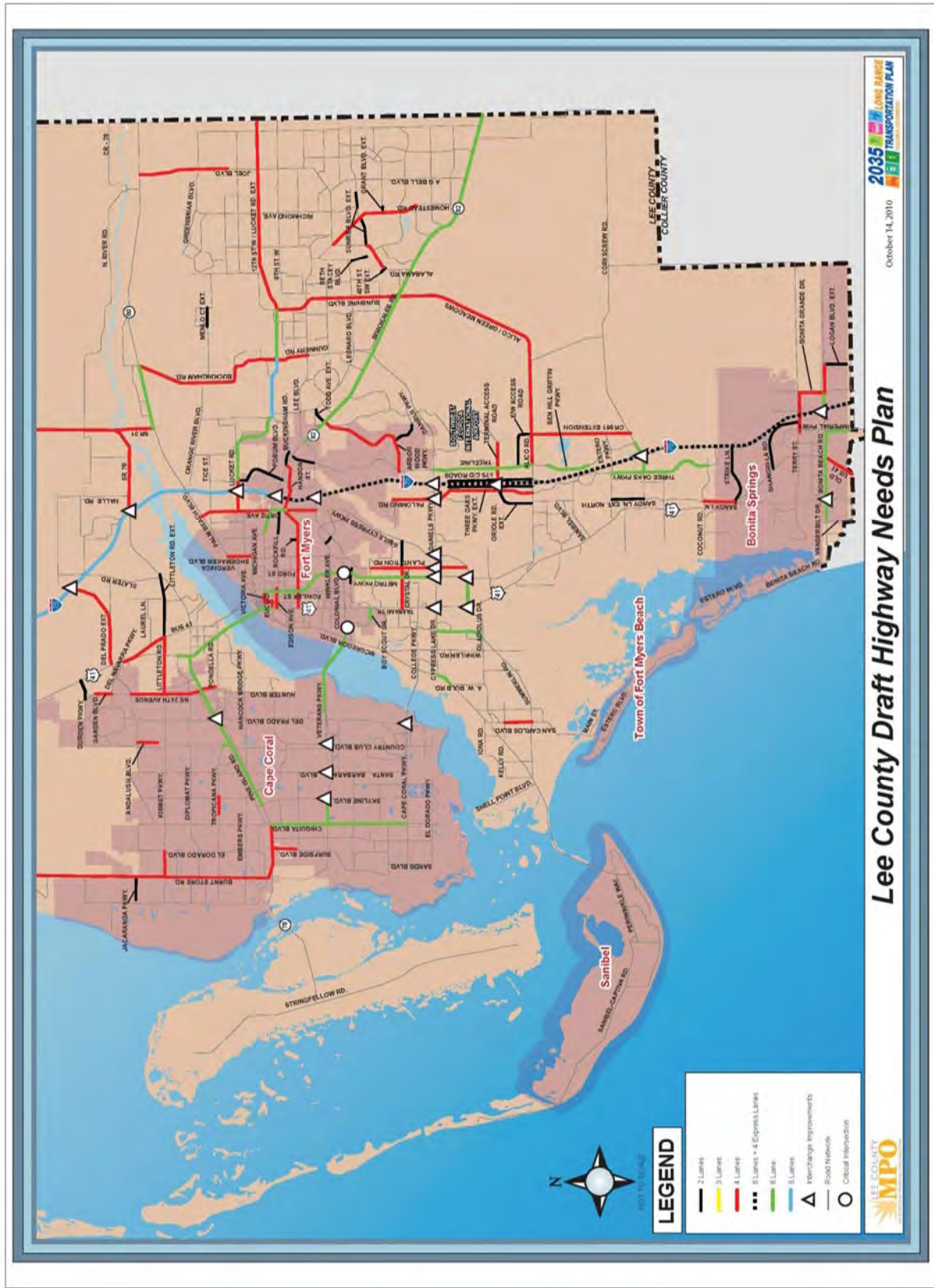
ROAD SEGMENT	FROM	TO	TRAFFIC DISTRICT	LENGTH (MILE)	ROAD TYPE	SERVICE VOLUMES (PEAK HOUR PEAK DIRECTION)					SERVICE VOLUMES (PEAK HOUR-BOTH DIRECTIONS)				
						A	B	C	D	E	A	B	C	D	E
COLLECTORS					2LU	0	0	550	890	960	0	0	960	1,530	1,530
					2LD	0	0	580	910	910	0	0	1,040	1,610	1,610
					4LU	0	0	1,240	1,700	1,700	0	0	2,200	3,030	3,030
					4LD	0	0	1,310	1,760	1,760	0	0	2,340	3,180	3,180

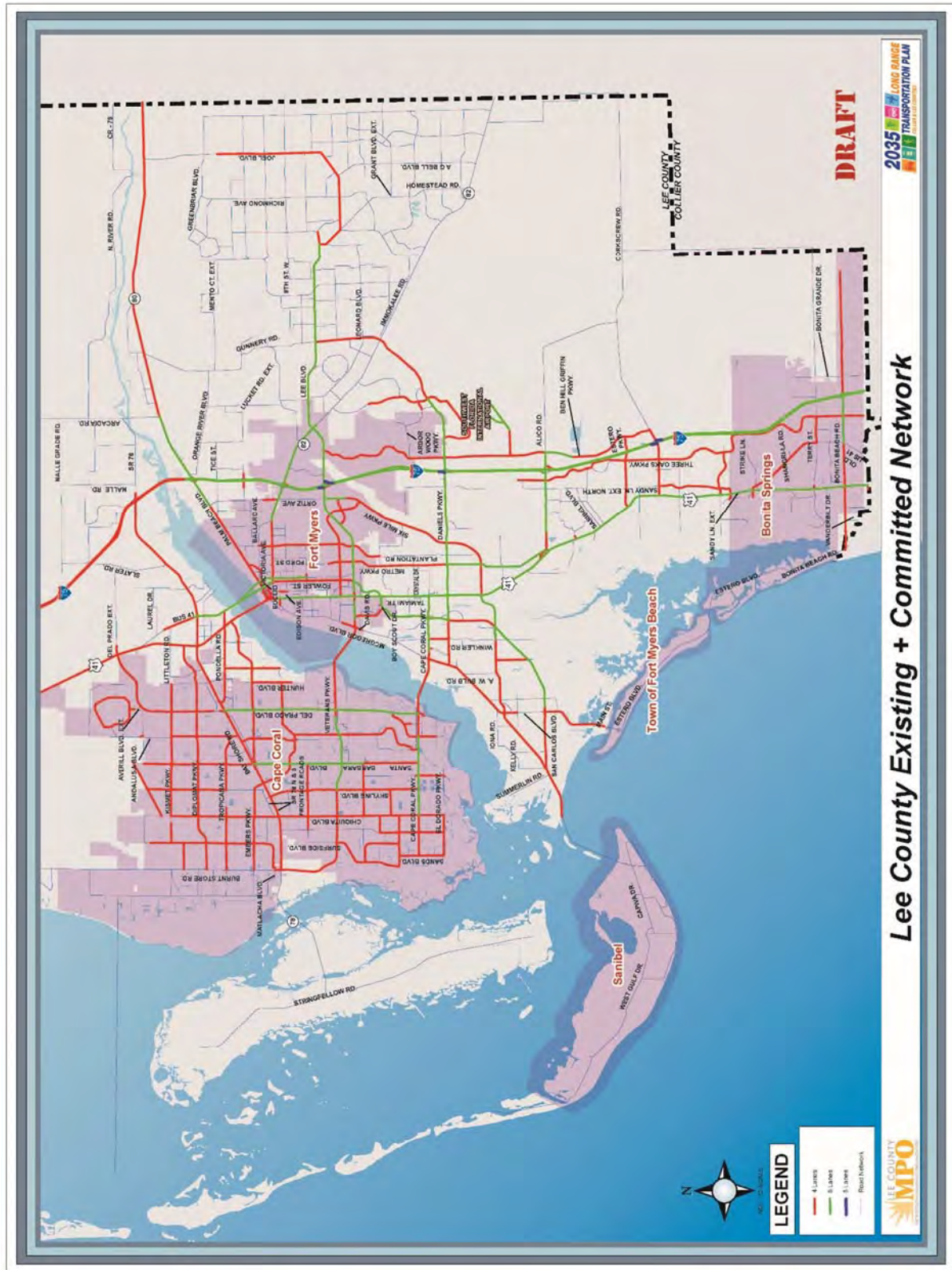
Appendix I: Lee County 2035 LRTP (Excerpts)

(3 Sheets)

DRAFT

Lee County-Roadway Improvements for Needs Plan				
Roadway	Limit From	Limit To	Improvement	
40th St SW Ext	Alabama Blvd	Connie Ave	New 2 lane Rd	
Alabama Rd	Milwaukee Blvd	40th St	2 to 4 lanes	
Alco Rd/Green Meadows	W of Tree Line Ave	SR 82	New 4 lane Rd	
Andalusia Blvd	Jacaranda Pkwy	Kismet Pkwy	New 4 lane Rd	
Andalusia Blvd	Pine Island Rd	Tropicana Pkwy	4 to 6 lanes	
Arborwood Pkwy	SR 82	Tree Line Ave	New 4 lane Rd	
Ben Hill Griffin Pkwy	FGCU Blvd	S of Alco Rd	4 to 6 lanes	
Beth Stacey Blvd	23rd St SW	Alabama Rd	New 2 lane Rd	
Bonita Beach Rd	Vanderbilt Dr	Imperial Pkwy	4 to 6 lanes	
Bonita Beach Rd	E of I-75	Bonita Grande Dr	4 to 6 lanes	
Bonita Grande Dr	Bonita Beach Rd	Terry St	2 to 4 lanes	
Boy Scout Dr	Tamiami Trail	Summerlin Rd	4 to 6 lanes	
Buckingham Rd	Gunnyer Rd	SR 80	2 to 4 lanes	
Burnt Store Rd	Pine Island Rd	N Lee County Line	2 to 4 lanes	
Bus US 41/Tamiami Trail N	Littleton Rd	Tamiami Trail	2 to 4 lanes	
I-75 CD Connector	E of I-75 (Alico and Daniels Pkwy)		New 2 lane Rd	
I-75 CD Connector	W of I-75		New 2 lane Rd	
Chiquita Blvd	Cape Coral Pkwy	Pine Island Rd	4 to 6 lanes	
Corkscrew Rd	Three Oaks Pkwy	Ben Hill Griffin Pkwy	4 to 6 lanes	
CR 951 Ext	Corkscrew Rd	Alco Rd	New 4 lane Rd	
Crystal Dr	Tamiami Trail	Plantation Rd	2 to 4 lanes	
Crystal Dr	Plantation	Six Mile Cypress Pkwy	New 2 lane Rd	
Del Prado Ext / Interchange I-75	US 41	I-75	New 4 lane Rd	
Durden Pkwy	Garden Blvd	US 41	New 2 lane Rd	
Edison Ave	US 41	Fowler St	4 to 6 lanes	
EW Road Access Rd	Treeline Rd	Alico Rd	New 2 lane Rd	
Forum Blvd	SR 82	Luckett Rd	New 2 lane Rd	
Garden Blvd	N of Del Navarre Pkwy	NE 42nd Ter	2 to 4 lanes	
Grant Blvd Ext	Homestead Rd S	Caywood St	New 2 lane Rd	
Gunnyer Rd	Lee Blvd	Buckingham Rd	2 to 4 lanes	
NE 24th Avenue	Pine Island Rd	Del Prado Blvd	2 to 4 lanes	
NE 24th Avenue	Pondella Rd	Pine Island Rd	2 to 4 lanes	
Hanson St	Tamiami Trail	Veronica S Shoemaker Blvd	2 to 4 lanes	
Hanson St	Veronica S Shoemaker Blvd	SR 82	New 4 lane Rd	
Hanson St	SR 82	Buckingham Rd	New 2 lane Rd	
Homestead Rd	Parkdale Blvd	Leeland Heights Blvd	2 to 4 lanes	
I-75	SR 82	N Lee County Line	4 to 8 lanes	
I-75	S Lee County Line	SR 82	6 to 10 lanes	
Imperial Pkwy	S Lee County Line	Bonita Beach Rd	4 to 6 lanes	
Jacaranda Pkwy	Old Burnt Store Rd	Burnt Store Rd	New 2 lane Rd	
Joel Blvd	SR 80	4th St	2 to 4 lanes	
Kismet Pkwy	Burnt Store Rd	El Dorado Blvd	New 4 lane Rd	
Littleton Rd	Hancock Creek Blvd	N Tamiami Trail	2 to 4 lanes	
Littleton Rd Ext	N Tamiami Trail	SR 78	New 2 lane Rd	
Luckett Rd	Sunshine Blvd	12th St W	2 to 4 lanes	
Luckett Rd	Ortiz Ave	E of I-75	2 to 4 lanes	
Luckett Rd	Forum Blvd	Buckingham Rd	New 6 lanes	
Luckett Rd	Buckingham Rd	Gunnyer Rd	New 8 lanes	
Luckett Rd	Gunnyer Rd	Sunshine Blvd	New 6 lanes	
McGregor Blvd	A & W Bulb Rd	Cypress Lake Dr	4 to 6 lanes	
Stat Street	Connection and Bridge	Sunshine Blvd	Road connections	
Metro Pkwy	Daniels Pkwy	Crystal Dr	2 to 6 lanes	
Metro Pkwy	Crystal Dr	SR 82	4 to 6 lanes	
Old US 41	S Lee County Line	Bonita Beach Rd	2 to 4 lanes	
Orange River Blvd	Neal Rd	Buckingham Rd	2 to 4 lanes	
Ortiz Rd Ext	N of Alico Rd	Three Oaks Pkwy	New 2 lanes	
Ortiz Ave	SR 82	SR 80	2 to 4 lanes	
Ortiz Ave	Hanson St	SR 82	2 to 6 lanes	
Ortiz Ave	Colonial Blvd	Hanson St	2 to 4 lanes	
Pine Island Rd	Burnt Store Rd	Chiquita Blvd	2 to 4 lanes	
Pine Island Rd	Skyline Blvd	Pondella Rd	4 to 6 lanes	
Pine Island Rd	Delphine Dr	Hancock Creek Blvd	2 to 4 lanes	
Pine Island Rd	Pondella Rd	US 41	4 to 6 lanes	
Plantation Rd	Six Mile Cypress Pkwy	Idle wild St	2 to 4 lanes	
Rock Hill Rd	Hanson St	Canal St	New 2 lane Rd	
San Carlos Blvd	Summerlin Rd	Kelly Rd	2 to 4 lanes	
Sandy Ln Ext	Strike Ln	Cocanut Rd	New 4 lane Rd	
Sandy Ln Ext North	Corkscrew Rd	Estero Pkwy	New 2 lane Rd	
Shangri-La Rd	Old US 41	Terry St	New 2 lane Rd	
Six Mile Cypress Pkwy	Tamiami Trail	Metro Pkwy	4 to 6 lanes	
SR 31	SR 80	N Lee County Line	2 to 4 lanes	
SR 78	I-75	SR 31	2 to 4 lanes	
SR 80	SR 31	Buckingham Rd	4 to 6 lanes	
SR 82 / Immokalee Rd	Lee Blvd	E Lee County Line	2 to 6 lanes	
Strike Ln	Old US 41	Three Oaks Pkwy	New 2 lane Rd	
Summerlin Rd	Gladolus Dr	Cypress Lake Dr	4 to 6 lanes	
Sunrise Blvd Ext	Alabama Rd	Homestead Rd S	New 2 lane Rd	
Sunrise Blvd Ext	Richmond Ave	Victoria Ave S	New 2 lane Rd	
Sunshine Blvd	SR 82	Luckett Rd	2 to 4 lanes	
Surfside Blvd	Trafalgar Pkwy	Pine Island Rd	New 4 lane Rd	
Tamiami Trail	Pondella Rd	Diplomat	4 to 6 lanes	
Tamiami Trail	Victoria Ave	Pondella Rd	4 to 6 lanes	
Terry St	W of Imperial Pkwy	Bonita Grande Dr	2 to 4 lanes	
Treeline Rd	Daniels Pkwy	Alico Rd	4 to 6 lanes	
Treeline Rd Ext	Colonial Blvd	SR 82	New 2 lane Rd	
Three Oaks Pkwy	Coconut Rd	Estero Pkwy	4 to 6 lanes	
Three Oaks Pkwy Ext	Alico Rd	Daniels Pkwy	New 4 lane Rd	
Three Oaks Pkwy Ext / Renaissance	Daniels Pkwy	Penzance Blvd	2 to 4 lanes	
Todd Ave Ext	SR 82	Meadow Rd	New 2 lane Rd	
Tropicana Pkwy	Chiquita Blvd	Nelson Rd	2 to 4 lanes	
Veronica S Shoemaker Blvd	Michigan Ave	SR 80	2 to 4 lanes	
Veterans Pkwy	Chiquita Blvd	Skyline Blvd	4 to 6 lanes	
Veterans Pkwy	Toil Plaza	McGregor Blvd	4 to 6 lanes	
Victoria Ave	Bonachway	Central Ave	2 to 4 lanes	
Interchange / Critical Needs Int	Corkscrew Rd and I-75		Interchange Improvements	
Interchange / Critical Needs Int	Bonita Beach Rd and US 41		Interchange Improvements	
Interchange / Critical Needs Int	Bonita Beach Rd and I-75		Interchange Improvements	
Interchange / Critical Needs Int	Daniels Pkwy and I-75		Interchange Improvements	
Interchange / Critical Needs Int	US 41 and Six Mile Cypress Pkwy		Interchange Improvements	
Interchange / Critical Needs Int	Metro Pkwy and Six Mile Cypress Pkwy		Interchange Improvements	
Interchange / Critical Needs Int	Daniels Pkwy and US 41		Interchange Improvements	
Interchange / Critical Needs Int	Daniels Pkwy and Metro Pkwy		Interchange Improvements	
Interchange / Critical Needs Int	Daniels Pkwy and Plantation Rd		Interchange Improvements	
Interchange / Critical Needs Int	Daniels Pkwy and Palomino		Interchange Improvements	
Interchange / Critical Needs Int	Colonial Blvd and I-75		Interchange Improvements	
Interchange / Critical Needs Int	SR 82 and I-75		Interchange Improvements	
Interchange / Critical Needs Int	Luckett Rd and I-75		Interchange Improvements	
Interchange / Critical Needs Int	SR 78 and I-75		Interchange Improvements	
Critical Needs Int	Metro Pkwy and Colonial Blvd		TBD	
Critical Needs Int	Summerlin Road and Colonial Blvd		TBD	
Interchange / Critical Needs Int	Del Prado Blvd and Pine Island Rd		Interchange Improvements	
Interchange / Critical Needs Int	Veterans Pkwy and Skyline Blvd		Interchange Improvements	
Interchange / Critical Needs Int	Veterans Pkwy and Santa Barbara Blvd		Interchange Improvements	
Interchange / Critical Needs Int	Veterans Pkwy and Country Club Blvd		Interchange Improvements	
Interchange / Critical Needs Int	Cape Coral Pkwy and Del Prado Blvd		Interchange Improvements	
Estero Boulevard Improvements	San Carlos Boulevard	Big Carlos Pass	Transit/Bike/Ped	
Transportation Enhancement Box	Countywide		Bike/Ped/Transit/CMP	
Computerized Traffic Signal Project	Countywide		Complete Phases	
Intermodal Freight Terminal	Rail/Truck Terminal			
Rail Improvements	Collier County Line	Charlotte County Line	Track/Spur/ROW	
RIS Bridges	Cape Coral Midpoint, Caloosahatchee and Edison Bridges		Phase II	
I-75 Approach Roads and Detours ITS	Collier County Line	Lee County Line	Dynamic and Message Signs	
TD Operations	TD Service Improvements			
Lee County TOC Operations	Countywide		Traffic Signals	





Appendix J: Potential Development – ITE Period Analysis Reports

(6 Sheets)

ID #	STRAP #	PARCEL NAME	DEVELOPMENT PARAMETERS	ZONING APPROVALS NOT YET CONSTRUCTED	POTENTIAL FUTURE CONSTRUCTION PERMITTING
1	07-47-25-B2-00004.00CE	Pelican Landing Community Association	Marina – 20 berths; Restaurant – 3,000sf		X
2A	07-47-25-B2-00003.0370	Estero Bay Marine LLC (Weeks Fish Camp)	Residential – Multi Family – 360 dwelling units – high-rise buildings		X
2B	07-47-25-B2-00003.0000				
3	07-47-25-B2-00000.0010	WCI/Pelican Landing DRI (Raptor Bay)	Residential – Multi Family – 360 dwelling units – high-rise buildings		X
4	06-47-25-00-00002.0030	Pelican Landing Timeshare Ventures LP	Timeshare – Rental Townhouses – 267 dwelling units	X	
5	17-47-25-B1-00001.012A	WCI Communities Inc (Altaira High Rise)	High-Rise Residential Condominium/ Townhouse – 76 dwelling units	X	
6	17-47-25-B1-U1681.1891	WCI Communities Inc (Two Future High Rises)	Residential – Multi Family – 150 dwelling units – high-rise buildings	X	
7	08-47-25-01 +	Eldorado Acres Subdivision	Residential – Single Family – 98 dwelling units – platted lots	X	
8	08-47-25-00-00003.0030	John T. Watson	Residential – Single Family – 2 dwelling units	X	
9	08-47-25-01-00016.0000	Judy K. Doyle	Residential – Single Family – 15 dwelling units	X	
10	08-47-25-00-00004.0000	Dhaliwal + J/T	Residential – Single Family – 3 dwelling units	X	
11	08-47-25-E2-U1757.2005	Dhaliwal + J/T		X	
12	09-47-25-E1-U1823.2024	Dewane/Docese of Venice	Residential – Multi Family – 93 dwelling units		X
13	09-47-25-E4-U1882.1994	Coconut Road Associates LLC	General Office Building – 122,484sf	X	
14	09-47-25-E1-U1874.2023	HG Coconut LLC	Shopping Center – 210,000sf Medical Office Building – 40,000sf Multi-Family Residential – 525 dwelling units Assisted Living – 152 beds Hotel – 130 Rooms	X	
15A	09-47-25-E1-U1877.2039	OBE Florida CRE Holdings LLC			
15B	09-47-25-E2-U1900.2033				
15C	09-47-25-E2-U1902.2012				
16	09-47-25-E3-31000.0050	Allsee Investment LP	General Office Building – 27,500sf	X	
17	09-47-25-E3-31000.0010	Naples Diagnostic Imaging	General Office Building – 15,000sf	X	
22A	09-47-25-E3-373A1.0000	Lee Memorial Health System	Acute Care Hospital – 160 beds Shopping Center – 60,000sf Medical Office Building – 198,000sf General Office Building – 102,000sf	X	
22B	09-47-25-E3-373A2.0000				
22C	09-47-25-E3-373A3.0010				

Project Name: Coconut Road - 1
 Date: 3/10/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 9th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
420 - Marina	20 ⁽¹⁾	30	29	1	1	2	2
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		30	29	1	1	2	2
932 - High-Turnover (Sit-Down) Restaurant	3 ⁽²⁾	191	190	18	12	18	12
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		191	190	18	12	18	12
Total		221	219	19	13	20	14
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		221	219	19	13	20	14

(1) Booth
 (2) 1000 Sq Feet Gross Floor Area

Project Name: Coconut - 2
 Date: 3/10/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 9th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condominium/Townhouse	360 ⁽¹⁾	980	980	24	120	115	57
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		980	980	24	120	115	57
Total		980	980	24	120	115	57
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		980	980	24	120	115	57

(1) Dwelling Units

Project Name: Coconut - 3
 Date: 3/10/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 9th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condominium/Townhouse	360 ⁽¹⁾	980	980	24	120	115	57
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		980	980	24	120	115	57
Total		980	980	24	120	115	57
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		980	980	24	120	115	57

(1) Dwelling Units

Project Name: Coconut - 4		No.:	
Date: 3/10/2016		City:	
State/Province:		Zip/Postal Code:	
Country:		Client Name:	
Analyst's Name:		Edition: ITE-TGM 9th Edition	

LAND USE	SIZE	AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit
224 - Rental Townhouse	267 ⁽¹⁾	62	125	98	94
Reduction		0	0	0	0
Internal		0	0	0	0
Pass-by		0	0	0	0
Non-pass-by		62	125	98	94
Total		62	125	98	94
Total Reduction		0	0	0	0
Total Internal		0	0	0	0
Total Pass-by		0	0	0	0
Total Non-pass-by		62	125	98	94

(1) Dwelling Units

Project Name: Coconut - 5		No.:	
Date: 3/10/2016		City:	
State/Province:		Zip/Postal Code:	
Country:		Client Name:	
Analyst's Name:		Edition: ITE-TGM 9th Edition	

LAND USE	SIZE	DAILY	AM PEAK HOUR		PM PEAK HOUR		
			Entry	Exit	Entry	Exit	
232 - High-Rise Residential Condominium/Townhouse	76 ⁽¹⁾	159	159	5	21	18	11
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		159	159	5	21	18	11
Total		159	159	5	21	18	11
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		159	159	5	21	18	11

(1) Dwelling Units

Project Name: Coconut - 6		No.:	
Date: 3/10/2016		City:	
State/Province:		Zip/Postal Code:	
Country:		Client Name:	
Analyst's Name:		Edition: ITE-TGM 9th Edition	

LAND USE	SIZE	DAILY	AM PEAK HOUR		PM PEAK HOUR		
			Entry	Exit	Entry	Exit	
230 - Residential Condominium/Townhouse	150 ⁽¹⁾	458	457	56	28	56	28
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		458	457	56	28	56	28
Total		458	457	56	28	56	28
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		458	457	56	28	56	28

(1) Dwelling Units

Project Name: Coconut - 7
 Date: 3/11/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 8th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	98 ⁽¹⁾	516	515	20	58	21	60
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		516	515	20	58	21	60
Total		516	515	20	58	21	60
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		516	515	20	58	21	60

(1) Dwelling Units

Project Name: Coconut - 8
 Date: 3/11/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 8th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	2 ⁽¹⁾	10	9	1	1	1	1
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		10	9	1	1	1	1
Total		10	9	1	1	1	1
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		10	9	1	1	1	1

(1) Dwelling Units

Project Name: Coconut - B
 Date: 3/11/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 8th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	15 ⁽¹⁾	92	91	5	15	12	7
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		92	91	5	15	12	7
Total		92	91	5	15	12	7
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		92	91	5	15	12	7

(1) Dwelling Units

Project Name: Coconut - 10 & 11
 Date: 3/11/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 9th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	3 ⁽¹⁾	15	14	1	1	2	1
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		15	14	1	1	2	1
Total		15	14	1	1	2	1
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		15	14	1	1	2	1

(1) Dwelling Units

Project Name: Coconut - 12
 Date: 3/11/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 9th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condominium/Townhouse	93 ⁽¹⁾	302	302	8	41	38	19
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		302	302	8	41	38	19
Total		302	302	8	41	38	19
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		302	302	8	41	38	19

(1) Dwelling Units

Project Name: Coconut - 13
 Date: 3/11/2016
 State/Province:
 Country:
 Analyst's Name:
 No:
 City:
 Zip/Postal Code:
 Client Name:
 Edition: ITE-TGM 9th Edition

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	122,48 ⁽¹⁾	766	766	198	27	37	179
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		766	766	198	27	37	179
Total		766	766	198	27	37	179
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		766	766	198	27	37	179

(1) 1000 Sq. Feet Gross Floor Area

Project Name: Coconut - 16		No:		City:		ITE-TGM 9th Edition	
Date: 3/11/2016		Zip/Postal Code:		Client Name:		Edition:	
State/Province:		Country:		Analyst's Name:			
Analyst's Name:							

LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	27.5 ⁽¹⁾	246	246	60	8	19	90
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		246	246	60	8	19	90
Total		246	246	60	8	19	90
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		246	246	60	8	19	90

(1) 1000 Sq Feet Gross Floor Area

Project Name: Coconut - 17		No:		City:		ITE-TGM 9th Edition	
Date: 3/11/2016		Zip/Postal Code:		Client Name:		Edition:	
State/Province:		Country:		Analyst's Name:			
Analyst's Name:							


LAND USE	SIZE	DAILY		AM PEAK HOUR		PM PEAK HOUR	
		Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	15 ⁽¹⁾	155	155	37	5	16	79
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		155	155	37	5	16	79
Total		155	155	37	5	16	79
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		155	155	37	5	16	79


(1) 1000 Sq Feet Gross Floor Area

Appendix K: FDOT Traffic Online – T24 Factors

(1 Sheet)

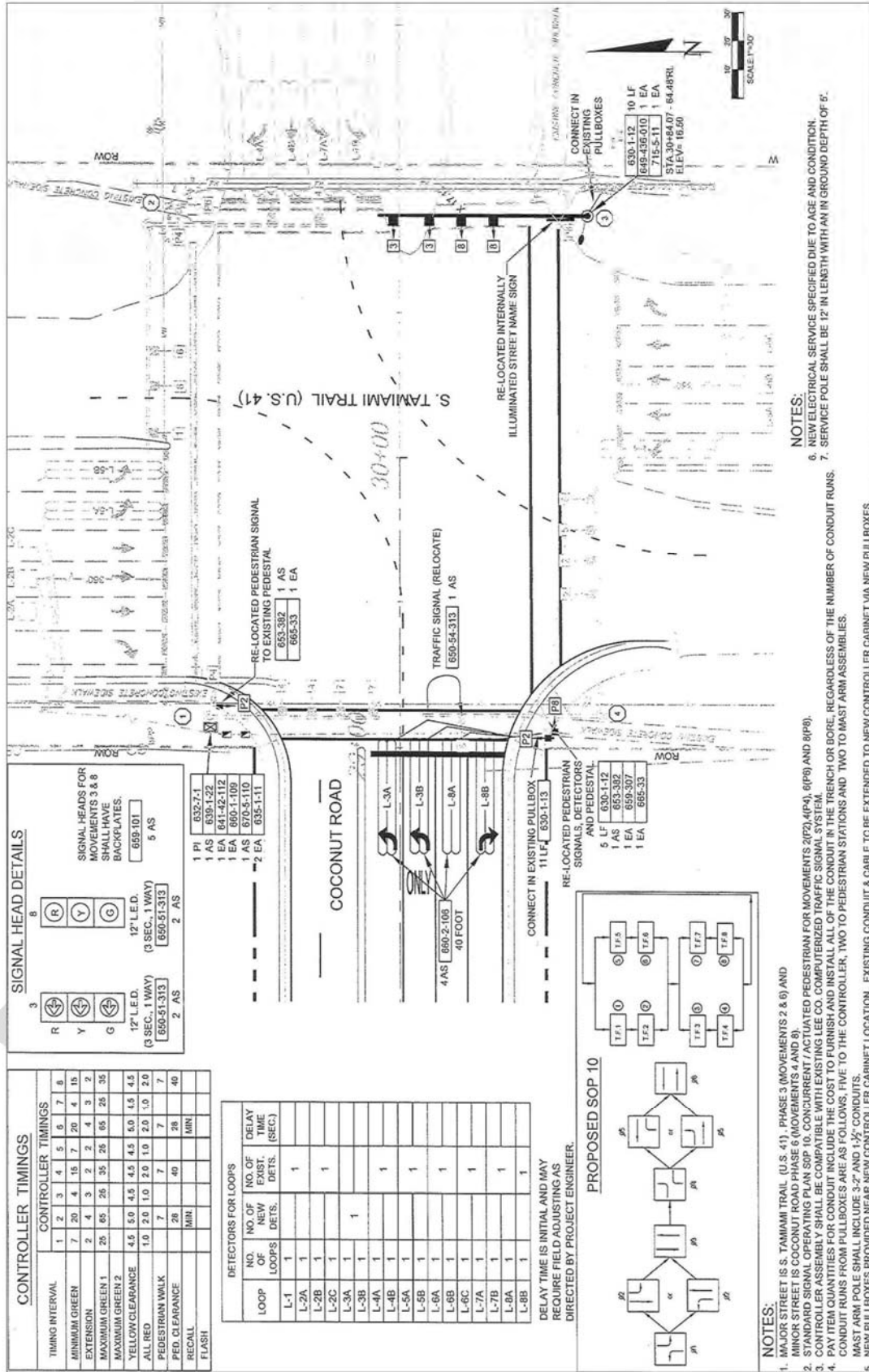
DRAFT

Site Information	
Feature	1
Site	124490
Description	COCONUT RD, E OF SR 45/US 41 LC 490
Section	12000119
Milepoint	0.04
AADT	11100
Site Type	Portable
Class Data	Yes
K Factor	10.3
D Factor	52
T Factor	3.1
TRAFFIC REPORTS (provided in  format)	
Lee County	Annual Average Daily Traffic
	Annual Vehicle Classification
	Historical AADT Data
	Synopsis 124490CL-20140611
	Vehicle Class History

Site Information	
Feature	1
Site	120065
Description	SR 45/US 41, NORTH OF CR 887/OLD US 41 LC436
Section	12010000
Milepoint	4.45
AADT	48000
Site Type	Portable
Class Data	Yes
K Factor	10.05
D Factor	54.6
T Factor	3.1
TRAFFIC REPORTS (provided in  format)	
Lee County	Annual Average Daily Traffic
	Annual Vehicle Classification
	Historical AADT Data
	Synopsis 120065CL-20140402
	Vehicle Class History

Appendix L: Lee County – Intersection Signal Data (10 Sheets)

DRAFT



Lee County, FL

409 - US 41 & Coconut Rd - - Econolite Type - ASC3

Coordination Pattern Data
Pattern Data (MM)3-2

Pattern - 33

Split Pattern	33	TS2 (Pat-Off)	10-3	Splits in	Percent
Cycle	130	Std (COS)	333	Offsets in	Percent
Offset Value	43%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 33)	12	50	15	23	12	50	19	19	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data

Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 34

Split Pattern	34	TS2 (Pat-Off)	11-1	Splits in	Percent
Cycle	130	Std (COS)	344	Offsets in	Percent
Offset Value	35%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 34)	12	54	13	21	12	54	17	17	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 36

Split Pattern	36	TS2 (Pat-Off)	11-3	Splits in	Percent
Cycle	130	Std (COS)	312	Offsets in	Percent
Offset Value	30%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 36)	12	50	14	24	12	50	20	18	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 37

Split Pattern	37	TS2 (Pat-Off)	12-1	Splits in	Percent
Cycle	130	Std (COS)	313	Offsets in	Percent
Offset Value	49%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 37)	15	52	12	21	12	55	15	18	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 41

Split Pattern	41	TS2 (Pat-Off)	13-2	Splits in	Percent
Cycle	150	Std (COS)	411	Offsets in	Percent
Offset Value	38%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 41)	15	43	18	24	15	43	24	18	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 42

Split Pattern	42	TS2 (Pat-Off)	13-3	Splits in	Percent
Cycle	150	Std (COS)	422	Offsets in	Percent
Offset Value	60%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 42)	15	42	21	22	16	41	22	21	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 43

Split Pattern	43	TS2 (Pat-Off)	14-1	Splits in	Percent
Cycle	150	Std (COS)	433	Offsets in	Percent
Offset Value	92%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 43)	16	43	16	25	16	43	25	16	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 44

Split Pattern	44	TS2 (Pat-Off)	14-2	Splits in	Percent
Cycle	150	Std (COS)	444	Offsets in	Percent
Offset Value	2%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	2		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 44)	15	45	20	20	12	48	22	18	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 45

Split Pattern	45	TS2 (Pat-Off)	14-3	Splits in	Percent
Cycle	150	Std (COS)	453	Offsets in	Percent
Offset Value	82%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 45)	15	50	18	17	10	55	18	17	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 51

Split Pattern	51	TS2 (Pat-Off)	0-0	Splits in	Percent
Cycle	160	Std (COS)	511	Offsets in	Percent
Offset Value	56%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 51)	17	43	20	20	12	48	20	20	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 53

Split Pattern	53	TS2 (Pat-Off)	0-0	Splits in	Percent
Cycle	200	Std (COS)	151	Offsets in	Percent
Offset Value	13%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 53)	10	50	14	26	8	52	26	14	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 57

Split Pattern	57	TS2 (Pat-Off)	0-0	Splits in	Percent
Cycle	200	Std (COS)	152	Offsets in	Percent
Offset Value	97%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 57)	16	54	15	15	8	62	15	15	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 60

Split Pattern	60	TS2 (Pat-Off)	0-0	Splits in	Percent
Cycle	180	Std (COS)	644	Offsets in	Percent
Offset Value	7%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 60)	16	42	13	29	9	49	23	19	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 61

Split Pattern	61	TS2 (Pat-Off)	0-0	Splits in	Percent
Cycle	180	Std (COS)	611	Offsets in	Percent
Offset Value	21%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 61)	13	49	16	22	13	49	22	16	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Pattern - 62

Split Pattern	62	TS2 (Pat-Off)	0-0	Splits in	Percent
Cycle	180	Std (COS)	622	Offsets in	Percent
Offset Value	24%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	1		
Phase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Splits (Split Pat 62)	13	52	18	17	10	55	19	16	0	0	0	0	0	0	0	0
Preference 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Ring	1	2	3	4
Ring Split Ext	0	0	0	0
Ring Disp.	-	0	0	0
Split Sum	100%	100%	0%	0%

Misc. Data					
Veh. Permissive 1	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Split Demand Pat 1	0	Split Demand Pat 2	0	Crossing Arterial Pat	0

Split Pattern Data

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Coordinated Phases		X				X										
Vehicle Recalls																
Ped Recalls																
Max Recalls		X				X										
Phase Omit									X	X	X	X	X	X	X	X
Special Function Output																

Lee County, FL

409 - US 41 & Coconut Rd - - Econolite Type - ASC3

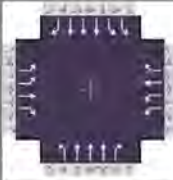
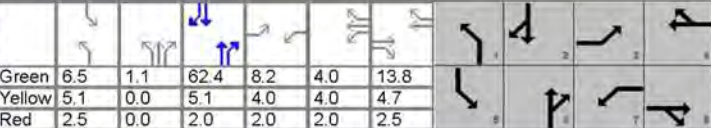
Controller Timing Plan (MM)2-1
Plan 1

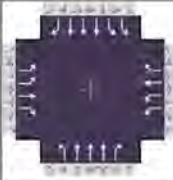
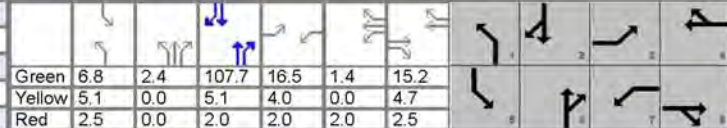
Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Min Green	7	26	7	10	7	26	7	10	0	0	0	0	0	0	0	0
BK Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	7	0	7	0	7	0	7	0	0	0	0	0	0	0	0
Walk 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear	0	26	0	38	0	28	0	40	0	0	0	0	0	0	0	0
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Vehicle Ext	3.0	5.0	3.0	3.0	3.0	5.0	3.0	3.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max 1	25	90	25	25	20	90	30	25	0	0	0	0	0	0	0	0
Max 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Max 3	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
DYM Stp	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.1	5.1	4.0	4.7	5.1	5.1	4.0	4.7	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Red Clear	2.5	2.0	2.0	2.5	2.5	2.0	2.0	2.5	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
ACT B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
SEC/ACT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	30	30	30	30	30	30	30	30	30	30	30	30	30	30	30	30
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Vlt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPT Duc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

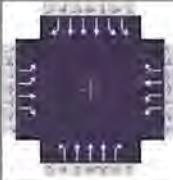
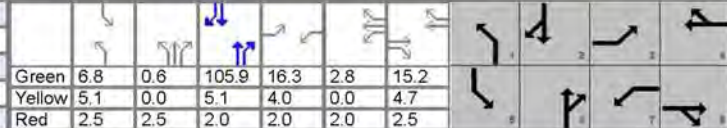
Appendix M: Intersection Analysis - HCS 2010 Printouts

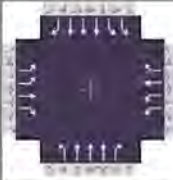
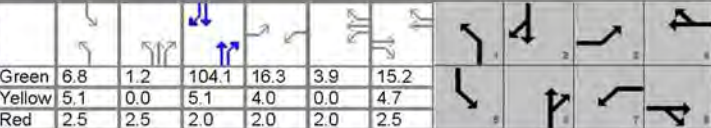
(9 Sheets)

DRAFT

HCS 2010 Signalized Intersection Results Summary																											
General Information						Intersection Information																					
Agency			Analysis Date			Duration, h		Area Type																			
Analyst			Mar 21, 2016			0.25		Other																			
Jurisdiction			Time Period			PHF		0.98																			
Urban Street			Analysis Year			Analysis Period		1> 4.00																			
Coconut Road			2016			2016 AM Pk Hr - 03-22-2016.xus																					
Project Description						2016 AM Pk Hr																					
Demand Information				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h				161	94	141	426	124	59	81	908	188	74	1997	197												
Signal Information																											
Cycle, s		Reference Phase																									
130.0		2																									
Offset, s		Reference Point																									
0		End																									
Uncoordinated		Simult. Gap E/W		Green		Yellow		Red		Green		Yellow		Red													
No		On		6.5	1.1	62.4	8.2	4.0	13.8	5.1	0.0	5.1	4.0	4.0	4.7												
Force Mode		Simult. Gap N/S		2.5	0.0	2.0	2.0	2.0	2.5	2.5	0.0	2.0	2.0	2.0	2.5												
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase				3			8			7			4			1			6			5			2		
Case Number				2.0			3.0			2.0			3.0			2.0			3.0			2.0			3.0		
Phase Duration, s				14.2			21.0			24.3			31.1			15.2			70.6			14.1			69.5		
Change Period, (Y+R _c), s				6.0			7.2			6.0			7.2			7.6			7.1			7.6			7.1		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.1			3.0			0.0			3.0			0.0		
Queue Clearance Time (g _s), s				8.1			13.4			18.1			9.7			8.0						4.8					
Green Extension Time (g _e), s				0.2			0.4			0.1			0.7			0.0			0.0			0.1			0.0		
Phase Call Probability				1.00			1.00			1.00			1.00			0.95						0.93					
Max Out Probability				0.03			0.25			1.00			0.00			0.18						0.00					
Movement Group Results				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12												
Adjusted Flow Rate (v), veh/h				164	96	144	435	127	60	83	927	192	76	2038	201												
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1774	1691	1449	1723	1691	1579												
Queue Service Time (g _s), s				6.1	6.3	11.4	16.1	7.7	4.2	6.0	14.9	10.2	2.8	45.4	9.9												
Cycle Queue Clearance Time (g _c), s				6.1	6.3	11.4	16.1	7.7	4.2	6.0	14.9	10.2	2.8	45.4	9.9												
Green Ratio (g/C)				0.06	0.11	0.11	0.14	0.18	0.18	0.06	0.49	0.49	0.05	0.48	0.48												
Capacity (c), veh/h				218	198	171	484	342	290	104	2477	708	173	2436	758												
Volume-to-Capacity Ratio (X)				0.753	0.484	0.840	0.899	0.370	0.208	0.796	0.374	0.271	0.436	0.837	0.265												
Back of Queue (Q), ft/ln (95 th percentile)				122.9	135.3	225.3	321.7	162.1	73.5	126.5	242.8	154.5	54.6	628.3	163.4												
Back of Queue (Q), veh/ln (95 th percentile)				4.8	5.3	9.0	12.7	6.4	2.9	5.0	9.6	6.2	2.1	24.7	6.5												
Queue Storage Ratio (RQ) (95 th percentile)				0.20	0.00	0.56	0.71	0.00	0.11	0.26	0.00	0.21	0.09	0.00	0.59												
Uniform Delay (d ₁), s/veh				59.9	54.7	57.0	55.0	46.5	45.1	60.4	20.8	19.6	59.9	29.4	20.1												
Incremental Delay (d ₂), s/veh				2.0	0.7	14.4	17.8	0.2	0.1	5.1	0.4	0.9	0.6	3.6	0.9												
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh				61.9	55.4	71.4	72.7	46.7	45.2	65.6	21.3	20.6	60.6	33.0	21.0												
Level of Service (LOS)				E	E	E	E	D	D	E	C	C	E	C	C												
Approach Delay, s/veh / LOS				63.7	E	64.8	E	24.2	C	32.8	C																
Intersection Delay, s/veh / LOS				37.7						D																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS				3.4	C	3.5	C	2.9	C	2.6	B																
Bicycle LOS Score / LOS				1.2	A	1.5	A	1.1	A	1.8	A																
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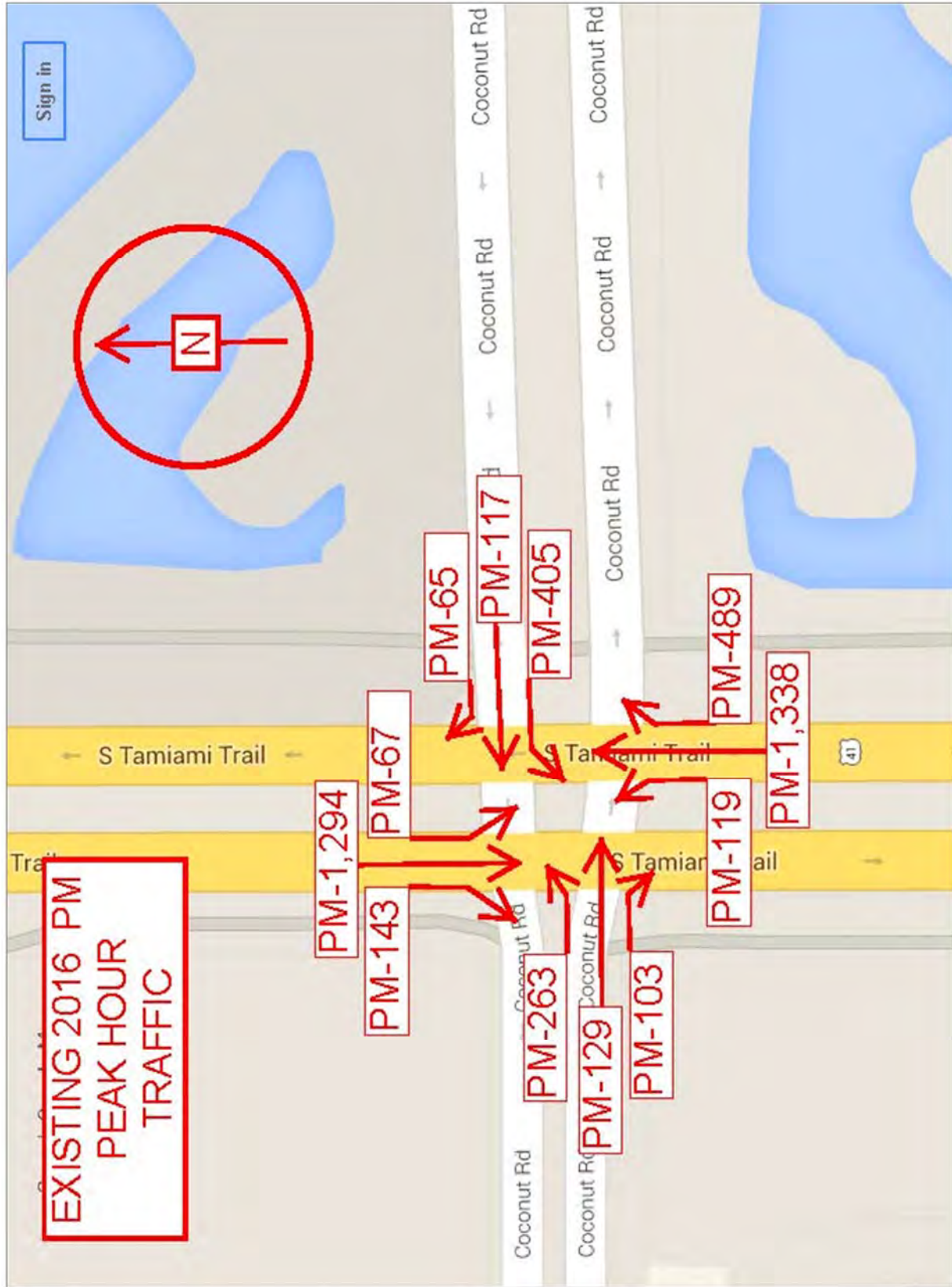
HCS 2010 Signalized Intersection Results Summary																											
General Information						Intersection Information																					
Agency			Analysis Date			Duration, h		Area Type																			
Analyst			Mar 22, 2016			0.25		Other																			
Jurisdiction			Time Period			PHF		0.98																			
Urban Street			Analysis Year			Analysis Period		1> 4.00																			
Coconut Road			2016			2016 PM Pk Hr - 03-22-2016.xus																					
Project Description			2016 PM Pk Hr																								
Demand Information				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h				268	129	103	405	117	65	119	1338	489	67	1294	143												
Signal Information																											
Cycle, s		Reference Phase																									
180.0		2																									
Offset, s		Reference Point																									
0		End																									
Uncoordinated		Simult. Gap E/W																									
No		On																									
Force Mode		Simult. Gap N/S																									
Fixed		On																									
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase				3			8			7			4			1			6			5			2		
Case Number				2.0			3.0			2.0			3.0			2.0			3.0			2.0			3.0		
Phase Duration, s				22.5			22.4			25.9			25.8			16.8			117.3			14.4			114.8		
Change Period, (Y+R _c), s				6.0			7.2			2.0			7.2			2.5			7.1			7.6			7.1		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.1			3.0			0.0			3.0			0.0		
Queue Clearance Time (g _s), s				16.1			14.5			23.3			13.1			14.2						5.5					
Green Extension Time (g _e), s				0.4			0.7			0.7			0.7			0.2			0.0			0.1			0.0		
Phase Call Probability				1.00			1.00			1.00			1.00			1.00						0.97					
Max Out Probability				0.00			0.00			0.01			0.00			0.00						0.00					
Movement Group Results				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12												
Adjusted Flow Rate (v), veh/h				273	132	105	413	119	66	121	1365	499	68	1320	146												
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1774	1691	1449	1723	1691	1579												
Queue Service Time (g _s), s				14.1	12.5	11.5	21.3	11.1	7.1	12.2	25.7	36.7	3.5	25.4	7.4												
Cycle Queue Clearance Time (g _c), s				14.1	12.5	11.5	21.3	11.1	7.1	12.2	25.7	36.7	3.5	25.4	7.4												
Green Ratio (g/C)				0.09	0.08	0.08	0.13	0.10	0.10	0.08	0.61	0.61	0.04	0.60	0.60												
Capacity (c), veh/h				317	157	136	458	193	163	141	3106	887	130	3037	945												
Volume-to-Capacity Ratio (X)				0.864	0.837	0.773	0.902	0.620	0.406	0.861	0.440	0.563	0.528	0.435	0.154												
Back of Queue (Q), ft/ln (95 th percentile)				271.1	261.2	213.3	387.9	231.4	129.8	245.2	381	458	71.7	380.1	121.8												
Back of Queue (Q), veh/ln (95 th percentile)				10.7	10.3	8.5	15.3	9.1	5.2	9.7	15.0	18.3	2.8	15.0	4.9												
Queue Storage Ratio (RQ) (95 th percentile)				0.45	0.00	0.53	0.86	0.00	0.20	0.51	0.00	0.62	0.12	0.00	0.44												
Uniform Delay (d ₁), s/veh				80.6	81.2	80.7	76.9	77.3	75.5	81.9	18.5	20.7	85.0	19.6	16.0												
Incremental Delay (d ₂), s/veh				5.7	4.4	3.5	11.5	1.2	0.6	5.8	0.5	2.6	1.2	0.5	0.3												
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh				86.3	85.6	84.2	88.3	78.5	76.1	87.7	19.0	23.2	86.3	20.1	16.3												
Level of Service (LOS)				F	F	F	F	E	E	F	B	C	F	C	B												
Approach Delay, s/veh / LOS				85.7		F	85.0		F	24.3		C	22.7		C												
Intersection Delay, s/veh / LOS				38.4						D																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS				3.4		C	3.5		D	2.9		C	2.6		B												
Bicycle LOS Score / LOS				1.3		A	1.5		A	1.6		A	1.3		A												
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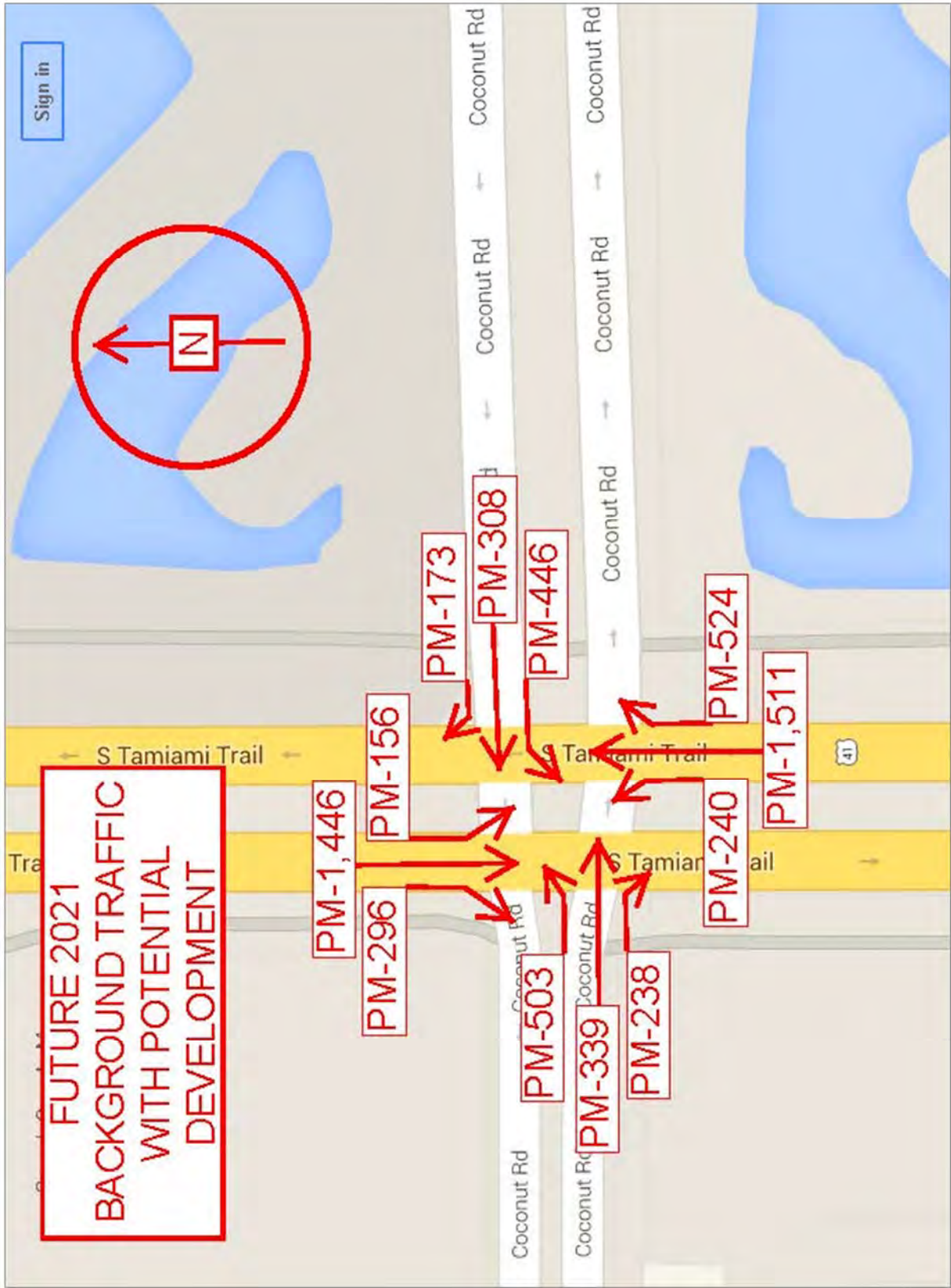
HCS 2010 Signalized Intersection Results Summary																											
General Information						Intersection Information																					
Agency			Analysis Date			Duration, h		Area Type																			
Analyst			Mar 22, 2016			0.25		Other																			
Jurisdiction			Time Period			PHF		0.98																			
Urban Street			Analysis Year			Analysis Period		1> 4.00																			
Intersection			File Name			2021 PM Pk Hr - 03-23-2016.xus																					
Project Description			2021 PM Pk Hr																								
Demand Information				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h				263	129	103	426	123	69	126	1407	514	71	1361	151												
Signal Information																											
Cycle, s		Reference Phase																									
180.0		2																									
Offset, s		Reference Point		End																							
0																											
Uncoordinated		Simult. Gap E/W		On																							
No																											
Force Mode		Simult. Gap N/S		On																							
Fixed																											
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase				3			8			7			4			1			6			5			2		
Case Number				2.0			3.0			2.0			3.0			2.0			3.0			2.0			3.0		
Phase Duration, s				22.3			22.4			27.0			27.2			17.5			116.1			14.4			113.0		
Change Period, (Y+R _c), s				6.0			7.2			2.0			7.2			2.5			7.1			7.6			7.1		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.1			3.0			0.0			3.0			0.0		
Queue Clearance Time (g _s), s				15.8			14.5			24.4			13.6			14.9						5.7					
Green Extension Time (g _e), s				0.4			0.7			0.7			0.7			0.2			0.0			0.1			0.0		
Phase Call Probability				1.00			1.00			1.00			1.00			1.00						0.97					
Max Out Probability				0.00			0.00			0.03			0.00			0.00						0.00					
Movement Group Results				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12												
Adjusted Flow Rate (v), veh/h				268	132	105	435	126	70	129	1436	524	72	1389	154												
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1774	1691	1449	1723	1691	1579												
Queue Service Time (g _s), s				13.8	12.5	11.5	22.4	11.6	7.5	12.9	28.0	40.3	3.7	27.9	8.0												
Cycle Queue Clearance Time (g _c), s				13.8	12.5	11.5	22.4	11.6	7.5	12.9	28.0	40.3	3.7	27.9	8.0												
Green Ratio (g/C)				0.09	0.08	0.08	0.14	0.11	0.11	0.08	0.61	0.61	0.04	0.59	0.59												
Capacity (c), veh/h				311	157	136	479	207	175	148	3074	878	130	2986	929												
Volume-to-Capacity Ratio (X)				0.862	0.836	0.773	0.907	0.607	0.402	0.868	0.467	0.598	0.556	0.465	0.166												
Back of Queue (Q), ft/ln (95 th percentile)				266.3	261.2	213.3	407.5	239.7	136.8	256.7	410.2	498	76.1	412.6	133.3												
Back of Queue (Q), veh/ln (95 th percentile)				10.5	10.3	8.5	16.0	9.4	5.5	10.1	16.1	19.9	3.0	16.2	5.3												
Queue Storage Ratio (RQ) (95 th percentile)				0.44	0.00	0.53	0.91	0.00	0.21	0.53	0.00	0.68	0.13	0.00	0.48												
Uniform Delay (d ₁), s/veh				80.8	81.2	80.7	76.3	76.3	74.4	81.5	19.5	21.9	85.1	21.0	16.9												
Incremental Delay (d ₂), s/veh				5.1	4.4	3.5	13.0	1.1	0.6	5.8	0.5	3.0	1.4	0.5	0.4												
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh				85.8	85.6	84.2	89.3	77.3	75.0	87.3	20.0	24.9	86.5	21.5	17.3												
Level of Service (LOS)				F	F	F	F	E	E	F	C	C	F	C	B												
Approach Delay, s/veh / LOS				85.4		F	85.3		F	25.4		C	24.0		C												
Intersection Delay, s/veh / LOS				39.0						D																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS				3.4		C	3.5		D	2.9		C	2.6		B												
Bicycle LOS Score / LOS				1.3		A	1.5		A	1.6		A	1.4		A												
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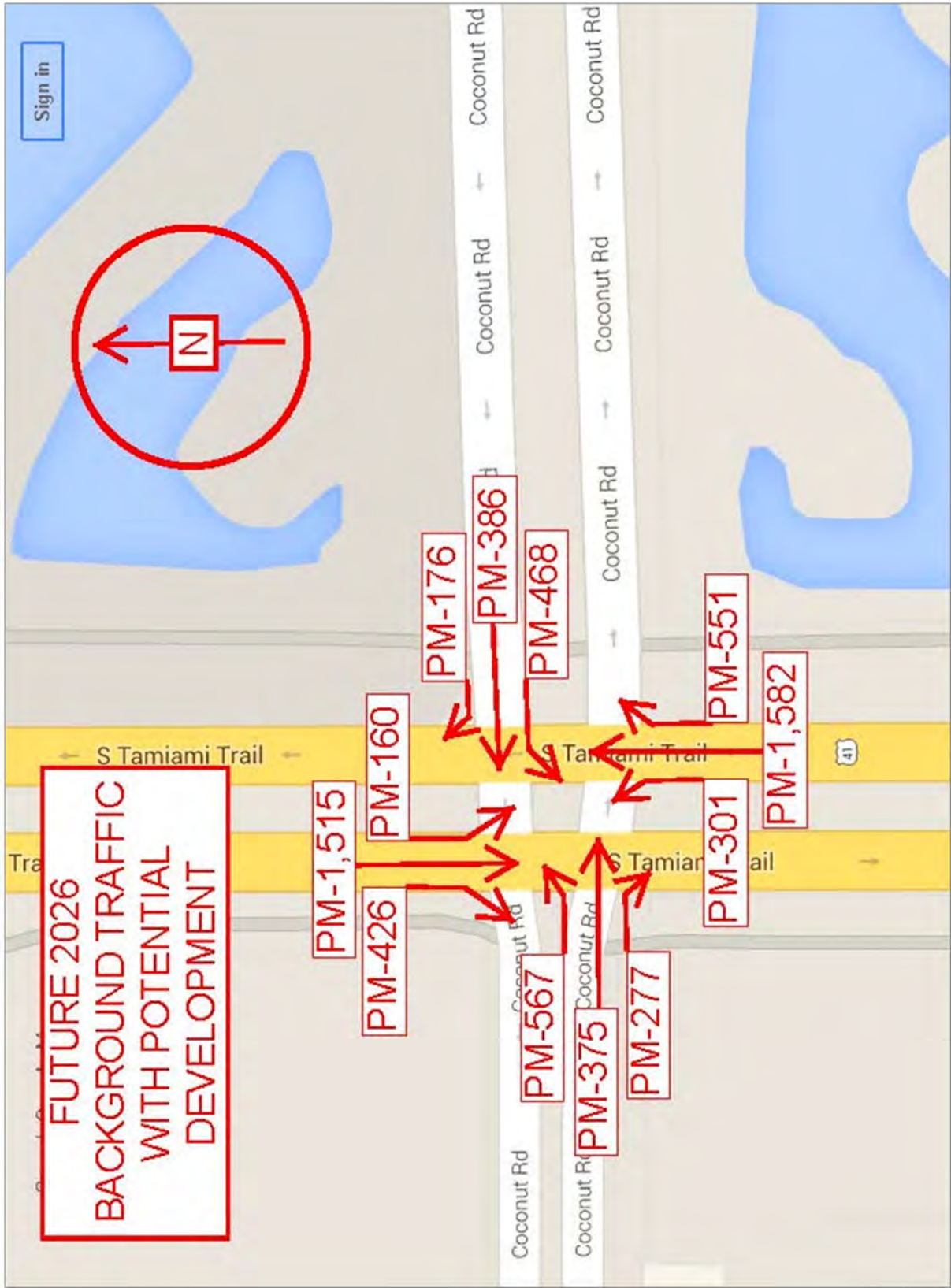
HCS 2010 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency						Duration, h		0.25							
Analyst		CM		Analysis Date		Mar 22, 2016		Area Type					Other		
Jurisdiction						Time Period		PHF					0.98		
Urban Street		US 41 (SR 45)		Analysis Year		2016		Analysis Period					1> 4:00		
Intersection		Coconut Road		File Name		2026 PM Pk Hr - 03-23-2016.xus									
Project Description		2026 PM Pk Hr													
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				263	129	103	448	130	72	132	1478	541	75	1430	158
Signal Information															
Cycle, s		180.0		Reference Phase		2									
Offset, s		0		Reference Point		End									
Uncoordinated		No		Simult. Gap E/W		On		Green		6.8		1.2		104.1	
				Yellow		5.1		0.0		5.1		4.0		0.0	
Force Mode		Fixed		Simult. Gap N/S		On		Red		2.5		2.5		2.0	
Timer Results				EBL			EBT			WBL			WBT		
Assigned Phase				3			8			7			4		
Case Number				2.0			3.0			2.0			3.0		
Phase Duration, s				22.3			22.4			28.2			28.3		
Change Period, (Y+R _c), s				6.0			7.2			2.0			7.2		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.1		
Queue Clearance Time (g _s), s				15.8			14.5			25.5			14.2		
Green Extension Time (g _e), s				0.4			0.7			0.7			0.2		
Phase Call Probability				1.00			1.00			1.00			1.00		
Max Out Probability				0.00			0.00			0.07			0.00		
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow Rate (v), veh/h				268	132	105	457	133	73	135	1508	552	77	1459	161
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1774	1691	1449	1723	1691	1579
Queue Service Time (g _s), s				13.8	12.5	11.5	23.5	12.2	7.8	13.5	30.5	44.4	3.9	30.6	8.6
Cycle Queue Clearance Time (g _c), s				13.8	12.5	11.5	23.5	12.2	7.8	13.5	30.5	44.4	3.9	30.6	8.6
Green Ratio (g/C)				0.09	0.08	0.08	0.15	0.12	0.12	0.09	0.60	0.60	0.04	0.58	0.58
Capacity (c), veh/h				311	157	136	501	219	185	154	3041	868	131	2936	913
Volume-to-Capacity Ratio (X)				0.862	0.836	0.773	0.912	0.607	0.397	0.873	0.496	0.636	0.584	0.497	0.177
Back of Queue (Q), ft/ln (95 th percentile)				266.3	261.2	213.3	428	249.6	141.9	266.2	442.7	544.4	80.5	447.8	144.4
Back of Queue (Q), veh/ln (95 th percentile)				10.5	10.3	8.5	16.8	9.8	5.7	10.5	17.4	21.8	3.2	17.6	5.8
Queue Storage Ratio (RQ) (95 th percentile)				0.44	0.00	0.53	0.95	0.00	0.22	0.55	0.00	0.74	0.14	0.00	0.52
Uniform Delay (d ₁), s/veh				80.8	81.2	80.7	75.8	75.5	73.5	81.2	20.6	23.4	85.2	22.4	17.8
Incremental Delay (d ₂), s/veh				5.1	4.4	3.5	14.5	1.0	0.5	5.8	0.6	3.5	1.5	0.6	0.4
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				85.8	85.6	84.2	90.3	76.5	74.1	87.0	21.2	26.9	86.7	23.0	18.2
Level of Service (LOS)				F	F	F	F	E	E	F	C	C	F	C	B
Approach Delay, s/veh / LOS				85.4		F	85.8		F	26.6		C	25.5		C
Intersection Delay, s/veh / LOS				39.9						D					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				3.4			C			3.5			D		
Bicycle LOS Score / LOS				1.3			A			1.6			A		
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HCS 2010 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency						Duration, h		0.25							
Analyst		CM		Analysis Date		Mar 25, 2016		Area Type					Other		
Jurisdiction						Time Period		PHF					0.98		
Urban Street		US 41 (SR 45)		Analysis Year		2016		Analysis Period					1 > 4:00		
Intersection		Coconut Road		File Name		2021 PM Pk Hr w Developments - 03-25-2016.xus									
Project Description		2021 PM Pk Hr w Developments													
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				503	339	238	446	308	173	240	1511	524	156	1446	296
Signal Information															
Cycle, s		180.0		Reference Phase		2									
Offset, s		0		Reference Point		End									
Uncoordinated		No		Simult. Gap E/W		On									
Force Mode		Fixed		Simult. Gap N/S		On									
Timer Results				EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT				
Assigned Phase				3	8	7	4	1	6	5	2				
Case Number				2.0	3.0	2.0	3.0	2.0	3.0	2.0	3.0				
Phase Duration, s				32.0	33.9	28.1	30.0	24.0	100.2	17.8	94.0				
Change Period, (Y+R _c), s				6.0	7.2	2.0	7.2	2.5	7.1	7.6	7.1				
Max Allow Headway (MAH), s				3.1	3.1	3.0	3.1	3.0	0.0	3.0	0.0				
Queue Clearance Time (g _s), s				28.0	28.7	25.4	24.8	23.5		10.2					
Green Extension Time (g _e), s				0.0	0.0	0.7	0.0	0.0	0.0	0.0	0.0				
Phase Call Probability				1.00	1.00	1.00	1.00	1.00		1.00					
Max Out Probability				1.00	1.00	0.07	1.00	1.00		1.00					
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow Rate (v), veh/h				513	346	243	455	314	177	245	1542	535	159	1476	302
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1774	1691	1449	1723	1691	1579
Queue Service Time (g _s), s				26.0	26.7	26.7	23.4	22.8	19.8	21.5	37.9	50.8	8.2	38.2	22.0
Cycle Queue Clearance Time (g _c), s				26.0	26.7	26.7	23.4	22.8	19.8	21.5	37.9	50.8	8.2	38.2	22.0
Green Ratio (g/C)				0.14	0.15	0.15	0.14	0.13	0.13	0.12	0.52	0.52	0.06	0.48	0.48
Capacity (c), veh/h				498	276	239	499	236	200	212	2624	749	196	2450	762
Volume-to-Capacity Ratio (X)				1.031	1.251	1.016	0.912	1.332	0.883	1.156	0.588	0.714	0.813	0.602	0.396
Back of Queue (Q), ft/ln (95 th percentile)				545	881.7	542.9	426.1	858.4	373.1	615	550.6	637.1	190.8	560.1	334.4
Back of Queue (Q), veh/ln (95 th percentile)				21.5	34.7	21.7	16.8	33.8	14.9	24.2	21.7	25.5	7.5	22.1	13.4
Queue Storage Ratio (RQ) (95 th percentile)				0.90	0.00	1.34	0.95	0.00	0.57	1.27	0.00	0.87	0.32	0.00	1.21
Uniform Delay (d ₁), s/veh				77.0	76.6	76.6	75.8	78.6	77.3	79.3	30.2	33.3	83.9	33.9	29.8
Incremental Delay (d ₂), s/veh				48.6	139.3	62.4	14.4	175.5	32.8	110.2	1.0	5.7	20.4	1.1	1.5
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				125.6	215.9	139.1	90.2	254.1	110.1	189.4	31.1	39.0	104.3	35.1	31.3
Level of Service (LOS)				F	F	F	F	F	F	F	C	D	F	D	C
Approach Delay, s/veh / LOS				156.9	F	F	148.4	F	F	49.6	D	D	40.2	D	D
Intersection Delay, s/veh / LOS				80.3						F					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				3.4	C	C	3.5	D	D	2.9	C	C	2.6	B	B
Bicycle LOS Score / LOS				2.3	B	B	2.0	B	B	1.8	A	A	1.6	A	A
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HCS 2010 Signalized Intersection Results Summary															
General Information						Intersection Information									
Agency						Duration, h		0.25							
Analyst		CM		Analysis Date		Mar 25, 2016		Area Type					Other		
Jurisdiction						Time Period		PHF					0.98		
Urban Street		US 41 (SR 45)		Analysis Year		2016		Analysis Period					1> 4:00		
Intersection		Coconut Road		File Name		2026 PM Pk Hr w Developments - 03-25-2016.xus									
Project Description		2026 PM Pk Hr w Developments													
Demand Information				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h				567	375	277	468	386	176	301	1582	551	160	1515	426
Signal Information															
Cycle, s		180.0		Reference Phase		2									
Offset, s		0		Reference Point		End									
Uncoordinated		No		Simult. Gap E/W		On		Green		10.4		3.5		86.9	
				Yellow		5.1		0.0		5.1		0.0		0.0	
Force Mode		Fixed		Simult. Gap N/S		On		Red		2.5		2.5		2.0	
Timer Results				EBL			EBT			WBL			WBT		
Assigned Phase				3			8			7			4		
Case Number				2.0			3.0			2.0			3.0		
Phase Duration, s				32.0			32.8			29.2			30.0		
Change Period, (Y+R _c), s				6.0			7.2			2.0			7.2		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.1		
Queue Clearance Time (g _s), s				28.0			27.6			26.6			24.8		
Green Extension Time (g _e), s				0.0			0.0			0.6			0.0		
Phase Call Probability				1.00			1.00			1.00			1.00		
Max Out Probability				1.00			1.00			0.17			1.00		
Movement Group Results				EB			WB			NB			SB		
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow Rate (v), veh/h				579	383	283	478	394	180	307	1614	562	163	1546	435
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1774	1691	1449	1723	1691	1579
Queue Service Time (g _s), s				26.0	25.6	25.6	24.6	22.8	20.2	21.5	40.6	55.2	8.4	40.8	35.4
Cycle Queue Clearance Time (g _c), s				26.0	25.6	25.6	24.6	22.8	20.2	21.5	40.6	55.2	8.4	40.8	35.4
Green Ratio (g/C)				0.14	0.14	0.14	0.15	0.13	0.13	0.12	0.52	0.52	0.06	0.48	0.48
Capacity (c), veh/h				498	265	229	521	236	200	212	2619	748	199	2450	762
Volume-to-Capacity Ratio (X)				1.163	1.445	1.235	0.917	1.669	0.898	1.449	0.616	0.752	0.820	0.631	0.570
Back of Queue (Q), ft/ln (95 th percentile)				676.5	1094.9	726.5	446.7	1244.8	383.8	901	584.6	688.3	196	593.1	500.3
Back of Queue (Q), veh/ln (95 th percentile)				26.6	43.1	29.1	17.6	49.0	15.4	35.5	23.0	27.5	7.7	23.4	20.0
Queue Storage Ratio (RQ) (95 th percentile)				1.12	0.00	1.79	0.99	0.00	0.59	1.86	0.00	0.94	0.33	0.00	1.82
Uniform Delay (d ₁), s/veh				77.0	77.2	77.2	75.3	78.6	77.5	79.3	30.9	34.4	83.9	34.6	33.2
Incremental Delay (d ₂), s/veh				93.4	220.2	137.3	15.9	319.2	36.1	226.7	1.1	6.9	21.8	1.2	3.1
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/veh				170.4	297.4	214.6	91.2	397.8	113.5	306.0	32.0	41.3	105.6	35.9	36.3
Level of Service (LOS)				F	F	F	F	F	F	F	C	D	F	D	D
Approach Delay, s/veh / LOS				219.5	F		209.9	F		68.0	E		41.3	D	
Intersection Delay, s/veh / LOS				108.5						F					
Multimodal Results				EB			WB			NB			SB		
Pedestrian LOS Score / LOS				3.4			C			3.5			D		
Bicycle LOS Score / LOS				2.5			B			2.2			B		







**Appendix N: Intersection Alternative – HCS 2010
Analysis**
(2 Sheets)

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HCS 2010 Signalized Intersection Results Summary																											
General Information						Intersection Information																					
Agency						Duration, h		0.25																			
Analyst		CM		Analysis Date		Mar 25, 2016		Area Type					Other														
Jurisdiction						Time Period		PHF					0.98														
Urban Street		US 41 (SR 45)		Analysis Year		2016		Analysis Period					1> 4:00														
Intersection		Coconut Road		File Name		2021 PM Pk Hr w Development - Improved - 03-2...																					
Project Description		2021 PM Pk Hr w Developments - Improved																									
Demand Information				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h				503	339	238	446	308	173	240	1511	524	156	1446	296												
Signal Information																											
Cycle, s		165.0		Reference Phase		2																					
Offset, s		0		Reference Point		End																					
Uncoordinated		No		Simult. Gap E/W		Off		Green		14.1	0.9	68.5	24.4	3.8	28.5												
Force Mode		Fixed		Simult. Gap N/S		Off		Yellow		0.0	0.0	5.1	0.0	4.7	4.0												
								Red		2.5	0.0	2.0	2.0	2.5	2.0												
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase				3			8			7			4			1			6			5			2		
Case Number				2.0			3.0			2.0			3.0			2.0			3.0			2.0			3.0		
Phase Duration, s				34.5			45.5			26.4			37.4			16.6			75.6			17.4			76.5		
Change Period, (Y+R _c), s				7.2			7.2			2.0			7.2			2.5			7.1			7.6			7.1		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.0			3.0			0.0			3.0			0.0		
Queue Clearance Time (g _s), s				26.1			30.9			23.4			29.4			13.5						9.5					
Green Extension Time (g _e), s				1.2			1.1			1.0			0.9			0.5			0.0			0.3			0.0		
Phase Call Probability				1.00			1.00			1.00			1.00			1.00						1.00					
Max Out Probability				0.00			0.00			0.00			0.00			0.00						0.00					
Movement Group Results				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12												
Adjusted Flow Rate (v), veh/h				513	346	243	455	314	177	245	1542	535	159	1476	302												
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1723	1691	1449	1723	1691	1579												
Queue Service Time (g _s), s				24.1	28.9	20.0	21.4	27.4	15.7	11.5	42.1	42.1	7.5	39.2	16.2												
Cycle Queue Clearance Time (g _c), s				24.1	28.9	20.0	21.4	27.4	15.7	11.5	42.1	42.1	7.5	39.2	16.2												
Green Ratio (g/C)				0.17	0.23	0.32	0.15	0.18	0.24	0.09	0.42	0.56	0.06	0.42	0.59												
Capacity (c), veh/h				570	432	511	510	341	383	294	2108	817	205	2134	925												
Volume-to-Capacity Ratio (X)				0.900	0.800	0.475	0.892	0.921	0.461	0.834	0.732	0.655	0.775	0.691	0.326												
Back of Queue (Q), ft/ln (95 th percentile)				407.9	501.8	316.8	368.3	494.3	201.1	224.6	616.3	523.7	153.1	578	120												
Back of Queue (Q), veh/ln (95 th percentile)				16.1	19.8	12.7	14.5	19.5	8.0	8.8	24.3	20.9	6.0	22.8	4.8												
Queue Storage Ratio (RQ) (95 th percentile)				0.67	0.00	0.62	0.65	0.00	0.31	0.31	0.00	0.71	0.26	0.00	0.28												
Uniform Delay (d ₁), s/veh				67.5	59.7	45.3	69.0	66.2	14.6	74.3	40.5	24.9	76.5	39.0	10.3												
Incremental Delay (d ₂), s/veh				2.2	1.3	0.3	2.2	6.8	0.3	2.4	2.3	4.1	2.4	1.9	0.9												
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh				69.7	61.1	45.5	71.2	73.0	14.9	76.7	42.8	29.0	78.9	40.9	11.2												
Level of Service (LOS)				E	E	D	E	E	B	E	D	C	E	D	B												
Approach Delay, s/veh / LOS				61.6		E	61.3		E	43.2		D	39.4		D												
Intersection Delay, s/veh / LOS				48.0						D																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS				3.5		C	3.5		D	3.0		C	3.0		C												
Bicycle LOS Score / LOS				2.3		B	2.0		B	1.8		A	1.6		A												
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HCS 2010 Signalized Intersection Results Summary																											
General Information						Intersection Information																					
Agency				Duration, h		0.25																					
Analyst		CM		Analysis Date		Mar 25, 2016		Area Type					Other														
Jurisdiction				Time Period				PHF					0.98														
Urban Street		US 41 (SR 45)		Analysis Year		2016		Analysis Period					1> 4:00														
Intersection		Coconut Road		File Name		2026 PM Pk Hr w Development - Improved - 03-2...																					
Project Description		2026 PM Pk Hr w Developments - Improved																									
Demand Information				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Demand (v), veh/h				567	375	277	468	386	176	301	1582	551	160	1515	426												
Signal Information																											
Cycle, s		165.0		Reference Phase		2																					
Offset, s		0		Reference Point		End																					
Uncoordinated		No		Simult. Gap E/W		Off																					
Force Mode		Fixed		Simult. Gap N/S		Off																					
				Green	10.0	2.0	56.2	25.5	9.7	31.7																	
				Yellow	5.1	0.0	5.1	0.0	4.7	4.0																	
				Red	2.5	0.0	2.0	2.0	2.5	2.0																	
Timer Results				EBL			EBT			WBL			WBT			NBL			NBT			SBL			SBT		
Assigned Phase				3			8			7			4			1			6			5			2		
Case Number				2.0			3.0			2.0			3.0			2.0			3.0			2.0			3.0		
Phase Duration, s				37.7			54.6			27.5			44.4			19.6			65.3			17.6			63.3		
Change Period, (Y+R _c), s				7.2			7.2			2.0			7.2			2.5			7.1			7.6			7.1		
Max Allow Headway (MAH), s				3.1			3.1			3.0			3.0			3.0			0.0			3.0			0.0		
Queue Clearance Time (g _s), s				29.2			32.4			24.4			36.3			16.5						9.7					
Green Extension Time (g _e), s				1.3			1.3			1.1			1.0			0.6			0.0			0.3			0.0		
Phase Call Probability				1.00			1.00			1.00			1.00			1.00						1.00					
Max Out Probability				0.00			0.00			0.00			0.00			0.00						0.00					
Movement Group Results				EB			WB			NB			SB														
Approach Movement				L	T	R	L	T	R	L	T	R	L	T	R												
Assigned Movement				3	8	18	7	4	14	1	6	16	5	2	12												
Adjusted Flow Rate (v), veh/h				579	383	283	478	394	180	307	1614	562	163	1546	435												
Adjusted Saturation Flow Rate (s), veh/h/ln				1723	1863	1610	1723	1863	1579	1723	1691	1449	1723	1691	1579												
Queue Service Time (g _s), s				27.2	30.4	21.4	22.4	34.3	15.1	14.5	49.8	51.5	7.7	47.7	29.8												
Cycle Queue Clearance Time (g _c), s				27.2	30.4	21.4	22.4	34.3	15.1	14.5	49.8	51.5	7.7	47.7	29.8												
Green Ratio (g/C)				0.18	0.29	0.39	0.15	0.23	0.29	0.10	0.35	0.51	0.06	0.34	0.53												
Capacity (c), veh/h				636	535	629	533	420	452	357	1790	735	209	1728	829												
Volume-to-Capacity Ratio (X)				0.910	0.716	0.449	0.896	0.937	0.397	0.859	0.902	0.765	0.781	0.895	0.524												
Back of Queue (Q), ft/ln (95 th percentile)				463.2	521.6	330.7	382.8	626.3	191.2	268.1	746.3	646.9	156.9	719.2	197.2												
Back of Queue (Q), veh/ln (95 th percentile)				18.2	20.5	13.2	15.1	24.7	7.6	10.6	29.4	25.9	6.2	28.3	7.9												
Queue Storage Ratio (RQ) (95 th percentile)				0.77	0.00	0.64	0.67	0.00	0.29	0.36	0.00	0.88	0.26	0.00	0.46												
Uniform Delay (d ₁), s/veh				65.9	52.8	37.1	68.4	62.7	14.6	72.8	50.7	32.7	76.4	51.6	11.4												
Incremental Delay (d ₂), s/veh				6.9	1.4	0.2	2.2	15.5	0.2	2.4	7.9	7.4	2.4	7.6	2.4												
Initial Queue Delay (d ₃), s/veh				0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0												
Control Delay (d), s/veh				72.9	54.1	37.3	70.6	78.2	14.8	75.1	58.6	40.1	78.8	59.2	13.8												
Level of Service (LOS)				E	D	D	E	E	B	E	E	D	E	E	B												
Approach Delay, s/veh / LOS				59.0		E	63.9		E	56.4		E	51.5		D												
Intersection Delay, s/veh / LOS				56.5						E																	
Multimodal Results				EB			WB			NB			SB														
Pedestrian LOS Score / LOS				3.5			C			3.5			C			3.0			C								
Bicycle LOS Score / LOS				2.5			B			2.2			B			1.9			A			1.7			A		
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Appendix O: Crash Data – West of US 41

(2 Sheets)

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Appendix P: Crash Data – East of US 41

(2 Sheets)

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