

Coconut Road Traffic Study

From Estero Bay to Three Oaks Parkway



Village of Estero, FL 06/08/2016

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Statement of Certification

I certify that this Traffic Study has been prepared by me or under my immediate supervision and that I have experience and training in the field of Traffic and Transportation Engineering.



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Project Description

Traffic counts on major roadways in Lee County had dropped significantly around 2007-2008 due to the downturn in the economy. With the improved economy, more development activity has been the result. As such, Lee County roadway traffic has been climbing up towards pre-recession levels.

Many roadways in the Village of Estero are faced with increased traffic congestion and delays. The Coconut Road Traffic Study was initiated by the Village of Estero, Department of Community Development, to evaluate the impact of development on this roadway. Coconut Road is an east-west roadway and is located in the southern part of the Village of Estero, within Lee County, Florida (refer to **Fig. 1 – Project Location Map**). Within the Village of Estero the west section of this roadway study runs from the Hyatt Hotel to US 41, a distance of approximately 1.6 miles and the east section of this roadway runs from US 41 to Three Oaks Parkway, a distance of approximately 1.7 miles.



Fig. 1 - Project Location Map

The main objective of this traffic study is to evaluate existing and future traffic conditions and to determine if there are improvement needs for Coconut Road. The study process includes a few steps. The first step involves traffic data collection, to help determine the existing roadway Level of Service (LOS), and estimating future travel demand to evaluate if the existing roadway

LOS will be acceptable in the future. The second step includes evaluating corridor improvement needs and potential conceptual alternative improvements, if needed. A safety review of the roadway is provided as well. Finally, conclusions and recommendations are provided.

Existing Roadway Conditions

Coconut Road is classified as a major collector under the jurisdiction and maintenance of the Lee County Department of Transportation, and it stretches from Estero Bay to the west to just west of Interstate 75 to the east (Pebble Pointe at the Brooks).

Refer to Appendix A: Lee County DOT Functional Classification, Appendix B: FDOT Federal – Aid Road Report (Excerpts) and Appendix C: Lee County Road Maintenance Map.

Coconut Road is an undivided two-lane roadway at its western terminus and has mainly an open drainage system. East of the Hyatt hotel, the roadway has turn lanes at many access points. There is also a sidewalk along the south side of the roadway. The posted speed limit for the east-west section is 40 mph. About 900 ft. west of the intersection with US 41 the



East-west portion of Coconut Road terminates at the Hyatt Hotel and the roads runs north-south for a short length (<500 ft) and then west for another 2,000 ft.

roadway changes to curb and gutter with a closed drainage system and sidewalks on both sides of the roadway. East of US 41, the roadway is a four-lane divided curb and gutter facility with a

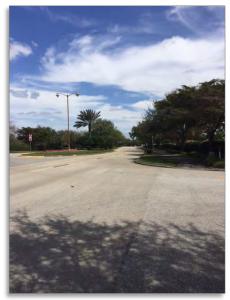
posted speed limit of 45 mph. In the eastern section (from US 41 to Three Oaks Parkway), there are undesignated on-street bicycle lanes on both sides and an asphalt pathway on the south side. Turn lanes are provided as well.

Lee County develops a concurrency report annually that includes an inventory of the maximum utilized and available capacity of public facilities for which minimum Level of Service (LOS) standards are prescribed. The latest report is the October 2015 Concurrency Report. The transportation inventory from the Concurrency



Coconut Road west of US 41 is mainly ane | 5 undivided 2-lane section of roadway.

Report shows roadway link traffic volumes and its corresponding LOS by "Existing" 2014 100th Highest Hour, a short-term projected "Future" estimated 2015 100th Highest Hour, and Future Forecast Volume.



Coconut Road east of US 41 is a divided 4-lane section of roadway.

The Lee County Department of Transportation (Lee DOT) continues to comply with the requirements within the Lee County Comprehensive Land Use Plan (the Lee Plan) by updating the calculations of the maximum service volume for LOS A through LOS E. The maximum service volumes are based on the existing roadway characteristics plus any changes that are part of an improvement that has been programmed for construction in the first three (3) years of the adopted 5-year Lee County Capital Improvement Program (CIP) or the Florida Department of Transportation (FDOT) Work Program.

The existing roadway conditions are extracted from the Lee County October 2015 Concurrency Report and illustrated in **Table 1, Existing Roadways Conditions**.

Table 1
Existing Roadways Conditions

Roadway Link Name	Lee County Link No.	Lee County Roadway Link Location	Exist Roadway ⁽¹⁾	Standard LOS	Standard Volume ⁽²⁾	2014 100 th Highest Hour LOS	2014 100 th Highest Hour Volume ⁽²⁾
Coconut	05000	Spring Creek	2LN	E	860	С	366
Road	00000	Road to US 41	,	_			
Coconut	05030	US 41 to Three	4LD	E	1,790	С	588
Road	05030	Oaks Parkway	4LD		1,790		368

Note(s):

(1) 2LN = 2-narrow lanes roadway; 4LD =4-lane divided roadway, respectively;

Refer to Appendix D: Lee County 2015 Concurrency Report (Excerpts).

Lee DOT operates traffic count programs on its major roadways to provide traffic characteristics and historical data. Coconut Road traffic count data is provided in **Appendix E: Lee County 2015 Traffic Count Report (Excerpts)**.

⁽²⁾ Peak Hour, Peak Season, Peak Direction.

Traffic Volume Data Collection

Existing daily and peak hour traffic count data were collected by Trebilcock Consulting Solutions (TCS) for the study corridor. This included traffic counts conducted along the corridor and its intersection with US 41 and count data provided by Lee DOT.

Bi-directional 72-hour machine traffic counts were conducted along the Coconut Road study corridor in February 2016 at four (4) selected locations (refer to **Appendix F: TCS Corridor Volume Counts**).

Selected corridor traffic count locations are illustrated in Fig. 2 and Table 2.

For the purpose of this study, turning movement counts for the intersection of Coconut Road and US 41 were conducted on Wednesday, January 27, 2016, from 7AM to 9AM, and from 4PM to 6PM.

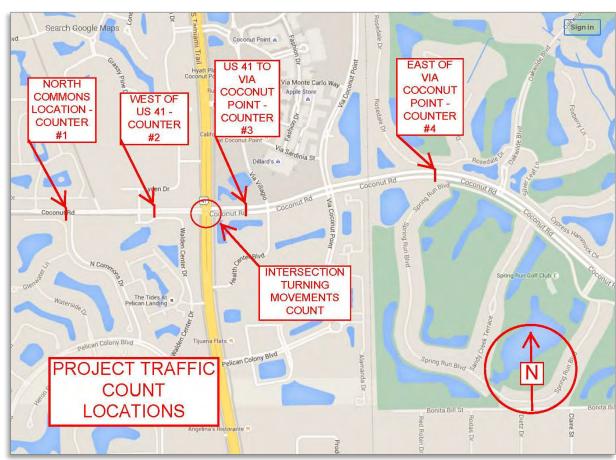


Fig. 2 – Project Traffic Count Locations

Above illustration depicts counting locations along corridor at four locations. In addition, intersection turning movement counts were taken at the US 41 and Coconut Rd.

Table 2
TCS Count Locations

Roadway Link Name	TCS Count Location	TCS Roadway Segment	Lee County Link No.
Coconut Road	1	From Spring Creek Road to US 41	05000
Coconut Road 2		From Spring Creek Road to US 41	05000
Coconut Road	3	From US 41 to Via Coconut Point	05030
Coconut Road	4	From Via Coconut Point to Three Oaks Parkway	05030

Traffic Corridor Level of Service Analysis

Collected daily and peak hour traffic count data was analyzed in accordance with FDOT and Lee DOT procedures.

1. Annual Average Daily Traffic (AADT)

Based on the FDOT procedures outlined in the Project Traffic Forecasting Handbook, the AADT volumes can be determined from short-term traffic count data collected by applying correction factors, such as Weekly Seasonal Correction Factor (SF) and the Axle Correction Factor (AF). The AADT is calculated based on the following formula: AADT = ADT x SF x AF.

The Average Daily Traffic (ADT) is the total traffic volume during a given time period divided by the number of days in that time period. For the purpose of this report, the 72-hour traffic volume is averaged for Tuesday, Wednesday and Thursday. The calculated 2016 AADT volumes for the field traffic counts are rounded to the nearest hundredths.

Weekly Seasonal Correction Factor (SF) is extracted from the latest published FDOT Peak Season Factor Category Report as shown in **Appendix G: 2014 FDOT Peak Season Factor Category Report (Excerpt)**.

As illustrated in the FDOT Traffic Monitoring Handbook, axle factor categories are more highway-specific than seasonal factor categories. For the purpose of this report, the AADT has not been adjusted with axle correction factor. Projected 2016 AADT volumes are illustrated in **Table 3.**

Table 3
Coconut Road – 2016 AADT

TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	3 DAY AVERAGE TWO-WAY	FDOT WEEKLY SEASONAL CORRECTION FACTOR (1)	FDOT AXLE CORRECTION FACTOR	2016 AADT TWO-WAY
1	FROM SPRING CREEK RD TO US 41	05000	10,884	0.81	1.0	8,800
2	FROM SPRING CREEK RD TO US 41	05000	11,969	0.82	1.0	9,800
3	FROM US 41 TO VIA COCONUT POINT	05030	15,824	0.82	1.0	13,000
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	17,339	0.82	1.0	14,200

2. Traffic Characteristics and Existing 2016 Level of Service (LOS)

The evaluation of existing and future traffic operating conditions along Coconut Road is determined based on Directional Design Hourly Volumes (DDHV). DDHV are obtained by applying a number of traffic factors such as Design Hour Factor (K) and Directional Factor (D).

Consistent with the information contained within the 2015 Lee County Traffic Count Report and the 2015 Lee County Concurrency Report, the current 2016 peak season, peak hour, peak direction is calculated by factoring the 2016 AADT by the appropriate K-100 and D-100 factors, approximating the 100th highest hour of the year.

The Lee County Traffic Count Report provides traffic characteristics for roadway stations located on all major roadways in Lee County. Monthly, daily and hourly factors are available for all permanent count station. Traffic characteristics of the permanent count stations are used to factor the periodic count location. A review of 2015 Lee County Traffic Count Report – Permanent Count Station 15 (PCS) – was conducted to determine K-100 factor.

The Directional Distribution, D-100 factor is the percentage of the total, two-way design hour traffic (the 100th highest hour of the year) traveling in the peak direction. The D-100 factor is used in Lee County in calculating the level of service for a roadway.

A "D" factor associated with a specific roadway link is derived using permanent traffic counters located throughout Lee County. As illustrated in the FDOT Traffic Monitoring Handbook, Florida values for "D" range between 50% and 80%.

Site-specific "D" factors are calculated for all four (4) TCS traffic count locations during the AM peak hour of 7-9 AM, and PM peak hour of 4-6 PM. It is noted that based on collected site traffic counts, the eastbound was the peak direction movement in the AM and PM for the Coconut Road link west of US 41. For the Coconut Road segment located east of US 41, it was observed that westbound was the peak direction in the AM, while eastbound was the peak direction in the PM. Site-specific "D" factor values are illustrated in **Table 4.**

Table 4
Coconut Road – D Factor Site Specific

THUF	RS 2-11-2016		COCONUT	RD AT NORTH	L COMMONS	
		TOTAL	DIRECTIONA	LVOLUME	DIRECTION	IAL FACTOR
	PK HR	VOLUME	EB	WB	D FACTOR	DIRECTION
AM	7:15-8:15	611	320	291	52%	EB
PM	4:00-5:00	845	471	374	56%	EB
THU	RS 2-4-2016		COCON	UT RD WEST	OF US 41	
		TOTAL	DIRECTIONA	LVOLUME	DIRECTION	IAL FACTOR
	PK HR	VOLUME	EB	WB	D FACTOR	DIRECTION
AM	7:30-8:30	788	421	367	53%	EB
PM	4:30-5:30	972	601	371	62%	EB
THU	RS 2-4-2016	CC	CONUT RD -	US 41 TO VIA	COCONUT P	OINT
		TOTAL	DIRECTIONA			IAL FACTOR
	PK HR	VOLUME	EB	WB	D FACTOR	DIRECTION
AM	8:00-9:00	900	281	619	69%	WB
PM	4:00-5:00	1,536	793	743	52%	EB
THU	RS 2-4-2016	CC	OCONUT RD -	EAST OF VIA	COCONUT PO	DINT
				LVOLUME	DIRECTION	
		TOTAL	DIVECTIONS	LVOLUIVIL		COLUMN TO SERVICE DE LA COLUMN TO SERVICE DESTRUCTURA DE LA COLUMN TO SERVICE
	PK HR	VOLUME	EB	WB	D FACTOR	DIRECTION
AM	PK HR 7:45-8:45				D FACTOR 61%	

As shown in **Table 4** and consistent with the 2015 Lee County Concurrency Report, the design peak hour utilized in this report is the PM peak hour. To better illustrate corridor specific conditions and account for a design 100th peak hour of the year, the estimated "D" factor utilized in this report is calculated by averaging the site-specific "D" factor and the

recommended Lee County PCS 15 "D" factor. A comparison analysis between site-specific "D" factor and the Lee County Traffic Count PSC 15 "D" factor is illustrated in **Table 5**.

Table 5
Coconut Road – Estimated D-100 Factor

TCS COUNTER		LEE COUNTY LINK ID	TIME	SITE SPECIFIC D-FACTOR		*LEE COUNTY D-FACTOR		ESTIMATED D-FACTOR	
LOCATION	ROADWAY SEGMENT	NUMBER	PERIOD	VALUE	DIRECTION	VALUE	DIRECTION	VALUE	DIRECTIO
1	FROM SPRING CREEK RD TO US 41	05000	AM	0.52	EB	0.53	WB	0.53	EB
5.41			PM	0.56	EB	0.52	EB	0.54	EB
2	FROM SPRING CREEK RD TO US 41	05000	AM	0.53	EB	0.53	WB	0.53	EB
			PM	0.62	EB	0.52	EB	0.57	EB
3	FROM US 41 TO VIA COCONUT POINT	05030	AM	0.69	WB	0.53	WB	0.61	WB
			PM	0.52	EB	0.52	EB	0.52	EB
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	AM	0.61	WB	0.53	WB	0.57	WB
			PM	0.56	EB	0,52	EB	0.54	EB

Existing 2016 operating conditions were determined for roadway segments along Coconut Road as follows: from Spring Creek Road to US 41; from US 41 to Via Coconut Point; and from Via Coconut Point to Three Oaks Parkway. Based on our field observations and a review of the Lee County Traffic Count Report data associated with Coconut Road, it is our recommendation to use TCS counter location #2 (west of US 41) to represent data corresponding to Coconut Road segment from Spring Creek to US 41.

In agreement with the Lee DOT level of service calculations, daily volumes are converted to peak hour, peak season, peak directional volumes by applying K-100 factor, the estimated D-100 factor and Lee County PCS 15 day of the week fraction. The estimated Directional Design Hour Volume (DDHV) is illustrated In **Table 6**.

Table 6
Coconut Road – Estimated Directional Design Hour Volume

TCS COUNTER LOCATION	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	2016 AADT TWO-WAY	*K-100	ESTIMATED D-100	* DAY OF WEEK FRACTION	2016 PEAK HOUR PEAK DIRECTION PEAK SEASON
2	FROM SPRING CREEK RD TO US 41	05000	9,800	0.100	0.57	1.07	522
3	FROM US 41 TO VIA COCONUT POINT	05030	13,000	0.100	0.52	1.07	632
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	14,200	0.100	0.54	1.07	717

The estimated 2016 DDHV is then compared to the directional capacities obtained from 2015 Lee County Concurrency Report and the Lee County Generalized Peak-Hour Directional Service Values. The LOS is reflected in **Table 7.**

Table 7
Coconut Road – Estimated 2016 LOS

TCS COUNTER	ROADWAY SEGMENT	LEE COUNTY LINK ID NUMBER	2016 PK HR,PK	ROAD	PERFÖRM	ANCE STANDARD	2013 LINK SPECIFIC SERVICE VOLUMES, PK HR, PK DIR LOS (2)			PROJECTED
LOCATION			DIR VOLUME	TYPE (1)	LOS	CAPACITY	C	D	E	2016 LOS
2	FROM SPRING CREEK RD TO US 41	05000	522	2LN	E	860	550	860	860	C
3	FROM US 41 TO VIA COCONUT POINT	05030	632	4LD	E	1,790	1,310	1,790	1,790	C
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	717	4LD	E	1,790	1,310	1,790	1,790	C

Consistent with operation performance standard of LOS E for Coconut Road roadway segments as illustrated in the 2015 Lee County Concurrency Report, no level of service deficiencies were identified for existing 2016 peak hour, peak season, peak direction background traffic conditions.

Lee County service volumes for peak hour and peak direction are provided in Appendix H: 2013 Lee County Link – Specific Service Volumes (Excerpt).

3. Future Area Growth

The Coconut Road region has been experiencing significant growth, not only in population but also in economic activities.

The historical traffic data was obtained from the 2015 Lee County Traffic Count Report (excerpts are included in **Appendix E**) and is summarized in **Table 8**.

Table 8
Coconut Road – Historical Traffic Data

ROADWAY	FROM	TO	NUMBER OF	AADT*							AAUI							GROWTH RATE
SEGMENT			LANES	2006	2007	2008	2009	2010	2011	2012	2013	2014	2015					
Coconut Road	Spring Creek Road	US 41	2 LN	6000	9300	N/A	N/A	N/A	7800	N/A	7600	N/A	9200	4.85%				
	US 41	Three Oaks Pkwy	4 LD	15100	15500	12600	9900	10700	9900	N/A	12200	N/A	12200	-2.35%				
US 41	Sout Hicko		6 LD	43300	41300	41200	40200	38600	42000	N/A	36600	37700	42500	-0.20%				

Note(s): *Refer to 2015 Lee Traffic Count Report

A review of the historical traffic data indicates that the annual growth rate is 4.85% for Coconut Road west of US 41, and it is negative for the US 41 and Coconut Road segment located east of US 41. As coordinated with the Village of Estero Transportation staff, a minimum growth rate of one percent (1%) was utilized for the Coconut Road segment east of US 41.

The exponential growth formula was implemented to calculate estimated annual growth rate as follows: $F = P \times (1+r)^n$ where -F = future volume; P = base year traffic volume; P = growth rate percentage; P = number of years from the base year.

4. Coconut Road Corridor Development Potential

Several government documents were reviewed in an effort to determine the consistency with future transportation demand and develop strategies for potential solutions along the corridor. As part of this report, the following documents were reviewed: Lee County Transportation Improvement Program, Fiscal Year 2015/2016 to Fiscal Year 2019/2020 (as adopted September 15, 2015); 2015 Lee County Concurrency Report; and the Lee County Metropolitan Planning Organization (MPO) 2035 Long-Range Transportation Plan (LRTP).

The 2035 LRTP is a long-range plan for transportation needs developed by the Lee County MPO. The 2035 LRTP recommends major transportation projects, systems, strategies, and policies in order to maintain and improve the current transportation system with the objective of meeting future travel demand.

Based on the review of these documents, there are no major improvements depicted in the 2035 LRTP Needs Plan. As such, the existing Coconut Road corridor conditions are consistent with the adopted 2035 LRTP.

The latest Lee County Draft Highway Needs Plan (roadway improvements for Needs Plan) and Lee County Existing and Committed Network are provided in **Appendix I**: Lee County 2035 LRTP (Excerpts).

Although the economic down-turn since 2007-2008 has decelerated the pace of planned projects from this area, several developments approved by either Lee County staff or the City of Bonita Springs Planning staff are still moving forward. The result of this localized growth and implementation of the approved developments are impacting roadway conditions. In addition to the zoning approved developments, there are undeveloped parcels that may be considered for future construction permitting.

For the purpose of this report, zoning approved developments are expected to be completed within the next five (5) years, while the potential future permitting is expected to develop within the next ten (10) or more years.

The projected Coconut Road development potential is summarized in **Table 9** and **Fig. 3** – **Coconut Road Development Potential Map**.

Table 9
Coconut Road – Estimated Development Potential

ID #	STRAP#	PARCEL NAME	DEVELOPMENT PARAMETERS	ZONING APPROVALS NOT YET CONSTRUCTED	POTENTIAL FUTURE CONSTRUCTION PERMITTING
1	07-47-25-B2-00004.00CE	Pelican Landing Community Association	Marina – 20 berths; Restaurant – 3,000sf		X
2A	07-47-25-B2-00003.0370	Estero Bay Marine LLC	Residential – Multi Family – 360 dwelling		X
2B	07-47-25-B2-00003.0000	(Weeks Fish Camp)	units – high-rise buildings		^
3	07-47-25-B2-00000.0010	WCI/Pelican Landing DRI (Raptor Bay)	Residential – Multi Family – 360 dwelling units – high-rise buildings		X
4	06-47-25-00-00002.0030	Pelican Landing Timeshare Ventures LP	Timeshare – Rental Townhouses – 267 dwelling units	X	
5	17-47-25-B1-00001.012A	WCI Communities Inc (Altaira High Rise)	High-Rise Residential Condominium/ Townhouse – 76 dwelling units	X	
6	17-47-25-B1-U1681.1891	WCI Communities Inc (Two Future High Rises)	Residential – Multi Family – 150 dwelling units – high-rise buildings	X	
7	08-47-25-01 +	Eldorado Acres Subdivision	Residential – Single Family – 98 dwelling units – platted lots	X	
8	08-47-25-00-00003.0030	John T. Watson	Residential – Single Family – 2 dwelling units	X	
9	08-47-25-01-00016.0000	Judy K. Doyle	Residential – Single Family – 15 dwelling units	X	
10	08-47-25-00-00004.0000	Dhaliwal + J/T	Residential – Single Family –	X	
11	08-47-25-E2-U1757.2005	Dhaliwal + J/T	3 dwelling units	X	
12	09-47-25-E1-U1823.2024	Dewane/Docese of Venice	Residential – Multi Family – 93 dwelling units		X
13	09-47-25-E4-U1882.1994	Coconut Road Associates LLC	General Office Building – 122,484sf	X	
14	09-47-25-E1-U1874.2023	HG Coconut LLC	Shopping Center – 210,000sf		
15A	09-47-25-E1-U1877.2039	005 51 11 005	Medical Office Building – 40,000sf Multi-Family Residential – 525 dwelling units	X	
15B	09-47-25-E2-U1900.2033	OBE Florida CRE Holdings LLC	Assisted Living – 152 beds	^	
15C	09-47-25-E2-U1902.2012		Hotel – 130 Rooms		
16	09-47-25-E3-31000.0050	Allsee Investment LP	General Office Building – 27,500sf	X	
17	09-47-25-E3-31000.0010	Naples Diagnostic Imaging	General Office Building – 15,000sf	Х	
22A	09-47-25-E3-373A1.0000		Acute Care Hospital – 160 beds		
22B	09-47-25-E3-373A2.0000	Lee Memorial Health System	Shopping Center – 60,000sf Medical Office Building – 198,000sf	X	
22C	09-47-25-E3-373A3.0010	Join	General Office Building – 102,000sf		
		•			

Note: ID# 2A and 2B development potential were based on developer submittals, which were withdrawn subsequent to initial draft of this study. Development parameters may change in the future, but are retained in this report for review consistency.



Fig. 3 – Coconut Road Development Potential Map

The trip generation for the potential development influencing Coconut Road was determined by referencing the Institute of Transportation Engineer's (ITE) report, titled Trip Generation Manual, the most current edition (9th Edition). The software program OTISS (Online Traffic Impact Study Software, Version 4.0.1) is used to create the raw unadjusted trip generation for the project. The ITE – OTISS trip generation – period analysis reports are provided in **Appendix J: Potential Development – ITE Period Analysis Reports**. A summary of the estimated trip generation for potential development along Coconut Road is summarized in **Tables 10A, 10B** and **10C**.

Table 10A

Trip Generation (Developments with Zoning Approvals – Not Yet Constructed)

	Developments	Р	M Peak Ho	ur
ID#	Parcel Name	Enter	Exit	Total
4	WCI/Pelican Landing Timeshare Ventures LP	98	94	192
5	WCI Communities Inc. (Altaira High Rise)	18	11	29
6	WCI Communities Inc. (Two Future High Rise)	56	28	84
7	Eldorado Acres Subdivision	21	60	81
8	John T. Watson	1	1	2
9	Judy K. Doyle	12	7	19
10/11	Dhaliwal + J/T	2	1	3
13	Coconut Road Associates LLC	37	179	216
14/15*	HG Coconut LLC/OBE Florida CRE Holdings LLC	627	642	1,269
16	Allsee Investment LP	19	90	109
17	Naples Diagnostic Imaging	16	79	95
22**	Lee Memorial Health System	205	409	614
	Total Net External	1,112	1,601	2,713

Note(s): *Per approved Coconut Crossing DCI2014-00019 TIS dated October 16, 2014.

Table 10B

Trip Generation (Developments with Potential Future Construction Permitting)

	Developments	PM Peak Hour				
ID#	Parcel Name	Enter	Exit	Total		
1	Pelican Landing Community Association	20	14	34		
2	Estero Bay Marine LLC (Weeks Fish Camp)	115	57	172		
3	WCI/Pelican Landing DRI	115	57	172		
12	Dewane/Diocese of Venice	38	19	57		
	Total Net External	288	147	435		

Note: ID# 2 development potential was based on developer submittals, which have been withdrawn subsequent to initial draft of this study. Development parameters may change in the future, but are retained in this report for review consistency.

^{**}Maximum allowed external trips per February 9, 2016 memorandum from the Village of Estero. Directional distribution assumed consistent with greatest traffic generator, LUC 720.

Table 10C
Trip Generation (Total Future Projected Development Potential)

Douglassanta	Р	M Peak Hoເ	ır
Developments	Enter	Exit	Total
Zoning Approvals – Not Yet Constructed	1,112	1,601	2,713
Potential Future Construction Permitting	288	147	435
Total Net External	1,400	1,748	3,148

Projected traffic generated by the future potential development is assigned to the Coconut Road segments using the knowledge of the area, associated approved traffic impact statements, and as coordinated with Village of Estero Transportation Planning staff.

The assignment of proposed trip distribution is illustrated consistent with the peak hour, peak direction as shown in **Tables 11A**, **11B** and **11C**.

Table 11A

Traffic Distribution* - Developments with Zoning Approvals – Not Yet Constructed

ID#	Parcel Name	Traffic Dist. %	Spring to U	Creek S 41	Traffic Dist. %		to Via ut Point	Traffic Dist. %	to Thre	nut Point e Oaks way
			Enter	Exit		Enter	Exit		Enter	Exit
4	WCI/Pelican Landing Timeshare Ventures LP	100%	WB-98	<u>EB-94</u>	25%	WB-25	<u>EB-24</u>	20%	WB-20	<u>EB-19</u>
5	WCI Communities Inc. (Altaira High Rise)	60%	WB-11	<u>EB-7</u>	20%	WB-4	<u>EB-2</u>	15%	WB-3	<u>EB-2</u>
6	WCI Communities Inc. (Two Future High Rise)	60%	WB-34	<u>EB-17</u>	20%	WB-11	<u>EB-6</u>	15%	WB-8	<u>EB-4</u>
7	Eldorado Acres Subdivision	100%	WB-21	<u>EB-60</u>	20%	WB-4	<u>EB-12</u>	15%	WB-3	<u>EB-9</u>
8	John T. Watson	100%	WB-1	<u>EB-1</u>	20%	WB-0	<u>EB-0</u>	15%	WB-0	<u>EB-0</u>
9	Judy K. Doyle	100%	WB-12	<u>EB-7</u>	20%	WB-2	<u>EB-1</u>	15%	WB-2	<u>EB-1</u>
10/ 11	Dhaliwal + J/T	100%	WB-2	<u>EB-1</u>	20%	WB-0	<u>EB-0</u>	15%	WB-0	<u>EB-0</u>
13	Coconut Road Associates LLC	75%	WB-28	EB-134	20%	WB-7	EB-36	15%	WB-6	<u>EB-27</u>
14/ 15	HG Coconut LLC/OBE Florida CRE Holdings LLC	20%	WB-125	EB-128	30%	WB- 188	EB-193	25%	WB-157	<u>EB-161</u>
16	Allsee Investment LP	80%	WB-15	EB-72	20%	WB-4	EB-18	15%	WB-3	<u>EB-14</u>
17	Naples Diagnostic Imaging	80%	WB-13	<u>EB-63</u>	20%	WB-3	<u>EB-16</u>	15%	WB-2	<u>EB-12</u>
22	Lee Memorial Health System	20%	<u>EB-41</u>	WB-82	25%	EB-51	WB-102	20%	<u>EB-41</u>	WB-82
Pe	eak Direction Total		EB – 625			EB – 359			<u>EB – 290</u>	

Note(s): *Peak hour, peak direction traffic volumes are **Bold and Underlined** as applicable.

Table 11B

Traffic Distribution* - Developments with Potential Future Construction Permitting

		Traffic Dist. %	Spring Creek to US 41		Traffic Dist. %		to Via ut Point	Traffic Dist. %		ut Point to s Parkway
ID#	Parcel Name		Enter	Exit		Enter	Exit		Enter	Exit
1	Pelican Landing Community Association	100%	WB-20	EB-14	20%	WB-4	<u>EB-3</u>	15%	WB-3	<u>EB-2</u>
2	Estero Bay Marine LLC (Weeks Fish Camp)	100%	WB-115	EB-57	25%	WB-29	EB-14	20%	WB-23	EB-11
3	WCI/Pelican Landing DRI	100%	WB-115	EB-57	25%	WB-29	EB-14	20%	WB-23	EB-11
12	Dewane/Diocese of Venice	100%	WB-38	EB-19	20%	WB-8	<u>EB-4</u>	15%	WB-8	<u>EB-3</u>
1	Peak Direction Total		<u>EB – 147</u>	·	<u>EB – 35</u>			<u>EB –27</u>		

Note(s): *Peak hour, peak direction traffic volumes are **Bold and Underlined** as applicable.

Table 11C

Traffic Distribution* - Total Future Projected Development Potential

Coconut Road Development Potential	Spring Creek to US 41	US 41 to Via Coconut Point	Via Coconut Point to Three Oaks Parkway
Development Zoning Approvals	EB – 625	EB – 359	EB – 290
Development Potential Future Construction Permitting	EB – 147	EB – 35	EB – 27
Total Peak Hour Traffic Volume	<u>EB – 772</u>	EB – 394	<u>EB – 317</u>

Note(s): *Peak hour, peak direction traffic volumes are **Bold and Underlined** as applicable.

For the purpose of this report, estimated future Coconut Road demand is analyzed under 2021 and 2026 traffic conditions. Future projected background traffic volumes are calculated based on historic growth rates calculated from annual counts illustrated in the Lee County 2015 Traffic Count Report (as previously summarized in **Table 8, Coconut Road – Historical Traffic Data**).

A review of the historical traffic data indicates that the annual growth rate is negative for the Coconut Road segments located east of US 41. As such, a minimum growth rate of one percent (1%) was utilized for Coconut Road segments east of US 41.

For the purpose of this report, the future traffic growth for the Coconut Road segment located west of US 41 is considered associated with the future projected development

potential. As such, no future growth is applied towards the background traffic for this roadway segment.

The peak hour, peak season, peak direction 2016 100th Highest Hour traffic volume is used as determined in **Table 6, Coconut Road – Estimated Directional Design Hour Volume** of this report. Future 2021 and 2026 background traffic conditions are illustrated in **Table 12**.

Table 12
Coconut Road – Future Background Traffic

Roadway Link	Roadway Link Location	2016 100 th Highest Hour Volume* (trips/hr)	Projected Traffic Annual Growth Rate** (%/yr)	Growth Factor**	Future 2021 Background Pk Hr, Pk Dir Vol*** (trips/hr)	Future 2026 Background Pk Hr, Pk Dir Vol**** (trips/hr)
Coconut Road	From Spring Creek Rd to US 41	522	0.0%	1.000	522	522
Coconut Road	From US 41 to Via Coconut Point	632	1.0%	1.0510, 1.1046	664	698
Coconut Road	From Via Coconut Point to Three Oaks Parkway	717	1.0%	1.0510, 1.1046	754	792

Note(s): *F

5. Coconut Road – Projected Future Level of Service

Future 2021 and 2026 projected traffic capacity and level of service were analyzed for Coconut Road segments. Roadway improvements that are currently under construction or are scheduled to be constructed as depicted within Lee County Transportation Improvement Program, Fiscal Year 2015/2016 to Fiscal Year 2019/2020 (as adopted September 15, 2015), are considered to be committed improvements for the purpose of this study. As no such improvements were identified, the evaluated roadway segments are anticipated to remain as such thru project build out.

The development potential with zoning approvals but not yet constructed is considered within the projected future 2021 traffic conditions. The overall future development potential (to include potential future construction permitting) is included in the future 2026 traffic conditions analysis.

^{*}Refer to **Table 6** of this report.

^{**1%} minimum or historical growth rate; Growth Factor = (1+Annual Growth Rate) AP, P is the number of years from 2016 for that period.

^{***2021} Projected Volume= 2014 100th Highest Hour Volume x Growth Factor with P = 5.

^{****2026} Projected Volume= 2014 100th Highest Hour Volume x Growth Factor with P = 10.

The estimated future 2021 and 2026 peak season, peak hour, peak direction traffic volumes were compared to the to the Level of Service thresholds volumes obtained from 2015 Lee County Concurrency Report and the Lee County Generalized Peak-Hour Directional Service Values. The future projected LOS for Coconut Road segments are reflected in **Table 13** and **Table 14**.

Table 13
Coconut Road – Estimated Future 2021 LOS

	COCONUT ROAD LINK VOLUMES - CAPACITY ANALYSIS												
TCS COUNTER		LEE COUNTY LINK ID	2021 BACKGROUND	2021 ESTIMATED DEVELOPED	2021 TOTAL ESTIMATED	PERFORM	ANCE STANDARD		NK SPECIFIC 5, PK HR, PK		PROJECTED		
LOCATION	ROADWAY SEGMENT	NUMBER	TRAFFIC (1)	TRAFFIC (2)	TRAFFIC	LOS	CAPACITY	С	D	E	2021 LOS		
2	FROM SPRING CREEK RD TO US 41	05000	522	625	1,147	E	860	550	860	860	F		
3	FROM US 41 TO VIA COCONUT POINT	05030	664	359	1,023	E	1,790	1,310	1,790	1,790	С		
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	754	290	1,044	E	1,790	1,310	1,790	1,790	С		
NOTES:	(1) SEE TABLE 12 OF THIS REPORT (2) SEE TABLE 11 OF THIS REPORT (3) 2015 LEE COUNTY CONCURENCY REPORT (4) REFER TO 2013 LEE COUNTY LINK SPECIFIC SERVICE VOL	.UMES											

Table 14
Coconut Road – Estimated Future 2026 LOS

TCS COUNTER		LEE COUNTY	2026 BACKGROUND	2026 ESTIMATED DEVELOPED	2026 TOTAL ESTIMATED	2.700	ORMANCE		K SPECIFIC IES, PK HR LOS (4)		PROJECTE
LOCATION	ROADWAY SEGMENT	NUMBER	TRAFFIC (1)	TRAFFIC (2)	TRAFFIC	LOS	CAPACITY	С	D	E	2021 LO
2	FROM SPRING CREEK RD TO US 41	05000	522	772	1,294	E	860	550	860	860	F
3	FROM US 41 TO VIA COCONUT POINT	05030	698	394	1,092	E	1,790	1,310	1,790	1,790	С
4	FROM VIA COCONUT POINT TO THREE OAKS PARKWAY	05030	792	317	1,109	E	1,790	1,310	1,790	1,790	С
	(1) SEE TABLE 12 OF THIS REPORT (2) SEE TABLE 11 OF THIS REPORT (3) 2015 LEE COUNTY CONCURENCY REPORT (4) REFER TO 2013 LEE COUNTY LINK SPECIFIC SERVICE VOL	UMES									

Based upon the results illustrated in the level of service analysis, the Coconut Road segment located west of US 41 is anticipated to be overcapacity in the year 2021 and 2026 future conditions. All other analyzed segments are projected to operate within the adopted level of service standard at future conditions.

Coconut Road – Segment west of US 41 – No build Alternative

This option would maintain the existing two-lane geometry for the entire section of Coconut Road, west of US 41. As more developments are be approved for construction as shown within this report, significant congestion and delays will increase due to the generated traffic along this roadway segment.

Coconut Road – Segment west of US 41 – Four-Lane Alternative

A four-lane divided Coconut Road segment west of US 41 would provide sufficient capacity to allow for future area development. However, due to Right-of-way

(ROW) constraints and given that this improvement has not been planned/programmed another alternative may be more cost effective. Coconut Road – Segment west of US 41 – Two-Lane Enhanced Alternative.

Two-lane with geometry enhancements or safety-related improvements may provide sufficient capacity, and result in a safer roadway with more efficient traffic operations. These improvements may include mainly intersection improvements, e.g., roundabouts and/or signal optimization timings. Excluding ROW costs reconstructing this 1.58 mile section of roadway to a 4-lane urban section would be in the



Existing two lane portion of Coconut Rd west of US 41 is ROW-constrained in many areas as depicted.

\$6.6 million dollar range as compared to an estimated \$2.6 million dollars to create a series of roundabouts with significantly less ROW impact anticipated (note costs are conceptual).

Over the past several years, roundabouts have become more popular for intersection solutions as they offer several advantages over other traffic controls, they may cost less to install, can accommodate a series of U-turns and left-turn lanes and reduce delay. They may have lower operation and maintenance costs as compared to signal alternatives. Roundabouts can improve safety by simplifying conflicts, reducing vehicle speeds and providing a clearer indication of the driver's

right-of way compared to other forms of channelization. They also provide an opportunity to improve aesthetics of an intersection with landscaping in connection with community enhancement projects. Roundabouts are particularly suited at intersections on local roads where it is not desirable to give priority to either road or where overall traffic calming is desired, or needed.

It is our recommendation that a series of roundabouts along this segment should be considered for the roadway west of US 41.



One potential method of improving the capacity and safety of Coconut Road west of US 41 is to install a series of roundabouts at locations as depicted above.

Coconut Road and US 41 Intersection Analysis

1. Existing 2016 Conditions

The intersection of US 41 and Coconut Road is a major four-legged signalized intersection. For the purpose of this report, US 41 is considered as the Major Street, while Coconut Road is analyzed as the Minor Street.

US 41 (SR 45) is a principal arterial roadway which runs generally north-south and provides connectivity to Naples to the south and Fort Myers to the north. At this location, its typical cross section is a suburban six-lane divided roadway with dedicated bicycle lanes, curbed

median and a paved shoulder (not curbed). The posted legal speed limit is 50 mph in the vicinity of the intersection.

The north approach has three (3) through lanes, one (1) right-turn lane and double dedicated left turn lanes. The south approach has three (3) through lanes, one (1) dedicated left-turn lane and one (1) right turn lane.

Sidewalks are provided on both sides of the roadway. There are overhead power lines along the west side of the roadway. Intersection street lighting is provided.

Coconut Road is a major collector roadway. The west approach is a curb and gutter facility with a closed drainage system and no dedicated bicycle lanes. The posted legal speed limit for this approach is 40 mph in the vicinity of the intersection. The east approach typical cross section is a four-lane divided roadway with dedicated bicycle lanes, curb and gutter, and a closed drainage system. The posted legal speed limit for this approach is 45 mph in the vicinity of the intersection.

The west approach has one through lane, one right-turn lane and double dedicated left-turn lanes. The east approach has one through lane, one right-turn lane and double dedicated left-turn lanes.

Sidewalks are provided on both sides of the roadway on the west approach, and on the south side for the east approach. There are overhead power lines along the south side of the roadway on the west approach.

The existing intersection lane configuration is illustrated in Fig. 4.



Fig. 4 – Existing Intersection Lane Configuration

2. Intersection Turning Movement Traffic Volumes

For the purpose of this study, turning movement counts for the intersection were conducted on-site on Wednesday, January 27, 2016, from 7AM to 9AM, and from 4PM to 6PM.

Traffic count data is adjusted for peak season by applying a peak season conversion factor to the turning movements. Peak Season Conversion Factor (PSCF) is extracted from the latest published FDOT Peak Season Factor Category Report as shown in **Appendix G: 2014 FDOT Peak Season Factor Category Report (Excerpt)**.

For the purpose of this report, intersection operational analysis was completed following three scenarios: (1) Existing 2016 Conditions; (2) Future 2021 background conditions plus estimated development potential with zoning approvals not yet constructed; and (3) Future 2026 background conditions plus estimated future development potential (to include potential future construction permitting).

In addition, the future traffic growth for the Coconut Road segment located west of US 41 is considered associated with the future projected development potential. As such, no future growth is applied towards the background traffic for this roadway segment.

The historical traffic data was obtained from the 2015 Lee County Traffic Count Report (excerpts are included in **Appendix E**) and it was previously analyzed in this report (refer to **Table 8**). A review of the historical traffic data indicates that the annual growth rate is negative for US 41 and Coconut Road, for the segment located east of US 41. As coordinated with Village of Estero Transportation staff, a minimum growth rate of one percent (1%) was utilized for the Coconut Road segment east of US 41.

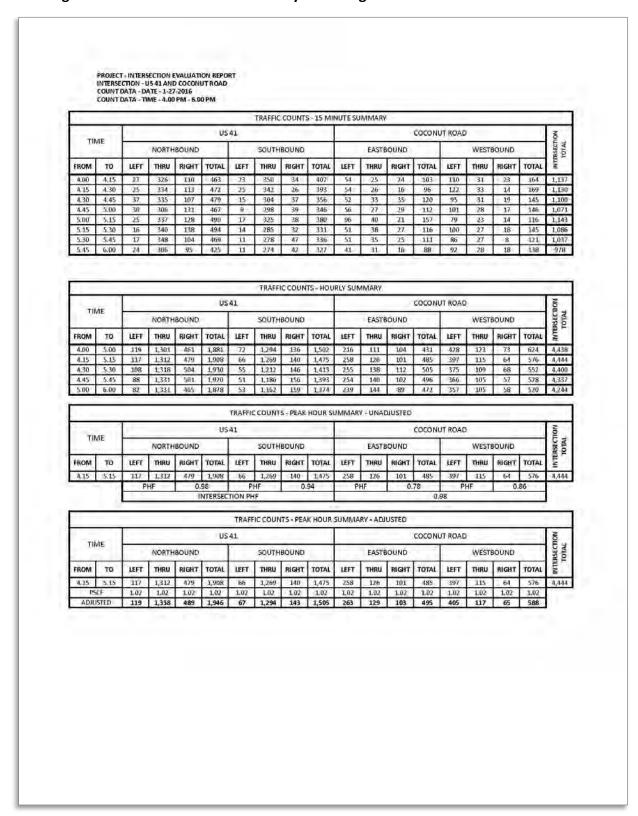
The exponential growth formula was implemented to calculate future traffic volumes for the intersection, as follows: $F = P \times (1+r)^n$ where: F = future volume; P = base year traffic volume; P = growth rate percentage; P = number of years from the base year.

A summary of the intersection turning movement count for peak season 2016 existing conditions is illustrated in Fig. 5A and Fig. 5B.

Fig. 5A – Intersection Count Summary – Existing 2016 Conditions – AM Peak Hour

					115	41	TRAFFIC	COUNT	S-15 MI	NUTESU	MMAR		COCONI	IT ROAD			-	2
TI	ME		NORTH	BOUND		74	SOUTH	BOUND			EASTE	BOUND	COCOM	II WOOL		BOUND		SECTION
FROM	10	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LET	THRU	RIGHT	TOTAL	WIERSECTION TOTAL
7:00	7.15	13	157	24	194 222	13	328 542	31	372 586	15	14 15	21 31	50 69	60 68	13	б.	70 88	686 965
7.30	7.45	16	204	. 44	264	12	520	40	572	27	23	31	- 81	104	18	0	131	1,048
7.45 8,00	8.00	15	213	53 41	287	22	535 469	47	597	33 -45	18	36 35	93	107	36 25	12	155 140	1,132
8.15 8.30	8,30 8.45	22	206	45	273 304	17	480	65 41	562 531	36 44	26 24	30	92 105	103	40	20	163 140	1,090
8,45	9.00	22	215	62	299	31	491	43	565	47	28	45	120	100	29	19	148	1,132
							70.600	ic count	TC UDI	DLV CIU	46.44 my							
-	-				US	41	JHAFF	IC COUN	TS-HOU	KLY SUN	MARY	-	COCONI	JT ROAD		-		Z
TI	ME	0.1	NORTH	BOUND			SOUTH	BOUND			EASTE	BOUND	771		-	BOUND		INTERSECTION
FROM	70	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	MTER
7.00	8.00	-61	765	141	967	60	1,925	142	2,127	98	76	119	293	339	71	34	444	3,831
7.15	8.15	74	841 856	158	1,062	69	2,004	158 192	2,289	128	91	133	341 364	380 415	119	55	514	4,206
7.30	8.30	7.4	050						2,200			4.04				33	589	4,334
7.30 7.45 8.00	8.30 8.45 9.00	79 80	890 892	124 193	1,153 1,165	73	1,958 1,914	193 196	2,224	158 172	92 96	138	388 415	418	122	58	598 591	4,363 4,363
7.45	8.45	79	890	184	1,153	73	1,914	193 196	2,224	158 172	92 96	138	415	418	122	58	598	4,363
7.45 8.00	9.00	79	890	184	1,153 1,165	73	1,914	193 196	2,224	158 172	92 96	138 147 DJUSTED	415	418	122	58	598	4,363 4,363
7.45 8.00	8.45	79	890 892	184	1,153 1,165	73 82 TRAFFI	1,914 COUNT	193 196	2,224	158 172	92 96 Y - UNA	138 147 DJUSTED	415	418	122	58	598	4,363 4,363
7.45 8.00 TII	8.45 9.00 ME	79 80	890 892 NORTH	184 193 BOUND	1,153 1,165 US	73 82 TRAFFII 41	1,914 COUNT SOUTH	193 196 5 - PEAK BOUND RIGHT	2,224 2,192 HOUR S	158 172 JMMAR	92 96 Y - UNAI EASTE	138 147 DJUSTED BOUND RIGHT	COCONU	418 411 UT ROAD	122 115 WEST	58 65 BOUND	598 591 TOTAL	107AL TOTAL
7.45 8.00	8.45 9.00	79 80 LEFT 79	890 892 NORTH	184 193 BOUND RIGHT	1,153 1,165 US	73 82 TRAFFII 41 LEFT 73	1,914 COUNT SOUTH	193 196 S - PEAK BOUND RIGHT 193	2,224 2,192 HOUR S	158 172 JMMAR LEFT 158	92 96 Y - UNA EASTI	138 147 DJUSTED BOUND RIGHT	COCONU	418 411 UT ROAL LEFT 418	122 115 WEST	58 65 BOUND RIGHT	598 591	4,363 4,363
7.45 8.00 TII	8.45 9.00 ME	79 80 LEFT 79	NORTH THRU 890	BOUND RIGHT	1,153 1,165 US TOTAL 1,153	73 82 TRAFFII 41 LEFT 73	SOUTH THRU 1,958	193 196 S - PEAK BOUND RIGHT 193	2,224 2,192 HOUR S	158 172 JMMAR LEFT 158	92 96 Y - UNAI EASTI THRU	138 147 DJUSTED BOUND RIGHT	COCONU TOTAL	418 411 JT ROAL LEFT 418	122 115 WEST THRU	58 65 BOUND RIGHT	598 591 TOTAL 598	107AL TOTAL
7.45 8.00 TII	8.45 9.00 ME	79 80 LEFT 79	NORTH THRU 890	BOUND RIGHT	1,153 1,165 US TOTAL 1,153	73 82 TRAFFII 41 LEFT 73 P	SOUTH THRU 1,958 HF	193 196 S - PEAK BOUND RIGHT 193 0,	2,224 2,192 HOUR S	158 172 JMMAR LEFT 158	92 96 Y - UNA EASTE THRU 92 HF	138 147 DJUSTED SOUND RIGHT 138 0.	TOTAL 388	418 411 JT ROAL LEFT 418	122 115 WEST THRU	58 65 BOUND RIGHT	598 591 TOTAL 598	4,363 4,363 TOTAL
7.45 8.00 TII FROM 7.45	9.00 ME TO 8.45	79 80 LEFT 79	NORTH THRU 890	BOUND RIGHT	1,153 1,165 US TOTAL 1,153 95 NTERSEC	73 82 TRAFFII 41 LEFT 73 P	SOUTH THRU 1,958 HF	193 196 S - PEAK BOUND RIGHT 193 0,	2,224 2,192 HOUR S TOTAL 2,224	158 172 JMMAR LEFT 158	92 96 Y - UNA EASTE THRU 92 HF	138 147 DJUSTED BOUND RIGHT 138 0.	TOTAL 388	418 411 UT ROAL LEFT 418 P	WEST THRU	58 65 BOUND RIGHT	598 591 TOTAL 598	4,363 4,363 TOTAL
7.45 8.00 TII FROM 7.45	8.45 9.00 ME	79 80 LEFT 79	NORTH THRU 890	BOUND RIGHT	1,153 1,165 US TOTAL 1,153 95 NTERSEC	73 82 TRAFFII 41 LEFT 73 PTION PI	SOUTH THRU 1,958 HF RF	193 196 S - PEAK BOUND RIGHT 193 0,	2,224 2,192 HOUR S TOTAL 2,224	158 172 JMMAR LEFT 158	92 96 Y - UNAI EASTI THRU 92 HF	138 147 DJUSTED BOUND RIGHT 138 0.	TOTAL 388	418 411 UT ROAL LEFT 418 P	122 115 WEST THRU 122 HE	58 65 BOUND RIGHT	598 591 TOTAL 598	4,363 4,363 TOTAL
7.45 8.00 TII FROM 7.45	8.45 9.00 ME TO 8.45	LEFT 79	NORTH THRU S90 HE	BOUND RIGHT 184 0.	1,153 1,165 US TOTAL 1,153 95 NTERSEC	73 82 TRAFFIN 41 LEFT 73 PTION PITTON	SOUTH THRU 1,914 THRU 1,918 HF FIC COUNT	193 196 S - PEAK BOUND RIGHT 193 0. JTS - PEA	2,224 2,192 HOUR S TOTAL 2,224 94 K HOUR	JAMMAR LEFT 158 PI SUIMMAR	92 96 Y - UNA EASTE THRU 92 HF	138 147 DJUSTED BOUND RIGHT 138 0. USTED	TOTAL 388 93 0.	418 411 JT ROAL LEFT 418 P JT ROAL LEFT	WEST WEST THRU	BOUND RIGHT	598 591 TOTAL 598 92	INTERSECTION (P. INTERSECTION TOTAL (S) 1992
7.45 8.00 TII FROM 7.45	8.45 9.00 ME TO 8.45	79 80 LEFT 79	NORTH THRU S90 HE	BOUND RIGHT 184 0.	1,153 1,165 US TOTAL 1,153 95 VTERSEC	73 82 TRAFFIG 41 LEFT 73 PTION PITRAFS	SOUTH THRU 1,958 HF FIC COUN	193 196 S - PEAK BOUND RIGHT 193 0.	2,224 2,192 HOUR S TOTAL 2,224 94 K HOUR	158 172 JMMAR JMMAR LEFT 158 PI	92 96 Y - UNA EASTE THRU 92 HF	138 147 DJUSTED BOUND RIGHT 138 0.	TOTAL 388 93 0.	418 411 JT ROAL LEFT 418 P	WEST	BOUND RIGHT 58 0	598 591 TOTAL 598 92	4,363 4,363 TOTAL

Fig. 5B - Intersection Count Summary - Existing 2016 Conditions - PM Peak Hour



Based on the traffic count data, the AM and PM peak hours for the intersection are determined to be 7.45 - 8.45 AM, and 4.15 - 5.15 PM, respectively.

Peak hour factor (PHF) is the hourly volume during the peak hour divided by the peak 15-min flow rate within the peak hour Highway Capacity Manual 2010 (HCM 2010). As illustrated in the 2014 FDOT Project Traffic Forecasting Handbook, PHF is a measure of traffic demand fluctuation within the analysis design hour. For the purpose of this analysis, PHF is determined to be 0.98 for AM and PM peak hour.

Future background traffic is analyzed based on peak season, peak hour traffic for roadway. As such, future 2021 and 2026 future background conditions are illustrated for PM peak hour (refer to Fig. 6).

PM PEAK HOUR FUTURE TRAFFIC COCONUT ROAD SOUTHBOUND EASTBOUND LEFT TOTAL THRU RIGHT TOTAL TOTAL THRU TOTAL 117 1,312 1,908 1,269 140 258 126 485 397 115 TMCs PSCF 1.02 1.02 1.02 1.02 1.02 1.02 1.02 2016 PEAK SEASON VOLUME 119 1,338 429 1,946 67 1,294 143 1,504 263 129 103 495 405 117 65 ANNUAL GROWTH RATE (AGR) 1 /19/ 1.0% 1.0% 1.0% 1.0% 1.0% 0.0% ning. 0.0% 1.0% 1 71% 1 0% 5 YEARS FROM 2016 TO 2021 3 9 5 8 5 3 5. 5 3 2021 BACKGROUND * 2,047 1,361 126 1,407 514 71 151 1,583 263 129 495 426 123 618 103 69 YEARS FROM 2016 TO 2026 10 10 10 10 10 10 10 10 10 10 10 10 2026 BACKGROUND* 1,478 132 541 2.151 75 1.430 158 1.663 263 129 103 448 130 72 650 Note(s): " Future Projected Volume = Base Volume" (1+AGR)⁶; n = number of years from the base year

Fig. 6 – Turning Movement Summary – 2021 and 2026 Conditions – PM Peak Hour

3. Intersection Analysis – General Description

This is an operational analysis that considers demand volumes, intersection signalization, intersection geometric design and the delay to analyze the quality of operations. The intersection is analyzed for capacity adequacy and level of service provided.

The capacity condition for an intersection is defined by a composite volume/capacity ratio for the critical lane groups for the intersection.

The delay incurred by drivers is used to define the level of service for signalized intersection since it reflects driver's discomfort, frustration, energy consumption and travel time. The level of service is based on the average stopped delay per vehicle in the peak 15 minutes is the criterion used for the traffic movements in the intersection.

Turn lanes are used at intersections to accommodate speed changes and maneuvering of turning traffic, and to increase capacity through an intersection. The length of turn lanes consists of deceleration length (to include entering taper) and storage length. Turn lanes should comply with FDOT Design Standards, Index 301 to the extent practical. The available queue length provided should be based on a traffic study.

The blockage of the turning traffic by the through vehicles should be avoided.

Left Turn Lanes

Left turn lanes are probably the single item having the most influence on intersection operations. Intersection capacity analysis procedures (as consistent with the most current Highway Capacity Manual – HCM) are used to determine the number and use of the left turn lanes.

In agreement with FDOT Plan Preparation Manual (PPM), Volume 1, where left turn volumes exceed 300 vehicles per hour (vph), a double left-turn lane should be considered. Fully protected signal phasing is required for double left turns.

Right Turn Lanes

Exclusive right-turn lanes are less critical in terms of safety than left-turn lanes. As illustrated in FDOT Plan Preparation Manual (PPM), Volume 1, right-turns are generally made more efficient than left-turns. Right-turn storage lanes should be considered when right-turn volume exceeds 300 vph and the adjacent through volume also exceeds 300 vehicles per hour per lane (vphpl).

Right-turn lanes can significantly improve the level of service of signalized intersection by providing means of deceleration and stacking for right turn traffic.

4. Level of Service Analysis

An assessment of the level of service (LOS) of the analyzed intersection was conducted based on existing (current 2016) traffic conditions, future year 2021 (to account for estimated development potential with zoning approvals not yet constructed) and future year 2026 (to include potential future construction permitting).

The intersection was analyzed for Level of Service (LOS) using the Highway Capacity Software 2010 (HCS 2010) computer modeling software, most current version (Release 6.80). The HCS 2010 analyzes signalized intersections by implementing the HCM 2010 procedures.

According to HCM 2010, the level of service criterion for signalized intersections is shown in **Table 15**.

Table 15
Level of Service for Signalized Intersections

Average Delay (seconds / vehicle)	1.00
Signalized Intersections	Unsignalized intersections	LOS
< 10.0	< 10.0	Α
> 10.0 to < 20.0	> 10.0 to < 15.0	В
> 20.0 to < 35.0	> 15.0 to < 25.0	С
> 35.0 to < 55.0	> 25.0 to < 35.0	D
> 55.0 to < 80.0	> 35.0 to < 50.0	E
> 80.0	> 50.0	F

The HCS 2010 percent heavy vehicle is assumed the design hour truck (DHT) – the percent of trucks expected to use the roadway segment during the design hour of the design year. DHT is determined as half of T24 (annual 24-hour percentage of trucks), as illustrated in 2014 FDOT Project Traffic Forecasting Handbook. The T24 factor was extracted from FDOT Florida Traffic Online (T24 = 3.1 for all approaches) and illustrated in **Appendix K: FDOT Traffic Online – T24 Factors**. Conservatively, a 2% heavy vehicle factor was assumed for all movements for the purpose of this analysis.

The existing signal timings/phasing as obtained from Lee County Department of Transportation was utilized in this analysis. The approved signalization plan, signal pattern plan and signal timing plan are illustrated in **Appendix L: Lee County – Intersection Signal Data.**

The level of service standard for Coconut Road and US 41 is E, as illustrated in the Lee County Concurrency Report. Each approach was analyzed to ensure that vehicles do not experience excessive delay.

The results of the traffic HCS 2010 intersection analysis for Existing 2016 Conditions, Future 2021 background conditions plus estimated development potential with zoning approvals not yet constructed, and Future 2026 background conditions plus estimated future development potential (to include potential future construction permitting) are summarized in **Table 13A**, **Table 13B** and **Table 13C**. Based on the existing 2016 conditions data, the weekday PM peak hour had higher intersection volumes than the weekday AM peak hour. The future conditions are based on the trip generation data, percent distributions and an

absorption is included as well. As such, the PM peak hour characteristics were selected for use in establishing the future background weekday design hour traffic.

Future background traffic with potential development is illustrated in Fig. 7.

Fig. 7 – 2021 and 2026 Background Traffic with Potential Development – PM Peak Hour

				1000	5.1							X 75.7m.	75a 7.45			
				US	41							COCONI	IT ROAD			
		NORTHE	BOUND			SOUTHE	DUND			EA5TB	OUND		WESTBOUND			
	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTAL	LEFT	THRU	RIGHT	TOTA
₹MCs	117	1,312	479	1,908	66	1,269	140	1,475	258	126	101	485	39/	115	64	576
PSCF	1.02	1,02	1.02		1.02	1:02	1,02		1.02	1.02	1.02		1.02	1.02	1.02	
2015 PEAK SEASON VOLUME	119	1,338	489	1,946	67	1,294	143	1,504	263	129	103	495	405	117	65	583
ANNUAL GROWTH RATE (AGR)	1.0%	1.0%	1.0%		1.0%	1.0%	1.0%		0.0%	0.0%	0.0%		1.0%	1.0%	1.0%	
YEARS FROM 2016 10 2021	5	5	5		5	5	5		5	5	5		5	5	5	
2021 BACKGROUND *	126	1,407	514	2,047	71	1,361	151	1,583	263	129	103	495	426	123	69	618
2021 POTENTIAL DEVELOPMENT	94	104	10	208	85	85	145	315	240	210	135	585	20	185	104	30
FUTURE 2021 TRAFFIC WITH POTENTIAL DEVELOPMENT	240	1,511	524	2,275	156	1,446	296	1,898	503	339	238	1,080	446	308	173	92
YEARS FROM 2016 TO 2026	10	10	10		10	10	10		10	10	10		10	10	10	
2025 BACKGROUND*	132	1,478	541	2,151	75	1,430	158	1,663	263	129	103	495	448	130	72	650
2026 POTENTIAL DEVELOPMENT	159	104	10	283	85	85	268	438	304	246	174	724	20	256	104	380
FUTURE 2026 TRAFFIC WITH POTENTIAL DEVELOPMENT	301	1,582	551	2,434	160	1,515	426	2,101	567	375	277	1,219	468	386	176	1,03

The HCS intersection worksheets and future projected total traffic are provided in **Appendix M: Intersection Analysis – HCS 2010 Printouts**.

Table 13A
Intersection LOS – Existing 2016 Traffic

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS					
AM Peak Hour										
Existing 2016	63.7/E	64.8/E	24.2/C	32.8/C	37.7/D					
		PM Pea	ak Hour							
Existing 2016	85.7/F	85.0/F	24.3/C	22.7/C	38.4/D					

NOTE(S) *Approach Delay in s/veh.

Table 13B
Intersection AM LOS – Future Background and Development Traffic (2021 and 2026)

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS
		AM Pea	ak Hour		
Future 2021	85.4/F	85.3/F	25.4/C	24.0/C	39.0/D
		AM Pea	ak Hour		
Future 2026	85.4/F	85.8/F	26.6/C	25.5/C	39.9/D

NOTE(S) *Approach Delay in s/veh.

Table 13C
Intersection PM LOS – Future Background and Development Traffic (2021 and 2026)

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS	
PM Peak Hour						
Future 2021	156.9/F	148.4/F	49.6/D	40.2/D	80.3/F	
PM Peak Hour						
Future 2026	219.5/F	209.9/F	68.0/E	41.3/D	108.5/F	

NOTE(S) *Approach Delay in s/veh.

The intersection HCS 2010 analysis for the existing 2016 conditions shows that eastbound and westbound approaches operate at Level of Service (LOS) F. In addition, the overall intersection background traffic will exhibit an acceptable LOS D for future 2026 conditions.

The intersection HCS 2010 analysis for future 2021 and 2026 conditions with projected potential development shows an overall intersection LOS F. As illustrated in **Appendix M**, queue spillover from turn lanes is present (Queue Storage Ratio exceeds 1.0). Additionally, the extended queue generated from downstream intersections can spill back into the upstream intersection and diminish the performance of the upstream treatment. This phenomenon is frequently observed in large urban areas where the traffic volume is heavy, intersection spacing is short and cycle length is long. As such, intersection treatments such as adding/extending turning lanes, turning movement restrictions that are traditionally applied to improve intersection capacity may not realize the expected benefits of relieving congestion and reducing delay.

5. Volume to Capacity Ratio Analysis

Failure of an intersection is also dependent on the volume over capacity ratio (v/c) being greater than the value of 1 on any approach of the intersection. The outputs provided by HCS (refer to **Appendix M: Intersection Analysis – HCS 2010 Printouts**) include volume over capacity ratios for each approach. The results of the traffic HCS 2010 volume over capacity analysis for Existing 2016 Conditions, Future 2021 background conditions plus estimated development potential with zoning approvals not yet constructed, and Future 2026 background conditions plus estimated future development potential (to include potential future construction permitting) are summarized in **Table 14A**, **Table 14B** and **Table 14C**.

Table 14A
Intersection V/C Ratio – Existing 2016 Traffic

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio		
AM Peak Hour						
	LT - 0.753	LT - 0.899	LT - 0.796	LT - 0.436		
Existing 2016	TH – 0.484	TH – 0.370	TH – 0.374	TH – 0.837		
	RT – 0.840	RT – 0.208	RT – 0.271	RT – 0.265		
PM Peak Hour						
	LT - 0.864	LT - 0.902	LT - 0.861	LT - 0.528		
Existing 2016	TH – 0.837	TH – 0.620	TH – 0.440	TH – 0.435		
	RT – 0.773	RT – 0.406	RT – 0.563	RT – 0.154		

Table 14B
Intersection v/c ratio – Future Background Traffic

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio		
PM Peak Hour						
	LT - 0.862	LT - 0.907	LT - 0.868	LT - 0.556		
Future 2021	TH – 0.836	TH – 0.607	TH – 0.467	TH – 0.465		
	RT – 0.773	RT – 0.402	RT – 0.598	RT – 0.166		
PM Peak Hour						
	LT - 0.862	LT - 0.912	LT - 0.873	LT - 0.584		
Future 2026	TH – 0.836	TH – 0.607	TH – 0.496	TH – 0.497		
	RT – 0.773	RT – 0.397	RT – 0.636	RT – 0.177		

Table 14C
Intersection v/c ratio – Future Background with Potential Development Traffic

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio		
PM Peak Hour						
	LT – 1.031	LT - 0.912	LT – 1.156	LT - 0.813		
Future 2021	TH – 1.251	TH – 1.332	TH – 0.588	TH – 0.602		
	RT – 1.016	RT – 0.883	RT – 0.714	RT – 0.396		
PM Peak Hour						
	LT – 1.163	LT - 0.917	LT - 1.449	LT - 0.820		
Future 2026	TH – 1.445	TH – 1.669	TH - 0.616	TH - 0.631		
	RT – 1.235	RT – 0.898	RT – 0.752	RT – 0.570		

The threshold value of failure for the background traffic volume over capacity ratio (V/C) is not exceeded by any of the scenario years for the intersection.

However, the threshold (V/C) value is exceeded under future 2021 and 2026 traffic conditions when potential development is considered.

6. Intersection Alternatives

Based on the HCS analysis performed for future projected traffic conditions, a significant increase in vehicle delay is expected and one or more pf the following conditions are expected to occur: thru vehicle queues extend back and block access to exclusive left-turn and right-turn lanes, left-turn and right-turn vehicle queues extend back and spill over into the adjacent thru lanes, and vehicles require more than one signal cycle to clear the intersection.

Signal retiming and intersection geometric improvements are recommended to promote safety, decreased vehicle delay and sufficient capacity.

To support future general growth in the area and to provide for improved intersection operations a number of intersection improvements are recommended as follows: intersection signal retiming; on Coconut Road — extend eastbound right-turn lane and extend westbound dual left-turn lanes; on US 41 (SR 45) — extend southbound right-turn lane and provide northbound dual left-turn lanes.

Additional HCS 2010 analyses were conducted to illustrate potential benefits that would be achieved with these improvements. The HCS intersection worksheets and future projected total traffic are provided in **Appendix N: Intersection Alternative – HCS 2010 Analysis**.

The HCS 2010 analysis for year 2026 potential future conditions shows that all approaches operate at Level of Service (LOS) E or better. In addition, all movements exhibit a volume over capacity ratio less than 1.0 for future 2026 estimated traffic.

The results of the traffic HCS 2010 intersection analysis for future 2021 and 2026 estimated overall traffic are summarized in **Table 15A** and **Table 15B**.

Table 15A
Intersection LOS – Potential Alternative

Intersection Configuration	EB Approach Delay*/LOS	WB Approach Delay*/LOS	NB Approach Delay*/LOS	SB Approach Delay*/LOS	Overall Delay*/LOS	
PM Peak Hour						
Future 2021	61.6/E	61.3/E	43.2/D	39.4/D	48.0/D	
PM Peak Hour						
Future 2026	59.0/E	63.9/E	56.4/E	51.5/D	56.5/E	

NOTE(S) *Approach Delay in s/veh.

Table 15B
Intersection v/c ratio – Potential Alternative

Intersection Configuration	EB Approach V/C Ratio	WB Approach V/C Ratio	NB Approach V/C Ratio	SB Approach V/C Ratio		
PM Peak Hour						
	LT - 0.900	LT - 0.892	LT - 0.834	LT - 0.775		
Future 2021	TH – 0.800	TH - 0.921	TH – 0.732	TH - 0.691		
	RT – 0.475	RT – 0.461	RT – 0.655	RT – 0.323		
PM Peak Hour						
	LT - 0.910	LT - 0.896	LT - 0.859	LT - 0.781		
Future 2026	TH – 0.716	TH – 0.937	TH – 0.902	TH – 0.895		
	RT – 0.449	RT – 0.397	RT – 0.765	RT – 0.524		

Coconut Road - Safety Review

West of US 41

Accident data was supplied by Lee County Transportation staff for the corridor between the west end of Coconut Road, east to Walden Center Drive. The data covers a four year period (12/31/2011 thru 11/13/2015) of accident report information from the Lee County Sheriff's office and Florida Highway Patrol. During this time there were 11 crashes reported. None were fatal. Four of the crashes occurred at night. Weather conditions were clear for 10 of the 11 crashes. Two of the 11 crashes involved possible injuries. None of the accidents involved bikes, motorcycles, pedestrians, intoxication, speeding, loss of control. Five of the crashes involved aggressive driving, two involved distracted driving. One was an angle collision, two were left turn crashes, one was a head on, one involved a heavy truck, one and involved a teen. None of the accidents required an access management review. The table below provides a summary of the crash locations during the reported period. Based on the crash data there does not appear to be any significant crash conditions along the west corridor. Additional crash detail can be found in Appendix O.

			Table of Crashes Al	ong Coconut Road West of US 41	
EventID	EventCrashDate	EventCrashTime	EventOnStreet	EventCrossStreet	EventNodeDescription
86102035	11/13/2015	14:17:00	NORTH COMMONS DR	COCONUT RD	COCONUT RD @ NORTH COMMONS DR
86101422	10/22/2015	23:00:00	ELDORADO BLVD	COCONUT RD	COCONUT RD @ EL DORADO BLVD
86100668	9/19/2015	7:42:00	COCONUT ROAD	VIA VENETO AT THE COLONY ENTRANCE	COCONUT RD @ VIA VENETO BLVD
85882501	5/17/2015	8:10:00	EL DORADO BLVD	COCONUT RD	COCONUT RD @ EL DORADO BLVD
85615619	2/7/2015	2:45:00	COCONUT RD	SPRING CREEK DR	COCONUT RD @ SPRING CREEK RD
84995514	10/8/2014	23:00:00	COCONUT RD	EL DORADO BLVDVD	COCONUT RD @ EL DORADO BLVD
84794472	7/22/2014	9:00:00	COCONUT RD	SAND FLY CT	COCONUT RD @ VIA VENETO BLVD
81598581	11/9/2013	2:00:00	ELDORADO BLVDVD	COCONUT RD	COCONUT RD @ EL DORADO BLVD
81580877	2/12/2013	6:55:00	COCONUT RD	OLD MEADOWBROOK CIR	COCONUT RD @ OLDE MEADOWBROOK BLVD
81588327	8/15/2012	18:19:00	COCONUT RD	NORTH COMMONS DR	COCONUT RD @ NORTH COMMONS DR
82814081	12/31/2011	1701	COCONUT RD	OLDE MEADOWBROOK CIRCLE	COCONUT RD @ OLDE MEADOWBROOK CIR

East of US 41

Accident data was supplied by Lee County Transportation staff for the corridor between US 41, east to Three Oaks Parkway. The data covers a three year period (1/1/2013 thru 12/30/2015) of accident report information from the Lee County Sheriff's office and Florida Highway Patrol. During this time there were fifty crashes reported, which is nearly six times more than the west section of the corridor for the same period of time. None were fatal. Eight of the crashes occurred at night. Weather conditions were clear for twenty-nine of the fifty crashes. Six of the fifty crashes involved possible injuries. One crash involved a bicycle. Two crashed involved a motorcycle. Fives crashes involved speeding, or driving too fast for conditions. Two crashes involved pedestrians. None of the accidents involved intoxication, or loss of control. Sixteen of the crashes involved aggressive driving, six involved distracted driving. Eleven were angle collisions, ten were left turn crashes, two a head on, three involved a heavy truck, and ten involved a teen. None of the accidents required an access management review. The following table provides a summary of the crash locations during the reported period. Based on the crash data provided, there does not appear to be any particular systemic crash conditions along the east corridor. Additional crash detail can be found in Appendix P. No crash data is provided for the intersection of US 41 and Coconut Road, which is a signalized intersection.

EventID	EventCrashDate	EventCrashTime	EventOnStreet	EventCrossStreet	EventNodeDescription
86103219	12/30/2015	15:34:00	COCONUT RD	IMPERIAL PKWY	THREE DAKS PKWY @ COCONUT RD
86102652	12/6/2015	15:47:00	COCONUT RD	HEALTH CENTER BLVD	COCONUT RD @ VIA VILLAGIO
86102546	12/3/2015	13:54:00	COCONUT RD	VIA VILLAGIO PKWY	COCONUT RD @ VIA VILLAGIO
86102441	11/25/2015	9:22:00	COCONUT RD	IMPERIAL PKWY	THREE OAKS PKWY @ COCONUT RD
86101905	11/9/2015	5:31:00	IMPERIAL BLVD	COCONUT DR	THREE OAKS PKWY @ COCONUT RD
85188983	10/31/2015	11:00:00	THREE OAKS PW	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
86101568	10/28/2015	7:40:00	IMPERIAL PKWY	COCONUT RD	THREE DAKS PKWY @ COCONUT RD
86101368	10/19/2015	17:35:00	COCONUT RD	OAKWILDE BLVD	COCONUT RD @ OAKWILDE BLVD
86100537 86100546	9/14/2015 9/14/2015	14:47:00 19:24:00	VIA COCONUT POINT	COCONUT RD.	COCONUT RD @ VIA COCONUT POINT THREE OAKS PKWY @ COCONUT RD
86100131	8/28/2015		COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
86099780	8/14/2015	1000	THREE OAKS PKWY	COCONUT RO	THREE OAKS PKWY @ COCONUT RD
85883659	7/7/2015		COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
85883099	6/12/2015	- 63.5	COCONUT RD	VIA COCONUT PT	COCONUT RO @ VIA COCONUT POINT
85882790	5/29/2015	V2.7	COCONUT RD	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
85118465	5/28/2015		THREE DAKS PW	COCONUT RD	THREE DAKS PKWY @ COCONUT RD
85882711			VIA COCONUT PT	Acres and the second	COCONUT RD @ VIA COCONUT POINT
Do 100-1	5/26/2015			COCONUT RB	
85882715	5/26/2015	1 7,53	IMPERIAL PKWY	COCONUT RD	THREE CIAKS PKWY @ COCCINUT RD
85882351	5/11/2015	9.13	THREE CAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85776473	4/6/2015		COCONUT RD	THREE OAKS PKWY	THREE OAKS PKWY @ COCONUT RD
85775612	3/10/2015		THREE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
85775332	3/2/2015	10 A 1	SPRING RUN BLVD	COCONUT RD	COCONUT RD @ SPRING RUN BLVD
85615128	1/20/2015	16:15:00	SANDY CREEK TERRACE	SPRINGRUN BLVD	SPRING RUN BLVD @ SANDYCREEK TER.
85614965	1/14/2015		THREE GAKS PKWY	COCONUT RD	THREE DAKS PKWY @ COCONUT RD
85614893	1/12/2015	15:25:00	COCONUT RD	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
85614112	12/15/2014	11:25:00	COCONUT DR	SPRING RUN BLVD	COCONUT RD @ SPRING RUN BLVD
84995305 84995239	11/12/2014 11/10/2014	17:24:00 4:01:00	COCONUT RD CR 881	SPRING RUN BLVDVD COCONUT RD	COCONUT RD @ SPRING RUN BLVD THREE DAKS PKWY @ COCONUT RD
84996252	11/10/2014	16:15:00	VIA COCONUT PT	COCONUT RD	COCONUT RB @ VIA COCONUT POINT
84995889	10/26/2014	18;50:00	IMPERIAL PKWY	COCONUT RB	THREE DAKS PKWY @ COCONUT RD
84995546	10/10/2014	0:08:00	COCONUT RD	VIA COCONUT PT	COCONUT RD @ VIA COCONUT POINT
84794116	7/2/2014	12:14:00	THREE CAKS PKWY	COCONUT RB	THREE DAKS PKWY @ COCONUT RD
84793592	6/3/2014	- Virg	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
84152318	3/4/2014	15:58:00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
9482355	1/29/2014		COUNTRO	VIA COCONUT POINT	COCONUT RD @ VIA COCONUT POINT
81585676	12/9/2013		COCONUT RD	VIA COCONUT RD	COCONUT RO @ VIA COCONUT POINT
81588597	10/3/2013		COCONUTRD	VIA COCONUT PT	COCONUT RD @ VIA COCONUT POINT
81585001	9/24/2013		IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
9506440	7/22/2013		CIR 881	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
81593383	6/16/2013		COCONUT BLVD		COCONUT RD @ SPRING RUN BLVD
81590299	5/16/2013	7.3	IE IMPERIAL PARKWAY IE THREE OAKS	COCONUT ROAD	THREE OAKS PKWY @ COCONUT RD
9499169	4/28/2013	- 57	THRE OAKS PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
7				The state of the s	
81589663	3/18/2013		COCONUT RD	HEALTH CENTER BLVDVD	COCONUT RD @ VIA VILLAGIO
83276176	3/14/2013		COCONUT RD	THREE OAKS PKWY	THREE CAKS PKWY @ COCONUT RO
81591774	3/12/2013	10.00	IMPERIAL PKWY	COCONUT RD	THREE OAKS PKWY @ COCONUT RD
9502305	2/28/2013		8200 HEALTH CENTER BLVDVD	Lanca Car	COCONUT RD @ VIA VILLAGIO
81590065	2/24/2013		COCGNUT RD	VIA VILLAGIO WAY	COCONUT RD @ VIA VILLAGIO
81591908	2/13/2013		COCONUT RD	HEALTH CENTER BLVDVD	COCONUT RD @ VIA VILLAGIO
81586566	1/16/2013	18:19:00	COCONUT RD	VIA COCONUT POINT	COCONUT RD @ VIA COCONUT POINT

Conclusions and Recommendations

The main objective of this traffic study is to evaluate engineering data, document information, and to determine if there are improvement needs for Coconut Road future traffic conditions.

Once forecasts were developed, a roadway segment analyses was performed for future 2021 and 2026 conditions. The Level of Service E standard was utilized as a screening method to evaluate whether a roadway segment was deficient for the future condition. Based upon the results illustrated in corridor segment level of service analysis, the Coconut Road segment located west of US 41 is anticipated to be over capacity in the year 2021 and 2026 future conditions. All other analyzed segments are projected to the adopted level of service standard at future conditions.

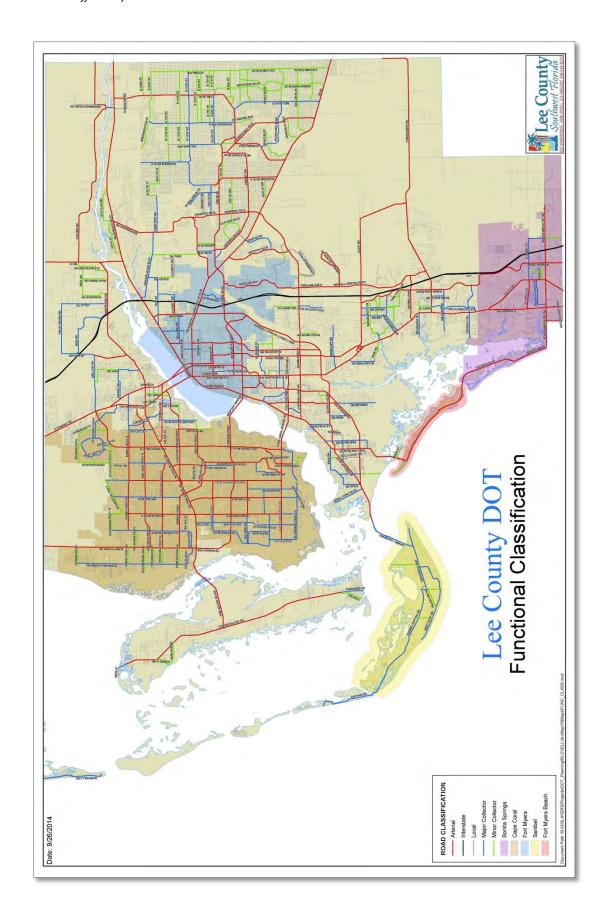
A significant benefit to overall mobility on Coconut Road west of US 41 is provided by a series of roundabouts along this segment. We would recommend analyzing this option as a way to meet the future buildout along this corridor.

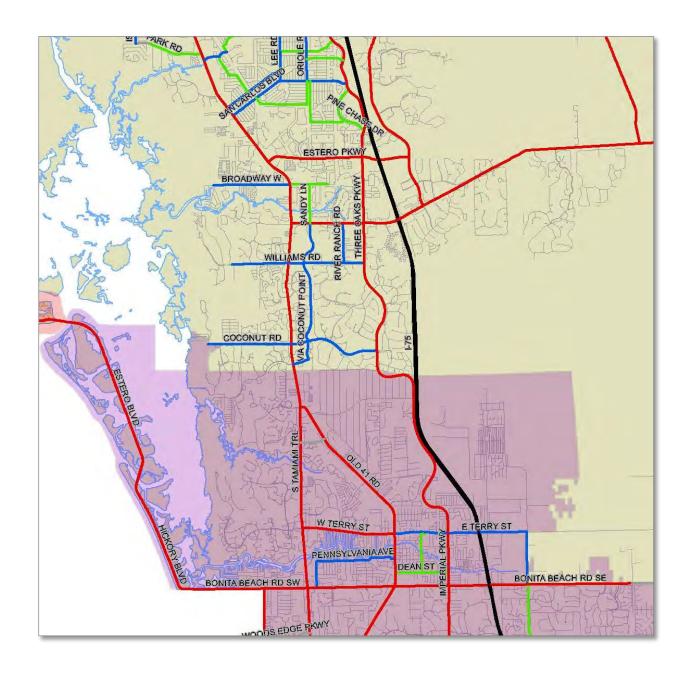
We would recommend working with Lee County and the City of Bonita Springs to identify and agree on needed improvements for Coconut Road and for these to be included in future Lee County Metropolitan Planning Organization Long Range Transportation Plan (MPO LRTP) programing.

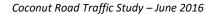
The operational analyses completed for the intersection of Coconut Road and US 41 indicates the need for future intersection improvements to accommodate future volume growth. These improvements include turn lane extensions on Coconut Rd/US 41 and a dual northbound left on US 41.

To provide adequate operations through the 2026 future traffic conditions, new intersection lane configuration and signal retiming are recommended to produce lower delays, sufficient capacity and an acceptable level of service.

nut Road Traffic Study – June 2016	
Appendix A: Lee County DOT Functional Classification (2 Sheets)	l









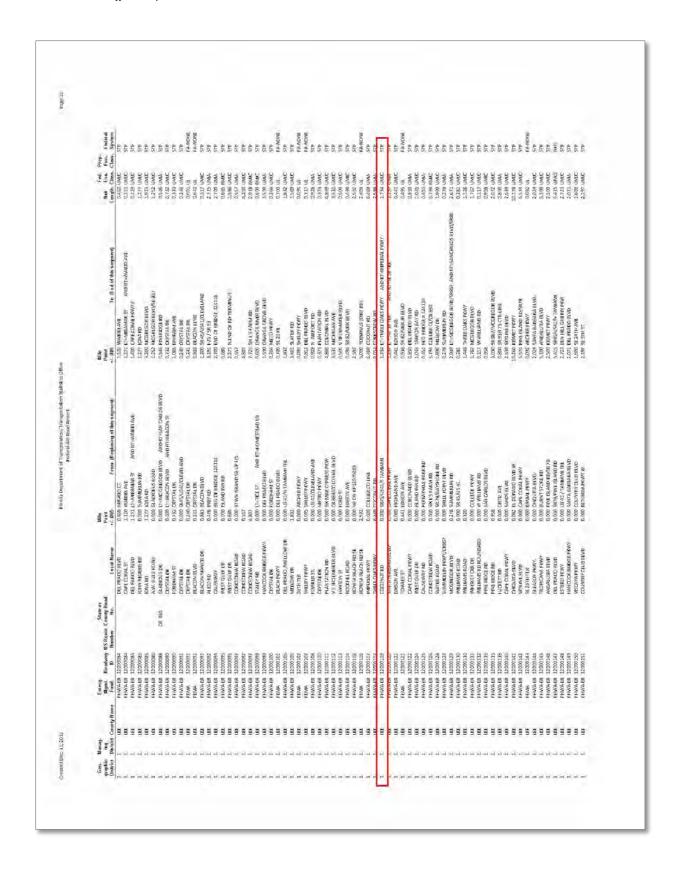
(3 Sheets)

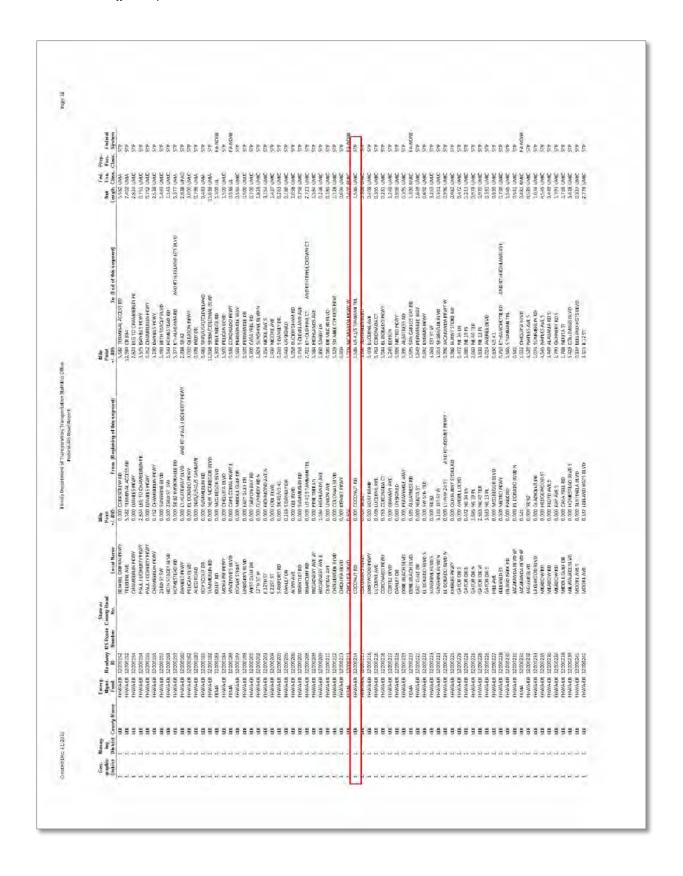
Federal-Aid Road Report

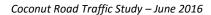
This report provides detailed information about all roads on the State Highway System and many additional public roads. It includes the Federal agency responsible for providing federal aid after a disaster, functional classification, local road name, and the name of the road or boundary at each end of each segment identified by FDOT's Roadway ID.

The roads are listed first by county, and then in the order of the Roadway ID. A given Roadway ID may include portions with several different local road names, and a single local road name may include multiple Roadway IDs.

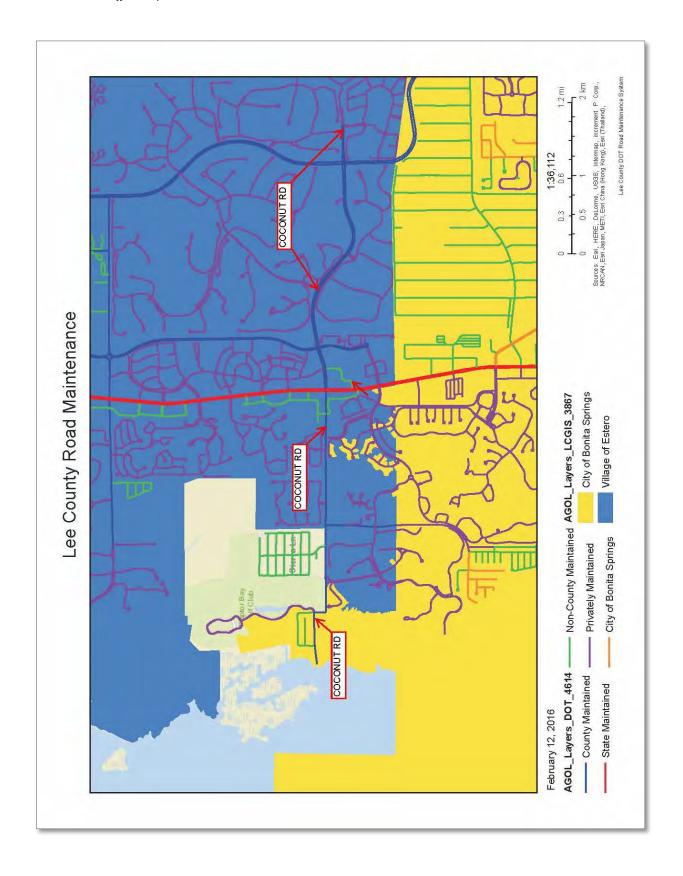
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und FHWA-ER FEMA 1 thru 7 1 thru 8 RPA Classification) RPAO RPAO RPAO RPAO RPAO RPAO RPAO RPAO	Column	Code	Description	
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1 thru 8 RPAI RPAO RPAO RPAO RPAO RPAO RPAO RMA RMIC RMIC RMIL UPAI UPAF UPAF UMAC UMAC UMAC UMAC UMAC UMAC UMAC UMAC	(Emergency Management Fund)	FEMA	Federal Emergency Management Agency	
RPAJ RPAO RPAO RPAO RPAO RPAO RPAO RPAO RPAO	Geographic District	1 thru 7	District Number by County Location	
RPAJ RPAO RPAO RPAO RPAO RPAO RPAO RPAIC RPAO RPAO RPAO RPAO RPAO RPAO RPAO RPAO	Managing District	1 thru 8	District Number by County Location, and	Turnpike ("District 8")
RPAO RRAO RRAO RRAIC RRAIC RRAIC RRAIC RRAIC RRAIC RRAIC RAIC	Fed. Fun. Class	RPAI	01 - Rural Principal Arterial - Interstate	
RPAD RMA RMAC RMIC RMIC RMIC RMIC RMIC UPAI UPAI UPAO UPAA UMA UMA UMA Codes same as for s	(Federal Functional Classification)	RPAO	02 - Rural Principal Arterial - Other Freew	rays & Expressways
RMA RMAL RMIC RMIC RMIC RMIC RMIC UPAI UPAI UPAG UMAC UMAC UMAC UMAC UMAC UMAC UMAC UMAC		RPAO	04 - Rural Principal Arterial - Other	
RMIC RMIC RL UPAI UPAF UPAO UMA UMA UMA UMA UMA UMA UMA UMA		RMA	06 - Rural Minor Arterial	
RMIC RRI LIPAI LIPAO LIPAO LIMAA LUMIC LUMIC LUMIC LUMIC LUMIC RANONE STP NHS NHS		RMJC	07 - Rural Major Collector	
RIL UPAI UPAE UPAE UPAG UMA UMA UMIC UMIC UL Codes same as for		RMIC	08 - Rural Minor Collector	
UPAI UPAF UPAO UPAO UMA UMA UMIC UMIC UMIC UMIC UL		RL	09 - Rural Local	
UPAF UPAO UMA UMA UMAC UMIC UMIC UMIC STP STP NHS IFANONE		UPAI	11 - Urban Principal Arterial - Interstate	
UPAD UMA UMAC UMIC UMIC UL Codes same as for same as f		UPAF	12 - Urban Principal Arterial - Other Free	ways & Expressways
UMA UMIC UMIC UMIC UL Codes same as fo STP NHS FANONE		UPAO	14 - Urban Principal Arterial - Other	
UMIC UMIC UL UL Codes same as fo same as fo NHS NHS FA-NONE		UMA	16 - Urban Minor Arterial	
UMIC UL Codes same as for same		UMJC	17 - UrbanMajor Collector	
UL Codes same as fo codes same as fo STP NHS FA.NONE		UMIC	18 - Urban Minor Collector	
Codes same as for codes same as for same a		'n	19 - Urban Local	
STP NHS FANONE	Prop. Fun. Class (Proposed Functional Classification)	Codes same s	is for Fed. Fun Class	
ONE	Federal System	STP	Surface Transportation Program	
		NHS	National Highway System	
		FA-NONE	Federal Aid None	Created
For More Information		Ē	More Information	200 100
near the state of			200	Dec. 31, 2013
The state of the s	Call The Control of t		Wille	Email
Internation of the British of Transportation Local: 850-414-4848 Transportation Statistics Office	Local: 850-414-4848		Florida Department of Transportation Transportation Statistics Office	Enc Patienner worldot, state III us. Tina, Haterier world state if III.s.
Toll-Free: 800-399-5503 608-5003 77 Talahasean 01 33390,0050	Toll-Free: 800-399-5503		605 Suwannee St, MS 27	







Appendix C: Lee County Road Maintenance Map (1 Sheet)



Appendix D: Lee County 2015 Concurrency Report (Excerpts)

(2 Sheets)

LINK	NO.	02800	02900	02950	03200	03300	03400	03500	03600	03700	03730	03800	03900	04000	04200	04300	04400	04500	04600	04700	04800	04900	00050	05030	05100	05200	05300	05400	05500	00990	00250	02800
NOTES*												1	4 Ln construction in FYs 16/17 & 18/19	4 Ln construction up to Van Buren in FY 14/15	3					Constrained v/c = 0.31		Port Authority maintained	No count since 2007						2	Alternatives analysis in FY 18/19		N. Airport Rd. Ext. current funding for design &
FORECAST FUTURE VOL	VOLUME	1,876	462	462	866	116	213	181	262	362	530	1,162	069	453	2,148	1,414	836	431	2.616	324	291	94	411	908	1,878	1,872	2,486	1,479	2,628	2,836	2,535	3,125
10 E	SOT	O	В	B	a	O	o	o	O	o	۵	ıL	٥	Ö	o	O	В	٧	0	O	O	O	O	O	O	a	o	a	Q	ш	a	LL.
HIGHEST HR	VOLUME	1,876	462	462	866	116	203	181	242	350	530	482	664	357	2,051	1,414	824	380	2,616	267	291	94	386	651	1,837	1,872	2,455	1,456	2,628	2,836	2,535	3,125
EST	TOS	O	В	В	a	Ö	o	Ü	0	o	a	a	O	a	0	O	В	A	S	O	O	o	O	ပ	O	Q	O	a	O	Hi:	0	IIL.
2014 100th HIGHEST HR	VOLUME	1,876	462	462	866	116	203	181	239	350	529	481	664	356	2,051	1,414	824	380	2,616	267	285	93	386	889	1,831	1,872	2,371	1,456	2,628	2,836	2,535	3,125
201 HGF	SOT	0	В	В	a	0	o	O	0	O	a	a	o	В	0	o	8	Y	2	O	o	0	o	o	Q	Q	Q	a	Q	ш	Q	Œ.
PERFORMANCE STANDARD	CAPACITY	2.740	2,000	2,000	2,550	860	860	860	860	066	066	066	1,100	1,100	2,740	2,740	1,820	1,080	4.000	860	098	1,790	860	1,790	2,980	2,980	2,980	2,980	2,780	2,780	2,780	2,780
PERFC	SOT	В	ш	ш	ш	ш	ш	ш	ú	ш	ш	ш	m	ш	a	Q	۵	٥	ú	ш	Ш	В	m	В	Е	ш	ш	ш	Е	ш	щ	ш
ROAD	116	ero	4LD	4LD	QT9	2LU	2LU	2LU	2LU	2LU	2LU	2LU	2LU	2LU	Q79	QT9	4LD	2LU	4F	2LU	21.0	4LD	SLN	4LD	GLD.	QT9	gT9	QT9	QT9	erD	9	9
ρ		1-75	BONITA GRANDE DR	LOGAN BLVD EXT	US 41	US 41	TRIPLE CROWN CT	NORTH RIVER RD	US 41	GUNNERY RD	ORANGE RIVER BL	PALM BEACH BL (SR 80)	DIPLOMAT PKWY	CHARLOTTE COUNTY LINE	PONDELLA RD	PINE ISLAND RD (SR 78)	LITTLETON RD	US 41	McGREGOR BL	SOUTH SEAS PLANTATION	HIGGINS AVE	DANIELS PKWY	US 41	THREE OAKS PKWY	WINKLER RD	WHISKEY CREEK DR	SUMMERLIN RD	US 41	SUMMERLIN RD	US 41	FOWLER ST	METRO PKWY
FROM		IMPERIALST	1.75	BONITA GRANDE DR	SUMMERLIN RD	SUMMERLIN RD	US 41	PALM BEACH BL (SR 80)	AVE	IMMOKALEE RD (SR 82)	GUNNERY RD	ORANGE RIVER BL	PINE ISLAND RD (SR 78)	DIPLOMAT PKWY	FORT MYERS CITY LIMIT	PONDELLA RD		LITTLETON RD			BUCKINGHAM RD	AIRPORT ENT	SPRING CREEK RD	US 41	McGREGOR BL	WINKLER RD	DR	SUMMERLIN RD		SUMMERLIN RD	US 41	FOWLER ST
ROADWAY LINK	INAME	BONITA BEACH RD	BONITA BEACH RD	BONITA BEACH RD	BOY SCOUT DR	BRANTLEY RD*	BRIARCLIFF DR*	BROADWAY (ALVA)*	BROADWAY (ESTERO)*	BUCKINGHAM RD	BUCKINGHAM RD	BUCKINGHAM RD	BURNT STORE RD	BURNT STORE RD	BUS 41 (SR 739)	BUS 41 (SR 739)		BUS 41 (SR 739)	CAPE CORAL BR RD		CEMETERY RD	CHAMBERLIN PKWY	COCONUT RD*	COCONUT RD*	COLLEGE PKWY*	COLLEGE PKWY	COLLEGE PKWY*		COLONIAL BL*	COLONIAL BL	COLONIAL BL (SR 884)	COLONIAL BL (SR 884)

rt Myers Beach, Bonita Springs or Cape Coral state Level of Service Standard. [Note: Some of these links are const. 14, so the 2013 AADT was utilized to develop the link volume. 19 ROADS STATE AND COUNTY ROADS OF "THE LEF PLAN"	Fort Myers, Fort Myers Beach, Bonita Springs or Cape Coral ed County or State Level of Service Standard. [Note: Some of these links are constrictmed in 2014, so the 2013 AADT was utilized to develop the link volume.	 Road links within incorporated areas of Fort Myers, Fort Myers Beach, Bonita Springs or Cape Coral Road links which do not meet the adopted County or State Level of Service Standard. [Note: Some of these links are constrained.] * = Road links which no traffic count was performed in 2014, so the 2013 AADT was utilized to develop the link volume. * = Roads are as indicated in TABLE 2(a) CONSTRAINED ROADS STATE AND COUNTY ROADS OF "THE LEE PLAN". 	rained.]
14, so the 2013 AADT was utilized to develop the DEPADS STATE AND COUNTY ROADS OF "TI	informed in 2014, so the 2013 AADT was utilized to develop the ICONSTRAINED ROADS STATE AND COUNTY ROADS OF "TI	which no traffic count was performed in 2014, so the 2013 AADT was utilized to develop the link volume. as indicated in TABLE 2(a) CONSTRAINED ROADS STATE AND COUNTY ROADS OF "THE LEE PLAN".	ink volume.
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14, so the 2013.	informed in 2014, so the 2013, SONSTRAINED ROADS STA	flic count was performed in 2014, so the 2013, in TABLE 2(a) CONSTRAINED ROADS STA' otherwise.	AADT was utilized
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Boad links within incorporated areas of Bead links which do not meet the adopt Boad links which no traffic count was pe "Constrained" Roads are as indicated in TABLE 2(a) Funding is by Lee County unless noted otherwise.	d links within incorp d links which do not d links which no trat ds are as indicated dounty unless noted		DEF = Road links v * = Road links v Note: "Constrained" Roads are Funding is by Lee County v v/c ratio = 2014 100th High

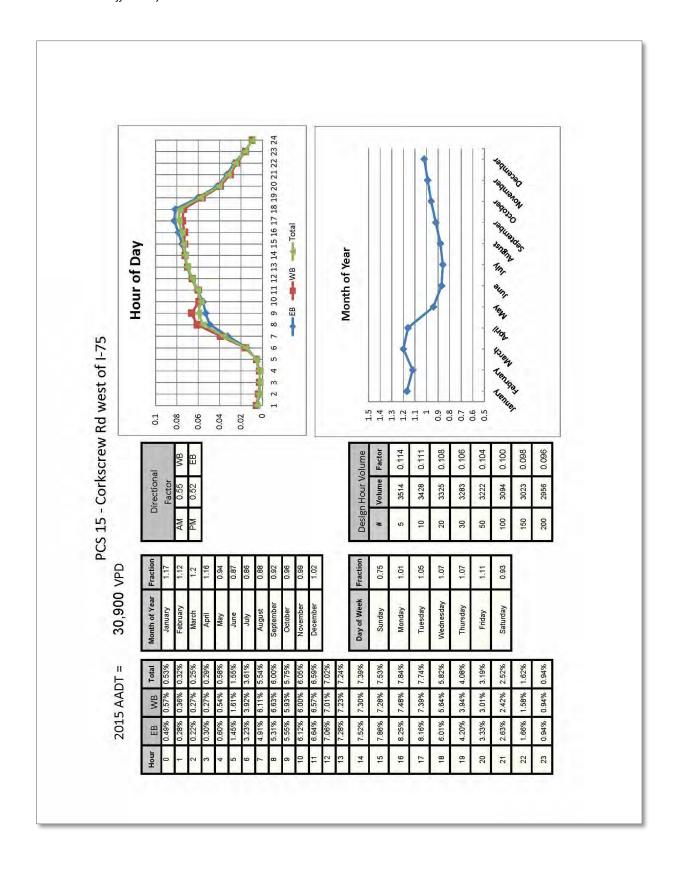
Appendix E: Lee County 2015 Traffic Count Report (Excerpts)

(4 Sheets)

2015 🧏 Anea	ZE 0099	9 0089	10300 6	9	9 00	21900 10	10	41100	25600	23	23	42	52	50	49	8200 36	99	79000	22900	64	00	22900 60	19400
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6 2007 2008 2009 2	ı	2200	0006	1900	7200	19500	19900	26600	20800	3600	1400	4000	1300	3200	2300	10400	28900	23500	18300	10600	24100	15300	H
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2007	6100	4700	0066	2800	9100	21200	22900			9400	2400	2300	200	4900	3200	10900	34000	07770	2	11600	23700	26800	
2006	2800	6300	9500	3200	8200	18900	22600		11700	12600	2800	6400	006	6400	3100	2900	33400	28500		13700	20900	20300	
Sta- tion #	215	201	200	202	203	204	207	임	53	205	206	496	461	504	209	216	218	219	64	217	8	514	71
LOCATION	N OF GLADIOLUS DR	N OF IMMOKALEE RD	S OF HOMESTEAD RD	N OF IMMOKALEE RD	S OF LEELAND HEIGHTS BV	E OF US 41	E OF LEE RD	W OF I - 75	E OF I - 75	E OF BEN HILL GRIFFIN PKWAY	N OF CORKSCREW RD	N OF BONITA BEACH RD	E OF US 41	W OF ORTIZ AV	S OF PINE ISLAND RD	N OF SUMMERLIN RD	E OF BUSINESS 41	W OF HART RD	W OF WILLIAMSBURG DR	E OF NALLE RD	S OF MIDFIELD TERMINAL	S OF ALICO RD	N OF ESTERO PKWY
STREET	A & W BULB RD	ALABAMA RD	ALABAMA RD	BELL BLVD	BELL BLVD	ALICO RD	ALICO RD	ALICO RD	ALICO RD	ALICO RD	ALICO RD	ARROYAL ST	BABCOCK RD	BALLARD RD	BARRETT RD	BASS RD	BAYSHORE RD (SR 78)	SAYSHOKE KD (SK /8)	SAYSHORE RD (SR 78)	SAYSHORE RD (SR 78)	BEN HILL GRIFFIN PKWY	BEN HILL GRIFFIN PKWY	BEN HILL GRIFFIN PKWY

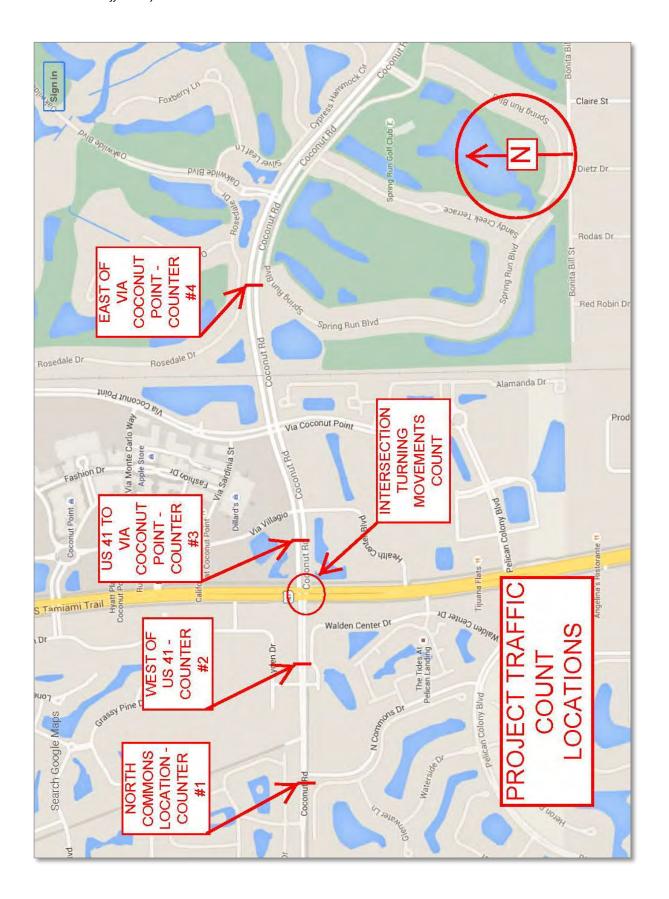
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17400 14600		2700	28200	51000		ı	47500		9200	2400	1800	1800	22200	9300	15500	2200	2,000	36200		37900	47500		33200	58800
19500	ı	8300	30100	54000		ı	48400		0009	5200	1700	2100	19600	0009	15100	2400	0069	38000		40500	20000		36500	61400
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N OF POWELL DR	N OF LITTLETON RD	N OF LAUREL DR	E OF SKYLINE BLVD	W OF PALM TREE	W OF DEL PRADO PKWY	W OF CAPE CORAL BR.	W OF BRIDGE	AT TOLL PLAZA	N OF BLIND PASS BRIDGE	E OF BUCKINGHAM RD	S OF COLONIAL BV	S OF DANIELS PKWY	N OF SW 27TH ST	W OF US 41	E OF US 41	N OF IMMOKALEE RD	E OF US 41	E OF McGREGOR BLVD	W OF SOUTH POINTE BLVD	E OF WINKLER RD	W OF SUMMERLIN RD	W OF NEW BRITTANY	E OF KENWOOD LN	F OF McGREGOR BI VD
BUSINESS 41 (SR 739)	BUSINESS 41 (SR 739)	BUSINESS 41 (SR 739)	CAPE CORAL PKWY	CAPE CORAL PKWY	CAPE CORAL PKWY	CAPE CORAL PKWY	CAPE CORAL BRIDGE	CAPE CORAL BRIDGE	CAPTIVA DR	CEMETERY RD	CHALLENGER BLVD	CHAMBERLIN PKWY	CHIQUITA BLVD	COCONUT RD	COCONUT RD	COLUMBUS BLVD	CONSTITUTION BLVD	COLLEGE PKWY	COLLEGE PKWY	COLLEGE PKWY	COLLEGE PKWY	COLLEGE PKWY	COLLEGE PKWY	COLONIAL BLVD

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35100	49000	ı	36500	50100			37000		57800	58200	58800	43100		53200	50400	52100	38400		43500		52100		43200			29900	24900		26100	17700			20700	5700	12900
36400	47400			53500		41300	41700		25700	57200	63400	49400			28000	52500	42700		49600	51600	53000		46400			32700	26300		25100	18300			15500	9100	14100
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N OF COLLIER CO LINE	N OF BONITA BEACH RD	N OF BONITA BEACH RD	N OF WEST TERRY ST	N OF OLD 41 RD	SOFCOCONUTRD	S OF HICKORY DR	N OF SANIBEL BLVD	N OF CONSTITUTION BLVD	NOFALICORD	N OF ISLAND PARK RD	N OF JAMAICA BAY WEST	N OF SIX MILE CYPRESS PKWY	N OF ANDREALN	N. OF CYPRESS LAKE DR.	N OF BRANTLEY RD	N OF SOUTH RD	N OF BOY SCOUT DR	N OF BOY SCOUT DR	N OF N AIRPORT RD	N OF COLONIAL BLVD	N.OF WINKLER AVE	N OF WINKLER AVE	N OF HANSON ST	S OF VICTORIA AVE	N. OF NORTH KEY DR	N.OF HANCOCK BR. PWY	N OF PONDELLA RD	S OF PINE ISLAND RD	N OF PINE ISLAND RD	N.OF LITTLETON RD	N OF LITTLETON RD	S OF TARA WOODS BLVD	S OF CHARLOTTE CO.	S OF BONITA BEACH RD	S OF PINE ISLAND RD
US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	US 41 (SR 45)	VANDERBILT RD	VETERANS PKWY



Appendix F: TCS Corridor Volume Counts

(11 Sheets)



Coconut Road Volume Study North Commons Drive #29504

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: 029504 Station ID: North Commons Drive #29504 Latitude: 0' 0.0000 Undefined

Start	08-Feb-16		estbound		astbound		mbined	09-Feb		stbound		stbound		bined
Time	Mon	A.M	P.M.	A.M	 P.M. 	A.M.	P.M.	Tue	A.M.	P.M	. A.M	P.M.	A.M.	P.M
12:00		*	*	*	*	*	*		4	101	11	88	15	18
12:15		18	*		*		· A		7	83	2	88	9	1
12:30			1.6						1	96	4	87	5	13
12:45			*	1.60	*	- 4	*		3	105	3	91	6	1
01:00			. A.				W.		2	86	1	121	3	2
01:15				1.4		1.4			3	102	6	109	9	2
		-	7			4	-							
01:30							*		2	107	2	120	4	2
01:45									2	101	1	102	3	2
02:00			*						1	105	2	100	3	2
02:15			A.				Α.		3	105	0	81	3	1
02:30									2	99	3 2	98	5	- 1
02:45			*				*		3	111	2	108	5	2
03:00					*		*		2	114	0	105	2	2
03:15				100		14	*		1	102	1	104	2	2
03:30			A.				*		3	102	3	97	6	1
03:45									3	123	2	98	5	2
			*		*		*						2	
04:00		-6	A.	-	4	- G	*		2	105	5	119	7	2
04:15			-						6	112	5	93	11	2
04:30									8	95	8	106	16	2
04:45									13	108	7	90	20	1
05:00		*					*		10	108	11	89	21	-1
05:15		*				1.4			13	99	10	88	23	1
05:30							5.1		18	97	15	84	33	1
05:45			III A	1,60	41	- 4	*		46	107	17	96	63	2
06:00			+				*		32	129	23	84	55	2
			-		69	G	187				29			
06:15			118		200		1 5 5 7 7		39	73		62	68	1
06:30			115	100	74		189		54	82	51	63	105	1
06:45		44.1	69		51	76	120		68	82	47	46	115	1
07:00			65	*	37		102		53	74	63	49	116	-1
07:15			62	-	40		102		42	58	69	46	111	1
07:30			55		33		88		70	61	101	45	171	1
07:45			44		40		84		86	69	81	42	167	1
08:00		*	50		41		91		74	52	74	34	148	
08:15			55		28	14	83		90	56	78	28	168	
08:30			55		27		82		74	47	82	35	156	
08:45			47	4	22	14	69		78	49	82	18	160	
							69				91			
09:00			52		17				49	48		13	140	
09:15			31	- 5	87		118		65	41	74	18	139	
09:30			30		29		59		62	47	88	22	150	
09:45			28		27		55		61	27	101	38	162	
10:00			21		24		45		52	30	104	33	156	
10:15			22		14		36		53	22	100	24	153	
10:30		*	21		12		33		68	23	86	9	154	
10:45			19	19	15	0	34		96	22	100	18	196	
11:00			12		19		31		93	7	98	.21	191	
11:15			8		7	1,4	15		96	7	107	10	203	
						-								
11.30			14		15	- 2	26		90	8	90	8	180	
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Total		0	994	0	731	0	1725		1809	3595	2012	3133	3821	67
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Vol.														

Coconut Road Volume Study North Commons Drive #29504

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: 029504 Station ID: North Commons Drive #29504 Latitude: 0' 0.0000 Undefined

Time 12:00 12:15 12:30 12:45 01:00 01:45 02:00 02:15 02:30 02:45 03:00 03:15 03:30	Wed	A.M 5 9 4 3 2 4 3 4 1 2	P.M 87 80 111 112 92 102 87 102 114 97	10 3 5 1 3 3 2 2	96 98 89 101 81 86 104	A.M 15 12 9 4 5	P.M. 183 178 200 213 173	Thu	A.M. 7 5 3	P.M 111 110 102	7 5 6	93 104 104	A.M. 14 10 9	P.M. 204 214 206
12:15 12:30 12:45 01:00 01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		9 4 3 2 4 3 4 1 2 1	80 111 112 92 102 87 102 114	3 5 1 3 3 2 2	98 89 101 81 86 104	12 9 4 5 7	178 200 213 173		5	110	7 5 6	104	10	21
12:30 12:45 01:00 01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		9 4 3 2 4 3 4 1 2 1	111 112 92 102 87 102 114	5 1 3 3 2 2	89 101 81 86 104	9 4 5 7	200 213 173		5		5 6		9	
12:30 12:45 01:00 01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		4 3 2 4 3 4 1 2 1	111 112 92 102 87 102 114	5 1 3 3 2 2	101 81 86 104	9 4 5 7	200 213 173		3		6		9	
12:45 01:00 01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		3 2 4 3 4 1 2 1	112 92 102 87 102 114	1 3 3 2 2	101 81 86 104	4 5 7	213 173		-					
01:00 01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		2 4 3 4 1 2	92 102 87 102 114	3 3 2 2	81 86 104	5 7	173		5	100	4	83	0	18
01:15 01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		4 3 4 1 2	102 87 102 114	3 2 2	86 104	7			5	107	2	97	9 7	20
01:30 01:45 02:00 02:15 02:30 02:45 03:00 03:15		3 4 1 2 1	87 102 114	2 2	104				4	98	10	99	14	
01:45 02:00 02:15 02:30 02:45 03:00 03:15		1 2	102 114	2			188				-			19
02:00 02:15 02:30 02:45 03:00 03:15		2	114	2		5	191		1	105	1	92	2 2	19
02:15 02:30 02:45 03:00 03:15		2			107	6	209		2	108	0	97	2	20
02:30 02:45 03:00 03:15		1	0.7	6	103	7	217		0	93	4	109	4	20
02:45 03:00 03:15		1	2/	3	88	5	185		2	101	2	103	4	20
02:45 03:00 03:15			124	5	110	6	234		3	117	0	105		22
03:00 03:15		- 2	103	3	109	5	212		0	106	5	109	3 5 2 5	21
03:15		2	88	1	118	3	206		1	100	1	111	2	21
		2	105	3	110	4	215		3	106	2	88	E	19
05:30													2 -	
		0	111	5	89	5	200		2 2	115	2	102	4	21
03:45		3	115	1	94	.4	209		2	117	1	98	3	21
04:00		2	108	2	102	4	210		0	103	0	127	0	23
04:15		4	133	3	107	7	240		7	97	3	116	10	-21
04:30		5	116	4	129	9	245		8	82	4	103	12	18
04:45		11	110	2	112	13	222		13	92	3	125	16	21
05:00		13	109	8	117	21	226		12	112	11	90	23	20
		18	96	10	99	28	195		21	82	11	98	32	18
05:15														
05:30		20	88	20	111	40	199		14	84	16	116	30	20
05:45		47	105	15	82	62	187		46	96	15	82	61	17
06:00		24	110	16	75	40	185		26	81	27	95	53	17
06:15		43	83	27	73	70	156		35	99	29	74	64	17
06:30		60	71	53	78	113	149		50	95	46	81	96	17
06:45		69	88	72	51	141	139		90	111	59	82	149	19
07:00		51	81	63	48	114	129		58	87	71	71	129	15
07:15		50	76	83	46	133	122		62	66	88	48	150	11
07:30		61	57	83	30	144	87		73	72	82	43	155	11
07:45		90	51	102	34	192	85		89	63	80	34	169	9
08:00		75	44	-87	22	162	66		67	58	70	29	137	8
08:15		64	.55	87	25	151	80		66	64	72	24	138	8
08:30		72	66	89	34	161	100		74	73	80	27	154	10
08:45		61	62	89	20	150	82		75	57	101	20	176	7
09:00		64	54	80	27	144	81		66	67	88	24	154	9
09:15		77	61	82	24	159	85		65	52	98	35	163	8
09:30		71	38	90	27	161	65		59	57	96	34	155	Š
09:45		82	46	99	19	181	65		77	45	81	35	158	8
10:00		79	29	118	27	197	56		69	40	90	30	159	.7
10:15		73	22	126	31	199	53		73	36	97	17	170	5
10:30		87	19	91	20	178	39		88	26	103	26	191	5
10:45		91	23	100	10	191	33		90	20	104	10	194	3
11:00		78	20	95	30	173	50		89	14	138	17	227	3
	_	83	13	103	7	186	20	_	115	6	270	9	228	Ĭ
11:15											113			
11:30	_	90	10	100	13	190	23	_	104	19	102	15	206	
11:45		102	11	110	11	212	22		110	8	98	11	208	1
Total		1863	3685	2165	3224	4028	6909		1936	3760	2128	3342	4064	710
Day Total		5	548	5	389	10	937		5	696	5	470	111	66
% Total		17.0%	33.7%	19.8%	29.5%			5	17.3%	33.7%	19.1%	29.9%		
Peak	1.2	11:00	03:45	10:00	04:15	10:00	04:15	1,2 1	11:00	03:15	10:30	04:00	11:00	03:3
Vol.	- 2	353	472	435	465	765	933		418	441	458	471	869	87
P.H.F.		0.865	0.887	0.863	0.901	0.961	0.952		0.909	0.942	0.830	0.927	0.953	0.95

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: #28817 Station ID: West of US 41 #28817 Latitude: 0' 0.0000 Undefined

Start	01-Feb-1		estbound		hannel 2		ombined	02-Feb		estbound/		hannel 2		bined
Time	Mon	A.N	1. P.M	A.A	 P.M 	A.M.	. P.M.	Tue	AA	И. P.M.	AA	A P.M.	A.M.	P.M.
12:00		*	75	*	0		75		8	84	0	0	8	8
12:15		19	72		0		72		3	83	0	0	3	8
12:30			100		0		100		3	101	0	0	3	10
12:45			83	1.6	0	- 4	83		2	79	0	0	2	7
01:00			77		0		77		3	83	0	0	3	8
						1.61							3	
01:15			98		0		98		1	88	0	0	4	8
01:30		-	132		0	19	132		1	109	0	0	1	10
01:45			.96		0	4.	96		0	105	0	0	0.	10
02:00			84		0		64		3	77	0	0	3	7
02:15		(A)	97	1.00	0	1.6	97		1	91	0	0	1	9
02:30			94		0		94		Ó	102	0	0	o I	10
02:45		-	84	14	0	19	84			101	0	0	2	
						4			2				4	10
03:00			79		0		79		4	100	0	0	4	10
03:15			92	- 90	0	17	92		3	108	0	0	3	10
03:30		4	101		0	16	101		7	87	0	0	7	8
03:45			90	1.0	0	19	90		7	106	0	0	7	10
04:00			84		0		84		4	102	0	0	4	10
04:15		-6	84		o	1.4	84		7	84	0	0	7	8
04:30			92		0		92		14	96	0	0	14	9
04:45		:	87		0		87		12	82	0	0	12	8
05:00			82		0		82		15	93	0	0	15	9
05:15			82		0		82		19	76	0	0	19	7
05:30			86		0		86		29	101	0	0	29	10
05:45			53	1.00	0	- 4	53		30	97	0	0	30	9
06:00			69		0		69		36	73	0	0	36	7
06:15			69		0	19	69		47	94	0	0	47	9
													65	
06:30			64		0		64		65	69	0	0		6
06:45			47		0		47		53	54	0	0	53	5
07:00			61		0		61		58	47	0	0	58	4
07:15			52		0		52		78	56	0	0	78	5
07:30			48		0		48		78	61	.0	0	78	6
07:45		V	45	1.0	0	1.0	45		77	42	0	0	77	4
08:00			37		0		37		103	41	0	0	1.03	4
08:15			39		0		39	_	78	51	0	0	78	5
						- 1								
08:30			46		0		46	_	- 98	37	.0	0	- 98	3
08:45		*	39		0		39		62	49	0	0	62	4
09:00			35		0		35		76	37	0	0	76	3
09:15			25		0	14	25		70	36	0	0	70	3
09:30			28		0		28		82	31	0	0	82	3
09:45			29		0		29		54	21	0	0	54	2
10:00			22		Ö		22		89	25	0	ő	89	2
							22			20				
10:15			21		0	-	21		68	19	0	0	68	1
10:30			21		0		21		84	10	0	0	84	4
10:45			12		0		12		71	7	0	0	71.	
11:00		83	7	0	0	83	7		81	11	0	0	81	1
11:15		82	4.	0	0	82	4		83	5	0	0	83	
11.30		113	7	o o	0	113	7		94	8	0	0	94	
11:45		86	2	0	0	86	2		90	11	0	0	90	1
Total		364	2933	0	0	364	2933		1953	3130	0	0	1953	313
Day Tota	d	and the second second	3297		0	3	297			5083		0	508	13
% Total		11.0%	89.0%	0.0%	0.0%				38.4%	61.6%	0.0%	0.0%		
Peak	1.3	11:00	01:15	- 5	4	11:00	01:15	- 3	07:45	02:30		5	07:45	02:3
Vol.	- 2	364	410	- 4		364	410		356	411	1.4	-	356	41
P.H.F.		0.805	0.777			0.805	0.777		0.864	0.951			0.864	0.95
		0.000	O. F. F. C			U.0U3	0.777		U.004	0.50			0.004	0.33

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: #28817 Station ID: West of US 41 #28817 Latitude: 0' 0.0000 Undefined

Start	03-Feb-1		estbound		hannel 2		ombined	04-Fe		estbound		hannel 2		nbined
Time	Wed	A.M		A.N				Thu			AA	A. P.M	A.M.	P.M.
12:00		5	94	0	0	5	94		6	100	0	0	6	10
12:15		2	114	0	0.	2	114		9	102	0	0	9	10
12:30		4	102	O	0	4	102		6	93	0	0	6	9
12:45		5	101	0	0	5	101		8	99	0	0	8	5
01:00		2	101	ő	ő	2	101		1	114	ō	ő	1	11
						1			1		0			
01:15		1	120	0	0		120			106		0	1	11
01:30		2	124	0	0	2	124		1	130	0	0	1	1.
01:45		1	108	0	0		108		2	77	0	0	2	7
02:00		0	95	0	0	0	95		1	104	0	0	4	10
02:15		3	82	0	0	3	82		2	97	0	0	2	9
02:30		1	93	0	0	1	93		1	89	0	0	9.	ŧ
02:45		0	88	0	0	0	88		3	94	0	0	3	9
03:00		1	95	0	0	1	95		1	106	0	0	1	10
03:15		0	98	0	0	0	98		3	82	0	0	3	2
03:30		1	92	0	0	1	92		3	105	.0	0	3	10
03:45		5	78	0	0	5	78		1	98	0	0		5
04:00		3	94	0	0	3	94		7	109	0	0	7	10
04:15		10	108	0	0	10	108		5	80	0	0	5	
04:30		17	103	0	0	17	103		7	107	0	0	7	10
04:45		16	81	0	0	16	81		14	84	0	0	14	
05:00		15	85	o o	0	15	85		14	84	0	0	14	ì
05:15		18	109	0	0	18	109		25	96	0	0	25	
05:30		34	78	0	0	34	78		35	88	0	0	35	1
05:45		29	89	0	0	29	89		29	99	0	0	29	9
06:00		22	86	0	0	22	86		29	88	.0	0	29	2
06:15		59	73	0	0	59	73		53	93	0	0	53	5
06:30		62	79	0	0	62	79		74	92	0	0	74	3
06:45		56	46	0	0	56	46		52	68	0	0	52	
07:00		55	58	o	0	55	58		58	52	0	0	58	-
07:15		81	58	0	0	81	58		74	57	0	0	74	i
07:30		100	59	0	0	100	59		98	72	0	0	98	- 3
07:45		92	61	0	0	92	61		100	43	0	0	100	
08:00		88	52	0	0	88	52		91	41	0	0	91	
08:15		78	57	0	0	78	57		78	53	0	0	78	3
08:30		81	44	0	0	81	44		81	43	0	0	81	
08:45		67	41	0	0	67	41		78	54	0	0	78	
09:00		68	37	0	0	68	37		72	56	0	0	72	
09:15		80	36	0	0	80	36		69	38	0	0	69	- 3
09:30		70	30	0	0	70	30		96	36	0	0	96	3
09:45		64	32	0	0	64	32		68	34	0	0	68	5
10:00		80	26	0	0	80	26		79	26	.0	0	79	- 0
10:15		7.4	22	0	0	7.4	22		96	23	0	0	96	
10:30		79	11	0	0	79	11		110	16	0	0	110	
10:45		80	17	0	0	80	17		85	13	0	0	85	-
11:00		81	5	0	0	81	5		81	10	ō	0	81	
11:15		111	7	0	o	111	7		107	11	0	Ö	107	
			7				7				0			
11:30		95		0	0	95			98	10		0	98	
11:45		88	7	0	0	88	7		83	6	0	0	83	
Total		1986	3283	0	0	1986	3283		2095	3378	0	0	2095	337
Day Total	D.	- 5	269		0	5	269		5	473		0	547	
% Total		37.7%	62,3%	0.0%	0.0%				38.3%	61.7%	0.0%	0.0%		
Peak	6	11:00	01:00	I d	1	11:00	01:00	-	10:30	00:45		-	10:30	00:
	3.	375	453	5		375	453	-	383	449		-	383	4
Vol.							1.50.50						W 100 M	0.86

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: #29504 Station ID: West of US 41 #29504 Latitude: 0' 0.0000 Undefined

Start	01-Feb-1		stbound		hannel 2		mbined	02-Feb		stbound		hannel 2		bined
Time	Mon	A.M	P.M	A.N		A.M.	P.M.	Tue	A.M			A. P.M	. A.M.	P.M.
12:00		*	147	1-00	0	- *	147		5	139	0	0	5	13
12:15			109	*	0	rith:	109		5	121	0	0	5	12
12:30			110	1.9	0		110		5	129	0	0	5	12
12:45			104		0		104		4	107	0	0	4	10
01:00			106		0		106		3	165	ő	0	3	16
01:15			106		0		106		5	124	0	0.	5	12
01:30			114		0		114		1	106	0	0	1	10
01:45			311		0	*	111		1	109	0	0	1	10
02:00			117		0	- An	117		1	128	0	0	1	12
02:15			114		0	*	114		0	117	0	0	0	11
02:30			122		-0		122		- 1	136	0	0	1	13
02:45		*	140	(100)	0	- 4	140		0	127	0	0	0	12
03:00			138		0		138		2	135	0	o	2	13
03:15			87		0		87		4	134	0	0	4	13
				w										
03:30			130		0		130		4	128	0	0	4	12
03:45			149		0		149		2	133	0	0	2	13
04:00			137		0		137		4	162	0	0	4	16
04:15			132		0		132		5	132	0	0	5	13
04:30			142	*	0		142		5	140	0	0	5	14
04:45		-	129		0	*	129		9	132	0	0	9	13
05:00		4	161	14.	0		161		14	174	0	0	14	17
05:15			111	1040	0		111		10	133	0	0	10	13
05:30			124	-	ő		124		23	129	o	0	23	12
						*					0			12
05:45		-	118		0		118		20	138		0	20	13
06:00			102		0	-	102		26	105	0	0	26	10
06:15			77	CA.	0	*	77		32	89	0	0	32	8
06:30			76	*	0		76		47	82	0	0	47	8
06:45		- 100	92		0	*	92		60	100	0	0	60	10
07:00			54		0		54		68	57	0	0	68	5
07:15			43		0		43		71	49	0	0	71	4
07:30			42		0		42		86	40	ō	0	86	4
07:45			40		0		40		90	27	0	0	90	2
			34				34		130	37	0	0	130	3
08:00				1.4	0	- 4								3
08:15			27	-	0		27		100	32	0	0	100	3
08:30			24		0		24		114	41	0	0	114	4
08:45			16	*	0	+	16		108	43	0	0	108	4
09:00		97	19		0		19		87	22	2	0	89	2
09:15			15		0	*	15		104	43	0	0	104	4
09:30			18		0		18		101	25	0	0	101	2
09:45		12.1	15	1041	0		15		95	19	0	0	95	1
							17		113		0	Ö	113	
10:00			17		0					34				3
10:15		- 1	19		0		19		122	15	0	0	122	1
10:30			15		0	100	15		126	18	0	0	126	1
10:45			14	- 4	0		14		131	7	0	0	131	
11:00		135	17	1	0	136	17		101	15	0	0	101	1
11:15		125	5	0	0	125	5		124	16	0	0	124	1
11:30		119	11	0	0	119	11		116	10	0	0	116	1
11:45		119	3	0	0	119	3		120	2	0	0	120	
		498	3753	1	0	499					2	0		***
Total				7			3753		2405	4106	2		2407	410
Day Total % Total	l.	11.7%	251 88.3%	0.0%	0.0%	4:	252		36,9%	511 63.0%	0.0%	0.0%	6513	5
Peak		11:00	04:15	11:00	540	11:00	04:15	-	10:00	04:30	08:15		10:00	04:3
Vol.	(%)	498	564	1	1	499	564	-	492	579	2	-	492	57
P.H.F.		0.922	0.876	0.250		0.917	0.876		0.939	0.832	0.250		0.939	0.83

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: #29504 Station ID: West of US 41 #29504 Latitude: 0' 0.0000 Undefined

Start	03-Feb-16		stbound		hannel 2	Co	mbined	04-Feb		stbound		hannel 2	Com	bined
ime	Wed	A.M.	P.M.	A.M	P.M.	A.M.	P.M.	Thu	A.M.	P.M.	A.N	 P.M. 	A.M.	P.M.
12:00		7	116	0	0	7	116		4	146	0	0	4	14
12:15		4	112	0	0	4	112		4	145	0	0	4	14
12:30		8	130	O	0	8	130		10	124	0	O.	10	12
12:45		4	128	0	0	4	128		2	118	0	0	2	11
01:00		4	114	ő	ő	4	114		3	113	ő	Ö	3	11
					0	4							3	
01:15		9	99	0	0	9	99		1	115	0	0	1	11
01:30		2	123	0	0	2	123		3	146	0	0	3	14
01:45		2 2	125	0	0	2 0	125		4	89	0	0	4	8
02:00		0	123	0	0	0	123		9	119	0	0	9	11
02:15		1	128	0	0	1	128		4	126	0	0	9	12
02:30		4	136	0	0	4	136		1	139	0	0	1	13
02:45		1	123	0	0	1	123		1	118	0	0	1.	11
03:00		o	143	o	ō	Ö	143		1	159	0	0	1	15
03:15		1	103	ő	Ö	1	103		0	145	0	0	0	14
									2				0	
03:30		5	162	0	0	5	162		3	134	0	0	3	13
03:45		1	126	0	0	1	126		4	121	0	0	4	12
04:00		2	165	0	0	2	165		3	149	0	0	3	14
04:15		4	142	0	0.	4	142		5	128	0	0	5	12
04:30		5	165	0	0	5	1.65		6	148	0	0	6	74
04:45		2	154	0	0	4 5 2	154		5	160	0	0	3 5 6 5	71
05:00		11	192	0	0	11	192		11	151	0	0	11	11
05:15		9	135	Ö	0	9	135		11	142	Ö	0	11	14
		17		0									24	
05:30		17	137		0	17	137		24	141	0	0		14
05:45		20	124	0	0	20	124		14	139	0	0	14	13
06:00		24	118	0	0	24	118		31	127	0	0	31	12
06:15		37	112	0	0	37	112		33	92	0	0	33	1.5
06:30		49	89	0	0	49	89		42	98	0	0	42	9
06:45		59	71	0	0	59	71		57	93	0	0	57	5
07:00		76	66	0	0	76	66		61	70	0	0	61	7
07:15		87	47	0	Ô	87	47		76	52	0	0	76	
07:30		102	42	0	Ö	102	42		108	55	Ö	0	108	i
07:45		102	23	0	0	102	23		101	51	0	0	101	
							39							
08:00		110	39	0	0	110			99	36	0	0	99	-
08:15		119	33	0	0	119	33		113	41	0	0	113	4
08:30		121	36	0	0	121	36		108	40	0	0	108	.4
08:45		109	32	0	0	109	32		116	45	0	0	116	
09:00		98	28	0	0	98	28		102	54	0	0	102	4
09:15		90	66	0	0	90	66		130	22	0	0	130	- 3
09:30		118	35	0	0	118	35		125	40	0	o.	125	
09:45		141	23	0	0	141	23		135	29	0	0	135	
10:00			18	0	0		18		132	21	0	o o	132	2
		113				113		_						
10:15		113	14	0	0	113	14		138	34	0	0	138	
10:30		104	21	0	0	104	21		118	14	0	0	118	7
10:45		114	10	0	0	114	10		146	23	0	0	146	
11:00		114	13	0	0	114	13		104	22	0	0	104	
11:15		126	12	0	0	126	12		120	6	0	0	120	
11:30		108	9	0	0	108	9		148	12	0	0	148	9
11:45		133	6	0	Ö	133	6		132	13	0	0	132	- 83
Total		2490	4168	0	0	2490	4168	-	2608	4305	0	0	2608	430
				U							0			
Day Total % Total		37.4%	658 62.6%	0.0%	0.0%	ь	358	3	7.7%	62.3%	0.0%	0.0%	691	3
Peak	115	09:30	04:15			09:30	04:15		10:00	04:30			10:00	04:
1 55 36 10		485	653	- 4		485	653		534	601			534	60
Vol.														

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110

Coconut Road Volume Study 41 to Via Vilagio #29856

Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz Station ID: Site Code: #29856 Station ID: 41 to Via Vilagio #29856 Latitude: 0' 0.0000 Undefined

Start	01-Feb-16	We	stbound	Ea	stbound	Co	mbined	02-Feb	We	stbound	Ea	stbound	Com	bined
Time	Mon	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	Tue	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00		*	186	1.00	182		368		0	183	3	122	3	308
12:15			178	W.	149	di	327		3	202	1	145	4	347
12:30			168	1.9	161		329		0	169	1	99	1	26
12:45		94	172		168		340		4	200	2	117	3	31
01:00			161	*	140		301		2	181	3	114	5	29
01:15			183		156		339		o I	205	ő	116	Ö	32
01:30					148		336		2	174		119	3	293
		- 20	188	1.00							1			
01:45		1	212	1	167	1	379		1	193	0	127	1	32
02:00			168		178		346		4	202	1	126	5 1	32
02:15			164		155		319		1	156	0	138	1	29
02:30			182		202		384		0	189	2	113	2 2	30
02:45		- 1	186		149		335		2	196	0	130	2	32
03:00			190	1.00	170	*	360		0	197	0	96	0	29
03:15			198		152		350		0	181	1	139	1	32
03:30			186	*	185	*	371		0	186	1	126	1	31
03:45			169		185	*	354		0	188	1	142	1	33
04:00			163	1.0	190		353		5	184	2	135	7	31
04:15			170	100	167		337		2	190	6	146	8	33
04:30			165	*	197		362		4	158	9	141	13	29
04:45			178		179		357		6	191	8	123	14	31
05:00			189		185	- 1	374		6	190	10	133	16	32
05:15			180		177		357		7	166	6	133	13	29
05:30			153		178		331		5	148	19	111	24	25
05:45			147		162		309		19	148	19	131	38	27
06:00			158	*	143	*	301		18	157	17	107	35	26
06:15			122		121	*	243		34	139	29	85	63	22
06:30			136		114	*	250		60	133	41	91	101	22
06:45		*	115	100	104	i k	219		73	99	86	88	159	18
07:00			86		87		173		81	84	52	53	133	13
07:15			84	1.40	86		170		114	67	70	57	184	12
07:30			66		75	*	141		143	67	75	38	218	10
07:45		-	61		74		135		165	66	85	27	250	9
			52											
08:00				(*)	68		120		159	54	107	55	266	10
08:15		-	39		57		96		164	53	95	43	259	9
08:30			36		54		90		141	46	120	38	261	8
08:45			43		46		89		153	74	124	43	277	11
09:00			51	*	44		95		119	59	92	27	211	8
09:15		*	52	*	45	*	97		138	50	91	35	229	8
09:30			32	*	29	*	61		156	50	94	25	250	7
09:45			34	*	27	*	61		154	37	77	26	231	6
10:00			27		20		47		146	22	92	22	238	4
10:15		*	22		28		50		169	23	95	25	264	4
10:30			21	*	15		36		164	16	98	12	262	2
10:45			15		8		23		149	16	100	4	249	2
11:00			12		10		22		159	14	101	6	260	2
		100				-		_						
11:15			5		4		9		156	10	105	4	261	-1
11:30		112	7	75	5	187	12		184	4	119	5	303	
11:45		196	3	190	9	386	12		190	6	128		318	
Total		308	5515	265	5355	573	10870		3259	5723	2189	3941	5448	966
Day Tota % Total		2.7%	48.2%	2.3%	320 46.8%	11	443	3	89 21.6%	982 37.9%	14.5%	26.1%	151	12
Peak	12	2	02:45		03:45	-	04:30		11:00	01:15	11:00	03:45	11:00	03:3
Vol.	15	4	760	-	739	-	1450	2	689	774	453	564	1142	129
					0.938									

Coconut Road Volume Study 41 to Via Vilagio #29856

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: #29856 41 to Via Vilagio #29856 Latitude: 0' 0.0000 Undefined

Start	03-Feb-1	6 We	estbound	Ea	stbound	Co	mbined	04-Feb	. W	estbound	Ea	stbound	Co	mbined
Time	Wed	A.M	P.M	A.M.	P.M.	A.M.	P.M.	Thu	A.M	P.M	A.M	P.M.	A.M.	P.M.
12:00		3	191	2 5	122	5	313		6	196	4	188	10	384
12:15		6	226	5	98	11	324		2	206	2	186	4	392
12:30		2	187	6	122	8	309		3	207	1	177	4	384
12:45		0	197	2	122	2	319		2	198	2	162	4	360
01:00		2	155	1	96	3	251		3	188	2	183	5	371
01:15		1	209	1	112	2	321		4	186	0	184	4	370
01:30		1	193	2	123	3	316		0	216	ő	176	Ö	392
		1		0		1	343		0	204	1	171	1	375
01:45			211	0	132		280							
02:00		3	176		104	3			1	188	1	170	2	358
02:15		2	17.0	0	117	2	287		1	180	0	185	1	365
02:30		1	171	0	115	1	286		2	185	0	166	2	351
02:45		2	180	2	129	4	309		0	162	0	186	0	348
03:00		1	181	0	108	1	289		0	152	0	147	0	299
03:15		1	187	0	134	1	321		0	169	0	204	0	373
03:30		0	203	0	132	0	335		3	186	0	184	3	370
03:45		2	185	1	127	3	312		0	170	2	195	2	365
04:00		4	174	1	138	5	312		4	172	2	194	6	366
04:15		2	176	2	148	4	324		1	205	2 5	191	6	396
04:30		5	189	1	118	6	307		5	180	3	211	-8	391
04:45		3	211	2	142	5	353		3	186	3	197	6	383
05:00		6	158	8	138	14	296		5	153	6	171	11	324
							322		7				10	
05:15		4	171	.3	151	7				181	3	191		372
05:30		14	144	8	129	22	273		9	153	9	169	18	322
05:45		22	131	7	110	29	241		20	131	13	194	33	325
06:00		15	159	10	114	25	273		23	154	10	166	33	320
06:15		32	139	19	98	51	237		26	123	21	122	47	245
06:30		55	127	29	82	84	209		46	133	32	104	78	237
06:45		69	109	65	74	134	183		73	131	66	104	139	235
07:00		87	86	37	64	124	150		72	105	26	96	98	201
07:15		122	82	44	59	166	141		98	82	30	54	128	136
07:30		143	58	65	55	208	113		145	80	61	78	206	158
07:45		167	71	57	42	224	113		158	80	63	59	221	139
08:00		150	68	69	46	219	114		165	77	75	72	240	149
08:15		176	61	58	41	234	102		174	67	58	57	232	124
08:30		161	58	59	31	220	89		133	54	73	53	206	107
08:45		149	48	74	29	223	77		147	56	75	55	222	111
09:00		142	51	72	26	214	77		134	58	84	68	218	126
09:00		135	48	62	31	197	79		145	63	116	45	261	108
						240				56	138			
09:30		167	39	73	31		70		164			28	302	84
09:45		144	31	97	26	241	57		175	36	119	33	294	69
10:00		156	43	106	19	262	62		146	30	162	31	308	61
10:15		155	25	104	14	259	39		183	25	140	30	323	55
10:30		168	24	104	11	272	35		187	13	158	20	345	33
10:45		180	11	112	4	292	15		204	12	1.97	13	401	25
11:00		165	10	108	10	273	20		184	13	200	17	384	30
11:15		181	8	122	3	303	11		195	4	173	6	368	10
11:30		199	2	108	2	307	4		190	8	178	6	368	14
11:45	_	196	6	143	2	339	8	_	209	7	189	6	398	13
Total		3402	5740	1851	3881	5253	9621		3457	5821	2503	5705	5960	11526
Day Tota			142		32		374			278		208		486
% Total		22.9%	38.6%	12.4%	26.1%	14	014		19.8%	33.3%	14.3%	32.6%	10	400
o Loral		22.070	30.0%	12.470	20.170				13.0%	33.376	14.370	32.070		
PRO CA	14	11:00	12:00	11:00	04:45	11:00	04:00	0-0	11:00	12:00	10:45	04:00	10:45	04:00
Peak		100												
Vol. P.H.F.	18	741 0.931	801 0.886	481 0.841	560 0.927	1222	1296 0.918	-	778 0.931	807 0.975	748 0.935	793 0.940	1521 0.948	1536 0.970

Coconut Road Volume Study East of Via Cocnut Point

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

Station ID: Site Code: #29737 Station ID: East of Via Cocnut Point #29737 Latitude: 0' 0.0000 Undefined

Start	01-Feb-16	Wes	tbound	Eas	tbound	Cor	mbined	02-Feb	We	stbound		stbound	Con	nbined
Time	Mon	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.	Tue	A.M.	P.M.	A.M.	P.M.	A.M.	P.M.
12:00		*-							1	198	2	175	3	37
12:15			4.		*	dei	*		4	178	3	176	7	35
12:30			*		*				0	162	3	171	3	33
12:45			178		153		331		0	197	4	167	4	36
01:00			153	*	150	*	303		2	155	3	179	5	33
01:15		4	161		156	+	317		ō	194	0	186	0	38
01:30		-	166		140	-	306		2	170	1	179	3	34
01:45			176	000	191	- 4	367		2	186	o	189	2	37
													2	
02:00			181	100	187		368		2	176	0	148	2	32
02:15			166		170		336		1	157	0	192	1	34
02:30			170	190	186		356		1	164	1	168	2	33
02:45			161		169		330		1	175	0	181	1	35
03:00			145		199		344		0	159	0	179	0	33
03:15			149		181		330		0	142	2	190	2	33
03:30			152	*	203	*	355		1	139	1	211	2	35
03:45			153		188		341		0	164	0	219	0	38
04:00			156	*	198	*	354		3	149	4	218	7	36
04:15			150		177		327		3	160	5	217	8	37
04:30			155	*	226		381		5	157	10	199	15	35
04:45			174		179	76	353		8	172	6	178	14	35
05:00			150		209	- 4	359			162	11	186	16	34
05:15			160		183		343		5 7	164	6	207	13	37
05:30			154		168		322		5	160	20	183	25	34
05:45					169		311		15	156	19	188	34	
06:00			142		153		281					165	32	34
				1.4		14			21	137	11			30
06:15			135		144		279		34	128	23	127	57	25
06:30			127		121		248		53	134	23	124	76	25
06:45			109		114	-	223		63	105	42	120	105	22
07:00			86		103		189		69	82	44	97	113	17
07:15			54	*	77	*	131		92	64	62	103	154	16
07:30			64		110	*	174		136	40	76	88	212	12
07:45			33		80		113		168	46	97	83	265	12
08:00			36		89		125		151	28	112	93	263	12
08:15		-	34	*	82	4.	116		152	33	98	-88	250	12
08:30			21		82		103		156	25	113	84	269	10
08:45			22		71		93		168	42	110	86	278	12
09:00			24	*	65		89		126	30	94	81	220	1
09:15			25	*	52	*	77		152	19	89	74	241	5
09:30			14		51		65		164	20	129	62	293	i
09:45			24		43		67		167	21	122	44	289	(
10:00			20		31		51		177	13	109	48	286	6
10:15		*	11		31		42		176	9	149	36	325	4
10:30			18		18		36		170	14	149	27	319	7
			9		11	- 4	20		174		156		330	
10:45										8		10	The second second	
11:00			14		11		22	_	185	8	1.48	13	333	3
11:15			3		5		8		182	4	175	8	357	3
11:30		*	5	(*)	8	*	13		174	5	163	8	337	3
11:45		*	2	1.0	7		9		189	5	162	3	351	
Total		0	4367	0	5341	0	9708		3367	5016	2557	6158	5924	1117
Day Total % Total		436 0.0%	67 45.0%	0.0%	41 55.0%	97	08	1	9.7% 83	83 29.3%	15.0%	15 36.0%	170	98
Peak	4	3	01:45		04:30		04:30	- 1	11:00	12:00	11:00	03:30	11:00	03:4
Vol.		-	693	-	797	-	1436	-	730	735	648	865	1378	148
P.H.F.			0.957		0.882		0.942		0.966	0.928	0.926	0.987	0.965	0.96

Coconut Road Volume Study East of Via Cocnut Point #29737

Trebilcock Consulting Solutions 1205 Piper Blvd, Suite 202 Naples, Florida 34110 Ph:239 566 9551 Fax:239 566 9553 email: ntrebilcock@trebilcock.biz

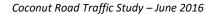
Station ID: Site Code: #29737 Station ID: East of Via Cocnut Point #29737 Latitude: 0' 0.0000 Undefined

Start	03-Feb-1		stbound		stbound		mbined	04-Feb		stbound		stbound		nbined
Time	Wed	A.M.	P.M	A.M.	P.M.	A.M.	P.M.	Thu	A.M.	P.M	A.M	P.M.	A.M.	P.M.
12:00		- 1	200	4	156	5	356		3	199	3	193	6	39:
12:15		4	195	5	176	9	371		2	186	4	199	6	38
12:30		d	176	8	158	9	334		2	196	3	171	5	36
12:45		0	191	4	151	4	342		0	171	1	155	1	32
01:00		1	133	1	151	2	284		2	194	2	185	4	37
01:15		0	191	1	146	1	337		5	162	1	193	6	35
01:30		0	156	1	181	1	337		o	188	0	205	0	39
01:45		1	159	2	176	3	335		1	175	1	174	2	34
02:00		7	162	ō	178	1	340		0	155	1	177	1	33
02:15		1	149	0	171	1	320		1	177	1	199	2	376
02:30		3	158	1	193	4	351		2	164	1	184	3	34
02:45		0	159	2	200	2	359		ō	142	1	206	1	34
03:00		1	144	1	200	2	344		0	154	o	190	Ö	34
03:15		1	172	2	196	3	368		0	149	2	213	2	363
03:30		ó	157	1	212	1	369		4	141	ō	214	4	355
03:45		3	168	4	200	4	368		1	158	2	202	3	360
04:00		4	145	2	224	6	369		4	162	3	189	3 7	35
04:15		2	159	4	190	6	349		0	163	6	194	6	35
04:30		4	151	5	226	9	377		5	148	6	213	11	36
04:45		7	182	5	203	12	385		5	173	7	198	12	37
05:00		6	146	15	219	21	365		5	145	10	200	15	345
05:15		5	171	4	206	9	377		7	181	4	204	11	38
05:30		18	158	17	200	35	358		9	173	21	174	30	34
05:45		20	139	15	157	35	296		20	158	18	189	38	
06:00		20	161	17	165	37	326		25	147	17	177	42	347
		30	121	25	162	55			29	135	27	135	56	
06:15							283							270
06:30		53	145	31	117	84	262		39	144	27	134	66	278
06:45		56	121	45	109	101	230		62	121	38	114	100	235
07:00		71	78	64	98	135	176		68	74	.53	124	121	198
07:15		106	72	62	125	168	197		88	81	55	85	143	166
07:30		148	38	86	91	234	129		125	53	90	91	215	144
07:45		176	39	100	86	276	125		173	65	95	92	268	15
08:00		138	42	92	92	230	134		165	50	106	109	271	159
08:15		149	44	97	81	246	125		172	47	102	85	274	133
08:30		151	28	87	99	238	127		146	35	116	78	262	110
08:45		171	24	112	61	283	85		145	39	101	88	246	127
09:00		134	18	95	59	229	77		170	27	113	100	283	127
09:15		145	23	111	67	256	90		179	27	132	65	311	97
09:30		172	21	101	53	273	74		205	29	148	49	353	71
09:45		164	15	127	54	291	69		201	23	118	47	319	7
10:00		175	13	139	51	314	64		169	21	152	42	321	63
10:15		139	17	149	21	288	38		196	22	136	30	332	5
10:30		160	19	115	15	275	34		204	10	157	24	361	3
10:45		178	6	150	11	328	17		207	9	174	19	381	2
11:00		171	6	177	10	348	16		181	9	172	17	353	2
11:15	_	178	6	152	6	330	12		181	3	181	8	362	1
11:30		184	3	171	4	355	7		190	4	190	4	380	- 3
11:45		193	2	171	5	364			194	6	172	5	366	1
Total		3346	4983	2577	6112	5923	11095		3592	5195	2770	6343	6362	11538
Day Total % Total		19.7%	329 29.3%	15.1%	35.9%	17	018	3	87 20.1%	787 29.0%	15,5%	113 35.4%	179	00
Peak	(2)	11:00	12:00	11:00	04:30	11:00	04:30		10:15	12:00	10:45	02:45	10:45	01:00
Vol.	14	726	762	671	854	1397	1504		788	752	717	823	1476	147
P.H.F.		0.940	0.953	0.948	0.945	0.959	0.977		0.952	0.945	0.943	0.961	0.969	0.939

Appendix G: 2014 FDOT Peak Season Factor Category Report (Excerpt)

(1 Sheet)

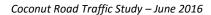
WEEK	DRY: 1252 BONITA SPRINGS DATES	SF	MOCF: 0.83 PSCF	2
	*********			***************
1	01/01/2014 - 01/04/2014	0.98	1.20	
2	01/05/2014 - 01/11/2014 01/12/2014 - 01/18/2014		1.12 1.06	
4	01/12/2014 - 01/16/2014		1.04	
5	01/26/2014 - 02/01/2014	0.84	1.02	
6	02/02/2014 - 02/08/2014		1.00	
7 8	02/09/2014 - 02/15/2014 02/16/2014 - 02/22/2014		0.99	
9.	02/23/2014 - 03/01/2014		0.96	
10	03/02/2014 - 03/08/2014	0.79	0.96	
11	03/09/2014 - 03/15/2014	0.78	0.95	
13	03/16/2014 - 03/22/2014 03/23/2014 - 03/29/2014		0.99	
14	03/30/2014 - 04/05/2014		1.02	
15	04/06/2014 - 04/12/2014		1.05	
16 17	04/13/2014 - 04/19/2014 04/20/2014 - 04/26/2014		1.09 1.13	
18	04/20/2014 - 04/26/2014		1.17	
19	05/04/2014 - 05/10/2014	1.00	1.22	
20	05/11/2014 - 05/17/2014		1.27	
21	05/18/2014 - 05/24/2014 05/25/2014 - 05/31/2014		1.32	
23	06/01/2014 - 06/07/2014		1.38	
24	06/08/2014 - 06/14/2014	1.16	1.41	
25	06/15/2014 - 06/21/2014		1.45	
27	06/22/2014 - 06/28/2014 06/29/2014 - 07/05/2014		1.46	
28	07/06/2014 - 07/12/2014		1.48	
29	07/13/2014 - 07/19/2014		1.49	
30	07/20/2014 - 07/26/2014 07/27/2014 - 08/02/2014		1.49 1.50	
32	08/03/2014 - 08/02/2014		1.50	
33	08/10/2014 - 08/16/2014	1.23	1.50	
34	08/17/2014 - 08/23/2014		1.51	
35	08/24/2014 - 08/30/2014 08/31/2014 - 09/06/2014		1.51 1.52	
37	09/07/2014 - 09/13/2014		1.52	
38	09/14/2014 - 09/20/2014	1.26	1.54	
40	09/21/2014 - 09/27/2014		1.48	
41	09/28/2014 - 10/04/2014 10/05/2014 - 10/11/2014		1.35	
42	10/12/2014 - 10/18/2014		1.30	
43	10/19/2014 - 10/25/2014		1.27	
44	10/26/2014 - 11/01/2014 11/02/2014 - 11/08/2014		1.24	
46	11/09/2014 - 11/15/2014		1.18	
47	11/16/2014 - 11/22/2014	0.95	1.16	
48	11/23/2014 - 11/29/2014		1.17	
50	11/30/2014 - 12/06/2014 12/07/2014 - 12/13/2014		1.18 1.18	
51	12/14/2014 - 12/20/2014	0.98	1.20	
52	12/21/2014 - 12/27/2014	0.92	1.12	
53	12/28/2014 - 12/31/2014	0.87	1.06	
	SEASON			
19-MAF	2-2015 16:07:48		830UPD	1_1252_PKSEASON.TXT



Appendix H: 2013 Lee County Link – Specific Volumes (Excerpt)

(1 Sheet)

PROAD SEGMENT VETERANS MEM. PKWY MCGREGOR BLVD DEL PRADO BLVD SANTA BARBARA BLVD SCHINE BLVD SCHINE BLVD SUMMER! N RD CADIOLUS DR BRANDY WINE CIR CYPRESS LAKE DR COLLEGE PKWY SUNSET VISTA	TO DEL PRADO BLVD SANTA BARBARA BLVD SKYLINE BLVD SR 78 GLADIOLUS DR BRANDYWINE CIR CYPRESS LAKE DR	DISTRIC 1&5	FNGTH	ROAD SERVICE	DI INES (PE	NK HOLIR DE	SERVICE VOLUMES (PEAK HOUR DEAK DIRECTION)	ĺ	SERVICEVO	SEBVICE VOLUMES (PEAK HOLIB-BOTH DIRECTIONS)	HOLIR-BOT	4 DIRECTION	8
Kwy	DEL PRADO BLVD SANTA BARBARA BLVD SKYLINE BLVD SR 78 GLADIOLUS DR BRANDYWINE CIR CYPRESS LAKE DR	18.5	(MILE)		8	o	٥	ш	A	8	O	۵	Е
	SANTA BARBARA BLVD SKYLINE BLVD SR 78 GLADIOLUS DR BRANDYWINE CIR CYPRESS LAKE DR	2	3.5	4LB 1,120	1,900	2,680	3,440	4,000	1,740	2,930	4,120	5,280	6,150
	SKYLINE BLVD SR 78 GLADIOLUS DR BRANDYWINE CIR CYPRESS LAKE DR		2.0	6LD 2,860	3,120	3,120	3,120	3,120	4,770	5,220	5,220	5,220	5,220
	SR 78 GLADIOLUS DR BRANDYWINE CIR CYPRESS LAKE DR	5	1.0	6LD 2,860	3,120	3,120	3,120	3,120	4,770	5,220	5,220	5,220	5,220
	GLADIOLUS DR BRANDYWINE CIR CYPRESS LAKE DR	ς,	3.5	4LD 1.830	2,080	2,080	2,080	2,080	3,060	3,460	3,460	3,460	3,460
GLADIOLUSDR BRANDYWINE CIR CYPRESSLAKE DR COLLEGE PKWY SUNSET VISTA	BRANDYWINE CIR CYPRESS LAKE DR	4	0.4	H	0	0	950	1,520	0	0	0	1,620	2,590
BRANDYWINE CIR CYPRESS LAKE DR COLLEGE PKWY SUNSET VISTA	CYPRESSLAKE DR	4	6.0	2LN 0	790	920	920	920	0	1,340	1,570	1,570	1,570
CYPRESSLAKE DR COLLEGE PKWY SUNSET VISTA		4	6.0	2LN 0	790	920	920	920	0	1,340	1,570	1,570	1,570
COLLEGE PKWY	COLLEGE PKWY	4	0.7	4LD 0	0	830	1,800	1,800	0	0	1,410	3,040	3,040
SUNSET VISTA	SUNSET VISTA	4	0.5	2LN 0	810	840	840	840	0	1,380	1,430	1,430	1,430
	McGREGOR BLVD	4	0.8	2LN 0	810	840	840	840	0	1,380	1,430	1,430	1,430
	SERVICE VOLUMES ON COLLECTORS IN LEF COUNTY (2013 DATA)	ON COLLEC	TORS IN I	SOLINTY	013 DATA	5							
		TRAFFIC	LENGTH	ROAD SERVICE	OLUMES (PE	IK HOUR PE	SERVICE VOLUMES (PEAK HOUR PEAK DIRECTION)		SERVICE VO	SERVICE VOLUMES (PEAK HOUR-BOTH DIRECTIONS)	HOUR-BOT	4 DIRECTION	(S)
ROAD SEGMENT FROM	10	DISTRIC	(MILE)	TYPE	m	O	Q	ii.	4	80	0	٥	ш
				2רח 0	0	550	860	860	0	0	066	1,530	1,530
				2LD 0	0	280	910	910	0	o	1,040	1,610	1,610
				4LU 0	0	1,240	1,700	1,700	0	0	2,200	3,030	3,030
				4LD 0	0	1,310	1,790	1,790	0	0	2.340	3,190	3,190



Appendix I: Lee County 2040 LRTP (Excerpts)

(3 Sheets)

	ADWAY PROJECTS			
ID	Facility	From	То	Improvemen
1	1-75	at 5	SR 884	Intersection
2	San Carlos Boulevard	Reconstruction/Transit, Pedest	trian, and Capacity Improvements	Capacity
3	SR 78	W. of Santa Barbara	East of Pondella	Widening
4	I-75	at Corks	screw Road	Intersection
5	Ortiz Avenue	Martin Luther King	Luckett Road	Widening
6	SR 82	Alabama	Homestead	Widening
7	US 41 Veterans		els Parkway ta Barbara	Intersection Intersection
9	SR 82		onial Blvd	Intersection
10	Old US 41	Bonita Beach Road	Collier Co, Line	Widening
11	Colonial	at Su	mmerlin	Intersection
12	SR 78	Business 41	1-75	Widening
13	SR 82	Shawnee	Alabama	Widening
14	Pine Island Road	Del Pine Dr	Hancock Creek Blvd (NE 24th Ave)	Widening
15	SR 78	Chiquita Boulevard	w/o Santa Barbara	Widening
16	SR 78	24th Ave	US 41	Widening
17	Estero	Lee Blvd	ment 4 Bell Blvd	Resurfacing
18 19	Leeland Heights Boulevard Ortiz Avenue	Colonial Blvc	SR 82 (MLK)	Widening Widening
20	Corkscrew Road	US 41	e/o Ben Hill Griffin Pkwy	Widening
21	Hanson Street	Evans Avenue	Veronica Shoemaker	Widening
22	SR 80	SR 31	Buckingham Rd	Widening
23	Estero	Seg	ment 5	Resurfacing
24	Estero	Seg	ment 6	Resurfacing
25	Lee Blvd	Leeland Heights Blvd	SR 82	Widening
26	Daniels Parkway	Chamberlin Pkwy	Gateway Blvd	Widening
27	Bonita Beach Road	1-75	Bonita Grande Drive	Widening
28	Homestead Road SR 82	Milwaukee	Sunrise way/Gunnery Road	Widening
29 30	Luckett Road	Ortiz Avenue	I-75	Intersection Widening
31	Littleton Road	NE 24TH	Business 41	Widening
32	Daniels Parkway	Gateway Blvd	SR 82	Widening
33	Hanson Street	US 41	Fowler St.	Widening
34	Gunnery Road	Lee Blvd	Buckingham Rd	Widening
35	Metro Parkway	Daniels Parkway/Gunnery Road	South of Winkler Avenue	Widening
36	Homestead Road	Sunrise	Alabama	Widening
37	Corkscrew Road	Ben Hill Griffin	Alico Road	Widening
38	Ortiz Avenue	Luckett Road Bonita Grande Drive	SR 80 West Imperial Parkway	Widening
39 40	Terry Street SR 31	SR 80	Charlotte Co. Line	Widening Widening
41	Leonard Boulevard	Lee Blvd	Gunnery Rd	Widening
42	SR 82	Homestead	Hendry County Line	Widening
43	Three Oaks Ext.	North of Alico Road	Daniels Parkway	New Roadway
44	23rd Street SW	Gunnery Rd	Beth Stacey Blvd	Widening
45	Beth Stacey Boulevard	23rd St SW	Homestead Rd	Widening
46	Chiquita Boulevard	Pine Island Road	Cape Coral Parkway	Widening
47	Alabama Street	SR 82	Homestead Rd	Widening
48	Livingston/Imperial Parkway	Collier Co. Line	Bonita Beach Road	Widening
49 50	Orange River Road Sunshine Boulevard	Buckingham Rd SR 82	SR 80 Lee Blvd	Widening Widening
51	Buckingham Road	Orange River Blvd.	SR 80	Widening
52	Joel Boulevard	17th St	Palm Beach Blvd	Widening

la Circle	ROPOLITAN PLANNING ORGANIZATION			
D D	ADWAY PROJECTS	From	То	Improvement
53	Facility Bell Boulevard	SR 82	Leeland Heights Blvd	Improvement Widening
54	Winkler Road	Gladiolus Drive	Cypress Lake Drive	Widening
55	Crystal Drive	US 41	Metro Pkay	Resurfacing
56	Edison Avenue	US 41	Fowler St	Widening
57	Cape Coral Bridge			Replace Bridge
58	Fowler Street	Metro/Fowler	SR 82	Resurfacing
59	Veronica Shoemaker	Michigan Avenue	SR 80	Widening
60	Burnt Store Road	Van Buren Parkway	Charlotte Co. Line	Widening
61	SR 82	Michigan Avenue	Ortiz Avenue	Widening
62 63	Little Pine Island Bridge Alico Road	Ben Hill Griffin	Airport Haul Road	Replace Bridge Widening
64	2nd Street	Fowler St	Palm Beach Blvd	Widening
65	Andalusia Boulevard	Pine Island Road	Tropicana Parkway	Widening
66	Andalusia Boulevard	Jacaranda Parkway	Kismet Parkway	New Roadway
67	Kismet Parkway	NW 18th Avenue	Chiquita Boulevard	Widening
68	NE 24th Avenue	Pondella Road	Garden Boulevard	Widening
69	I-75	at Bonita Beach Rd		Intersection
70	Little Carlos Pass Bridge			Replace Bridge
71	1st Street	Fowler St	Palm Beach Blvd	Widening
72	SR 78	US 41	Business 41	Widening
73 74	Bonita Grande Drive Littleton Road	Terry Street US 41/N. Tamiami Trl	Bonita Beach Road SR 78	Widening New Roadway
75	North Airport Rd Extension	Metro Parkway	Plantation Road	New Roadway
76	1-75	Collier Co. Line	s/o Caloosahatchee Bridge	New Roadway
77	1-75	at Daniels I		Intersection
78	Big Carlos Bridge			Replace Bridge
79	Tropicana Parkway	Chiquita Boulevard	Nelson Road	Widening
80	Nelson Road North	Embers Parkway	Tropicana Parkway	Widening
81	Luckett Road ext.	Sunshine Blvd	Hendry County Line	New Roadway
82	Del Prado Extension	US 41	1-75	New Roadway
83	Del Prado Extension	e/o US 41	e/o Prarie Pines	New Roadway
84 85	Luckett Road Big Hickory Pass Bridge	1-75	12th St	Widening Replace Bridge
86	New Pass Bridge			Replace Bridge
87	Sandy Lane Extension	Strike Lane	Pelican Colony	New Roadway
88	East West	Ben Hill Griffin	Airport Haul Road	New Roadway
89	Luckett Road Extension	Buckingham Rd	Gunnery Rd	New Roadway
90	Luckett Road Extension	Gunnery Rd	Sunshine Blvd	New Roadway
91	Homestead Road	SR 82	Milwaukee	Widening
92	Alico Road	Airport Haul Road	Alico Connector	Widening
93	Kismet Parkway	Burnt Store Road	El Dorado Parkway	Widening New Readway
94	NE 24th Avenue	Garden Boulevard	Del Prado Boulevard	New Roadway
95 96	Surfside Boulevard Del Prado Extension	Trafalgar Parkway 1-75	Pine Island Road SR 31	New Roadway New Roadway
97	Luckett Road ext.	e/o I-75	Buckingham Rd	New Roadway
98	Alico Connector	Alico	SR 82	New Roadway
99	Garden Boulevard	North of DeNavarra Parkway	NE 23rd Place	Widening
.00	Hanson Extension	Veronica Shoemaker	Ortiz Avenue	New Roadway
.01	Del Prado Extension	Mellow Dr	1-75	New Roadway
.02	Jacaranda Parkway	Old Burnt Store Road	Burnt Store Road	New Roadway
.03	CR 951 Extension	Corkscrew Road	Alico Road	New Roadway

2040 TRANSPORTATION PLAN LEE COUNTY MPO

CHAPTER 6: THE 2040 COST FEASIBLE PLAN

DEFINING THE 2040 COST FEASIBLE PLAN

Note: Cost Feasible Plan projects are presented in year of expenditure (YOE).

Projects included in the Cost Feasible Plan were selected based on their performance against the established set of evaluation criteria. The best performing projects for each mode were then balanced against the revenues forecasted over the next 25 years and vetted for public opinion to arrive at the Cost Feasible Plan. Project size and geography were also considered.

The Cost Feasible Plan reflects approximately \$3 billion (YOE) worth of implementable projects. Improvements between

2015 and 2020 are considered committed projects, as they are already funded in the work program. These projects are included in the Existing Plus Committed list. All Cost Feasible Plan projects are listed in this chapter beginning on page 63.

ROAD/HIGHWAY PROJECTS

Figure 6-1 shows the cost feasible road and highway projects. The major road projects included in the Cost Feasible Plan support economic growth, provide for a balanced multimodal transportation network, and improve the safety and security for the Lee County community. The Cost Feasible Plan includes \$2 billion (YOE) in road expansion projects. Highlights of the proposed Cost Feasible road projects include:



Appendix J: Potential Development - ITE Period Analysis Reports

(6 Sheets)

ID #	STRAP#	PARCEL NAME	DEVELOPMENT PARAMETERS	ZONING APPROVALS NOT YET CONSTRUCTED	POTENTIAL FUTURE CONSTRUCTION PERMITTING
1	07-47-25-B2-00004.00CE	Pelican Landing Community Association	Marina – 20 berths; Restaurant – 3,000sf		X
2A	07-47-25-B2-00003.0370	Estero Bay Marine LLC	Residential – Multi Family – 360 dwelling		X
2B	07-47-25-B2-00003.0000	(Weeks Fish Camp)	units – high-rise buildings		^
3	07-47-25-B2-00000.0010	WCI/Pelican Landing DRI (Raptor Bay)	Residential – Multi Family – 360 dwelling units – high-rise buildings		X
4	06-47-25-00-00002.0030	Pelican Landing Timeshare Ventures LP	Timeshare – Rental Townhouses – 267 dwelling units	X	
5	17-47-25-B1-00001.012A	WCI Communities Inc (Altaira High Rise)	High-Rise Residential Condominium/ Townhouse – 76 dwelling units	X	
6	17-47-25-B1-U1681.1891	WCI Communities Inc (Two Future High Rises)	Residential – Multi Family – 150 dwelling units – high-rise buildings	X	
7	08-47-25-01 +	Eldorado Acres Subdivision	Residential – Single Family – 98 dwelling units – platted lots	X	
8	08-47-25-00-00003.0030	John T. Watson	Residential – Single Family – 2 dwelling units	X	
9	08-47-25-01-00016.0000	Judy K. Doyle	Residential – Single Family – 15 dwelling units	X	
10	08-47-25-00-00004.0000	Dhaliwal + J/T	Residential – Single Family –	Х	
11	08-47-25-E2-U1757.2005	Dhaliwal + J/T	3 dwelling units	X	
12	09-47-25-E1-U1823.2024	Dewane/Docese of Venice	Residential – Multi Family – 93 dwelling units		X
13	09-47-25-E4-U1882.1994	Coconut Road Associates LLC	General Office Building – 122,484sf	X	
14	09-47-25-E1-U1874.2023	HG Coconut LLC	Charaine Carter 210 000ef		
15A	09-47-25-E1-U1877.2039		Shopping Center – 210,000sf Medical Office Building – 40,000sf		
15B	09-47-25-E2-U1900.2033	OBE Florida CRE Holdings LLC	Multi-Family Residential – 525 dwelling units Assisted Living – 152 beds	X	
15C	09-47-25-E2-U1902.2012	, Holdings EEC	Hotel – 130 Rooms		
16	09-47-25-E3-31000.0050	Allsee Investment LP	General Office Building – 27,500sf	X	
17	09-47-25-E3-31000.0010	Naples Diagnostic Imaging	General Office Building – 15,000sf	X	
22A	09-47-25-E3-373A1.0000		Acute Care Hospital – 160 beds		
22B	09-47-25-E3-373A2.0000	Lee Memorial Health	Shopping Center – 60,000sf	X	
22C	09-47-25-E3-373A3.0010	System	Medical Office Building – 198,000sf General Office Building – 102,000sf		

Project Name: Coconut F Date: 3/10/2016 State/Province: Country:			No: City: Zip/Post Client Na				
Analyst's Name:			Edition:		ITE-TGM 9th E	dition	
LAND USE	SIZE	DA	ILY	AM PEA	K HOUR	PM PEA	KHOUR
		Entry	Exit	Entry	Exit	Entry	Exit
420 - Marina	20 (1)	30	29	1	1	2	2
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		30	29	1	1	2	2
932 - High-Turnover (Sit-Down) Restaurant	3 (2)	191	190	18	12	18	12
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		191	190	18	12	18	12
Total		221	219	19	13	20	14
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		221	219	19	13	20	14

roject Name: ate: tate/Province: ountry: nalyst's Name:	Coconut - 2 3/10/2016		No: City: Zip/Posi Client N Edition:	al Code: ame:	ITE-TGM 9th	Edition	
LANDUG	E SIZE	. DA	dLY	AM PEA	AK HOUR	PM PEA	KHOUR
LAND US	E SIZE	Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condomini	um/Townhouse 360	980	980	24	120	115	57
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		980	980	24	120	115	57
Total		980	980	24	120	115	57
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		980	980	24	120	115	57

roject Name:	Coconut - 3		No:				
Date:	3/10/2016		City:				
State/Province:			Zip/Pos	tal Code:			
Country:			Client N	ame:			
Analyst's Name:			Edition:		ITE-TGM 9th	Edition	
LAND USE	SIZE	DA	AILY	AM PEA	AK HOUR	PM PEA	CHOUR
D-110 00E	OLL	Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condominium/	Townhouse 360 (1	980	980	24	120	115	57
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		980	980	24	120	115	57
Total		980	980	24	120	115	57
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		980	980	24	120	115	57

roject Name:	Coconut - 4		No:		
Date:	3/10/2016		City:		
State/Province:			Zip/Postal Code:		
Country:			Client Name:		
Analyst's Name:			Edition:	ITE-TGM 9th Edition	
LAND USE	SIZE	AM	PEAK HOUR	PM PEAK	HOUR
	O lacta	Entry	Exit	Entry	Exit
224 - Rental Townhouse	267 (1)	62	125	98	94
Reduction		0	0	0	0
Internal		0	0	0	0
Pass-by		0	0	0	0
Non-pass-by		62	125	98	94
Total		62	125	98	94
Total Reduction		0	0	0	0
Total Internal		0	0	0	0
Total Pass-by		0	0	0	0
Total Non-pass-by		62	125	98	94

Project Name:	Coconut - 5			No:				
Date:	3/10/2016			City:				
State/Province:				Zip/Postal Code):			
Country:				Client Name:				
Analyst's Name:				Edition:		ITE-TGM 9th Edit	ion	
LAND	USE	SIZE	D	AILY	AM PEA	AK HOUR	PM PEA	K HOUR
			Entry	Exit	Entry	Exit	Entry	Exit
232 - High-Rise Residential Co	ndominium/Townhouse	76 (1)	159	159	5	21	18	11
Reduction			0	0	0	0	0	0
Internal			0	0	0	0	0	0
Pass-by			0	0	0	0	0	0
Non-pass-by			159	159	5	21	18	11
Total			159	159	5	21	18	11
Total Reduction			0	0	0	0	0	0
Total Internal			0	0	0	0	0	0
Total Pass-by			0	0	0	0	0	0
Total Non-pass-by			159	159	5	21	18	11

Project Name:	Coconut - 6		No:				
Date:	3/10/2016		City:				
State/Province:			Zip/Posta	l Code:			
Country:			Client Na	me:			
Analyst's Name:			Edition:		ITE-TGM 9th E	dition	
LAND USE	SIZE	DA	JLY	AM PEA	K HOUR	PM PEA	KHOUR
2412002	5 Hz L	Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condominium/T	ownhouse 150 (1)	458	457	56	28	56	28
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		458	457	56	28	56	28
Total		458	457	56	28	56	28
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		458	457	56	28	56	28

Project Name:	Coconut - 7		No:				
Date:	3/11/2016		City:				
State/Province:			Zip/P	ostal Code:			
Country:			Clien	t Name:			
Analyst's Name:			Editio	on:	ITE-TGM 9th	Edition	
LAND USE	SIZE	DA	ILY	AM PEA	KHOUR	PM PEA	KHOUR
17410 001	OIZE	Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	98 (1)	516	515	20	58	21	60
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		516	515	20	58	21	60
Total		516	515	20	58	21	60
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		516	515	20	58	21	60

Project Name: Co	conut - 8		No:				
Date: 3/1	11/2016		City:				
State/Province:			Zip/Pos	al Code:			
Country:			Client N	ame:			
Analyst's Name:			Edition:		ITE-TGM 9th	Edition	
LAND USE	SIZE	DA	ILY	AM PEA	KHOUR	PM PEA	KHOUR
		Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	2 (1)	10	9	1	1	1	1
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		10	9	1	1	1	1
Total		10	9	1	1	1	1
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		10	9	1	1	1	1

Project Name:	Coconut - 9		No:				
Date:	3/11/2016		City:				
State/Province:			Zip/Po	stal Code:			
Country:			Client	Name:			
Analyst's Name:			Edition	:	ITE-TGM 9th	Edition	
LAND USE	SIZE	DA	JLY	AM PEA	AK HOUR	PM PEA	KHOUR
2712 002	O.L.L	Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	15 ⁽¹⁾	92	91	5	15	12	7
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		92	91	5	15	12	7
Total		92	91	5	15	12	7
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		92	91	5	15	12	7

Project Name:	Coconut - 10 & 11		No:				
Date:	3/11/2016		City:				
State/Province:			Zip/Post	al Code:			
Country:			Client N	ame:			
Analyst's Name:			Edition:		ITE-TGM 9th	Edition	
LAND USE	SIZE	DA	JLY	AM PEA	KHOUR	PM PEA	KHOUR
DATE	OLL	Entry	Exit	Entry	Exit	Entry	Exit
210 - Single-Family Detached Housing	3 (1)	15	14	1	1	2	1
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		15	14	1	1	2	1
Total		15	14	1	1	2	1
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		15	14	1	1	2	1

roject Name: Coconut -	12		No:				
Date: 3/11/2016			City:				
State/Province:			Zip/Posta	I Code:			
Country:			Client Nar				
Analyst's Name:			Edition:		ITE-TGM 9th I	Edition	
LAND USE	SIZE	DA	ILY	AM PEA	KHOUR	PM PEA	KHOUR
2712.002		Entry	Exit	Entry	Exit	Entry	Exit
230 - Residential Condominium/Townhouse	93 /11	302	302	8	41	38	19
Reduction	nium/Townhouse 93 "		0	0	0	0	0
Internal	ndominium/Townhouse 93 ⁷¹¹		Q	0	0	0	0
Pass-by				0	0	0	0
Non-pass-by		302	302	8	41	38	19
Total		302	302	8	41	38	19
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		302	302	8	41	38	19

Project Name:	Coconut - 13			No:			
Date:	3/11/2016			City:			
State/Province:				Zip/Postal Code:			
Country:				Client Name:			
Analyst's Name:				Edition:	ITE-TGM	9th Edition	
LAND USE	SIZE	DA	dLY	AM PEAI	KHOUR	PM PE	K HOUR
		Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	122.48 (1)	766	766	198	27	37	179
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		766	766	198	27	37	179
Total		766	766	198	27	37	179
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		766	766	198	27	37	179

Project Name:	Coconut - 16			No:			
Date:	3/11/2016			City:			
State/Province:				Zip/Postal Code:			
Country:				Client Name:			
Analyst's Name:				Edition:	ITE-TGM	9th Edition	
LAND USE	SIZE	DA	ILY	AM PEA	KHOUR	PM PEA	KHOUR
LAND OOL	OILL.	Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	27.5 (1)	246	246	60	8	19	90
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		246	246	60	8	19	90
Total		246	246	60	8	19	90
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		246	246	60	8	19	90

Project Name:	Coconut - 17			No:			
Date:	3/11/2016			City:			
State/Province:				Zip/Postal Code:			
Country:				Client Name:			
Analyst's Name:				Edition:	ITE-TGM	th Edition	
LAND USE	SIZE	DA	JLY	AM PEA	KHOUR	PM PEA	K HOUR
		Entry	Exit	Entry	Exit	Entry	Exit
710 - General Office Building	15 ⁽¹⁾	155	155	37	5	16	79
Reduction		0	0	0	0	0	0
Internal		0	0	0	0	0	0
Pass-by		0	0	0	0	0	0
Non-pass-by		155	155	37	5	16	79
Total		155	155	37	5	16	79
Total Reduction		0	0	0	0	0	0
Total Internal		0	0	0	0	0	0
Total Pass-by		0	0	0	0	0	0
Total Non-pass-by		155	155	37	5	16	79

Appendix K: FDOT Traffic Online - T24 Factors(1 Sheet)

FDOT Florida Traffic Online Identify Information

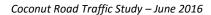
Page 1 of 1

Feature	1
Site	124490
Description	COCONUT RD, E OF SR 45/US 41 LC 490
Section	12000119
Milepoint	0.04
AADT	11100
Site Type	Portable
Class Data	Yes
K Factor	10.3
D Factor	52
T Factor	3.1
TRAFFIC	REPORTS (provided in 2 format)
Lee County	Annual Average Daily Traffic
	Annual Vehicle Classification
	Historical AADT Data
	Synopsis 124490CL-20140611
	Vehicle Class History

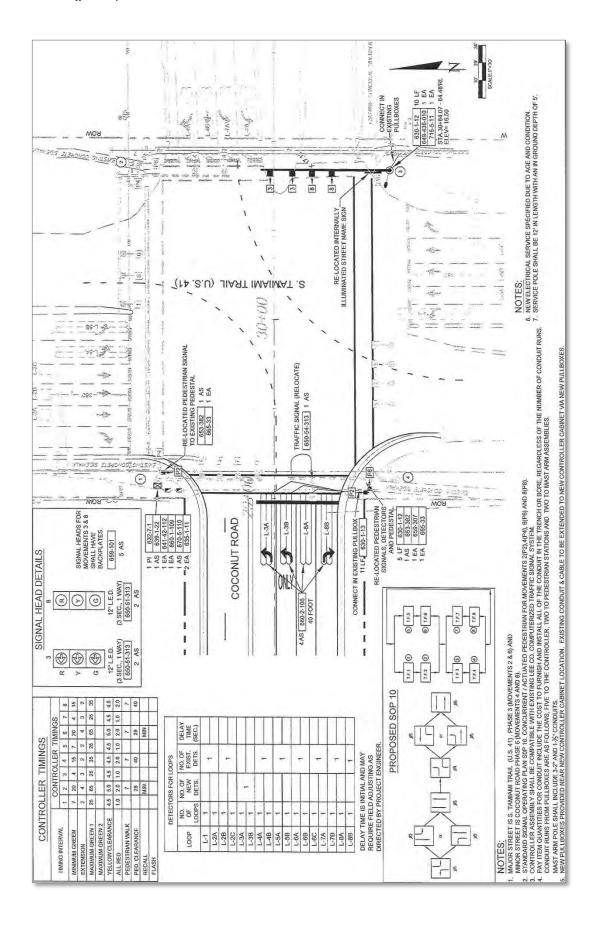
FDOT Florida Traffic Online Identify Information

Page 1 of 1

Feature	1
Site	120065
Description	SR 45/US 41, NORTH OF CR 887/OLD US 4 1 LC436
Section	12010000
Milepoint	4.45
AADT	48000
Site Type	Portable
Class Data	Yes
K Factor	10.05
D Factor	54.6
T Factor	3.1
TRAFFIC	REPORTS (provided in 🏂 format)
Lee County	Annual Average Daily Traffic
	Annual Vehicle Classification
	Historical AADT Data
	Synopsis 120065CL-20140402
	Vehicle Class History



Appendix L: Lee County – Intersection Signal Data(10 Sheets)



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Lee County, FL

409 - US 41 & Coconut Rd - - Econolite Type - ASC3

Coordination Pattern Data Pattern Data (MM)3-2

Pattern - 33

Split Pattern	33	TS2 (Pat-Off)	10-3	Splits in	Percent
Cycle	130	Sid (COS)	333	Offsets in	Percent
Offset Value	43%	Dwell/Add Time	0		
Actuated Coord	Yes	Timing Plan	0		
Actuated Walk Rest	No	Sequence	11		
Pliase Reservice	No	Action Plan	0		
Max Select	None	Force Off	None		

Split Preference Phases

Phase	1	2	3	4	5	- 6	7	8	9	10	11	12	13	14	15	16
Description	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB				7				
Splits (Split Pat 33)	12	50	15	23	12	50	19	19	0	0	0	0	0	0	0	0
Preference 1	0	0	D	0	0	0	0	0	0	0	0	0	0	0	0	0
Preference 2	0	0	D	0	0	- 0	0	0	.0	0	0	0	0.	0	0	0

Ring	1	2	3	4	Misc. Data					
Ring Split Ext	0	0	0	0	Veh. Permissive I	0	Veh. Permissive 2	0	Veh. Permissive 2 Disp.	0
Ring Disp.	1	0	D	0	Split Demand Pat 1	0	Split Demand Pat 2	D	Crossing Arterial Pat:	0
Split Sum	100%	100%	0%	0%						

Split Pattern Data

Phase	1	2	3	4	- 5	- 6	7	8	9	70	11	12	13	14	15	. 16
Coordinated Phases		X				X										
Vehicle Recalls			1				1.1.1				-10			1		. =
Ped Recalls						-										
Max Recalls		X				X										
Phase Omit					-		10 10 10		X	X	X	Х	X	Х	Х	X
Special Function Output	1				A . A											

file:///C:/Users/padgetms/AppData/Roaming/Econolite/0/PrintAll.html

2/18/2016

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Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice	34 130 35% Yes No		Std (C Dwell/ Timing Seque Action	Add Tim Plan nce Plan	19	11 34 0 0	4		Splits i			cent:				
Max Select Split Preference Ph	None ases		Force	Off		No	ine									
Phase Description	NB	LT SE	_		5 SBLT	NB.	VBLT	EB B	9	10	11	12	13	14	15	16
Splits (Split Pat 34) Preference 1 Preference 2	1	0	13 D	0 0	0	54 0	17 0	0 0	0	0	0	0	0	0	0	0
Ring 1 Ring Split Ext 0 Ring Disp. : Split Sum 1009	0	3 0 0	0 0%			Data Termissi ernand			Ven Pe Split De				Veh. Pe Crossing		e 2 Disp al Pat	. 0 0
Split Pattern Data Phase		2	3	-4	5	6	7.7	8	9	10	11	12	13	14	15	16
Coordinated Phases Vehicle Recalls		Х				X										
Ped Recalls				-		-										
Max Recalls		X			-	X										+ = 1
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice	36 130 30% Yes No	X	Std (C Dwell) Timing Seque Action	Add Tim Plan nce Plan	ne	11- 31 0 0	2		X Splits i			cent	1.8	х	X	x
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Phe Phase Description Splits (Split Pat 36) Preference 1	36 130 30% Yes No No None	2 LT SB2 50	Std (C Dwell/ Timing Seque Action Force 3 EBLT 14	OS) Add Tirr Plan nce Plan Off 4 WB 24 D	5 SBLT 12 0	11. 31. 0 0 1 0 No	7 WBLT 20 0	8 EB 18	Splits i	10 0	Per Per	rcent rcent	13	14 0 0	15	16 0 0
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 36) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5 Ring 1 R	36 130 30% Yes No No None	2 2 500 0 0 0 0 0 0 0 0	Std (C Dwell/, Timing Seque Action Force 3 EBLT 14 0 0	OS) Add Tirr p Plan nce Plan Off 4 WB 24 0	5 SBLT 12 0 0 Wisc. I	111. 311. 0 0 No No	7 WBLT 20 0 0	8 EB 18 0	Splits i Offsets	10 0 0	Per Per	12	13 0 0	14 0 0 0	15 0 0 0	16 0 0
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph. Phase Description Splits (Split Pat 36) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5plit Ext 0 Ring Disp Split Sum 100° Split Sum 100° Split Pattern Data Phase	36 130 30% Yes No No None ases	2 2 500 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (C Dwell/, Timing Seque Action Force 3 EBLT 14 0 0	OS) Add Tirr p Plan nce Plan Off 4 WB 24 0	5 SBLT 12 0 0 Wisc. I	111.311.311.00 0 0 No.	7 WBLT 20 0 0	8 EB 18 0	Splits i Offsets 9 0 0 0 Vén. Pe	10 0 0	Per Per	12	13 0 0	14 0 0 0	15 0 0 0	16 0 0 0
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 36) Preference 1 Preference 2 Ring 1 Ring 9 Ring 1 Ring Split Ext 0 Ring Disp Split Sum 100° Splits Tattern Data Phase Coordinated Phases Vehicle Recalls	36 130 30% Yes No No None ases NB 1 0 0	2 S88 S S S S S S S S S S S S S S S S S	Std (C Dwell/ Timing Seque Action Force 3	OS) Add Tim Plan nce Plan Off 4 WB 24 D 0	SBLT 12 0 0 Misc. I Veh. P Split D	11.311 0 0 0 1 0 No	7 WBLT 20 0	8 EB 18 0 0	Splits i Offsets 9 0 0 0 Vén. Pr	10 0 0 0	Per	12	13 0 0 0	0 0 0 0	15 0 0 0	0 0 0 0
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Phe Phase Description Splits (Split Pat 36) Preference 1 Preference 2 Ring 1 Ring 5plit Ext 0 Ring 15plit Ext 0 Ring 15plit Sum 100° Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Max Recalls	36 130 30% Yes No No None ases NB 1 0 0	2 2 500 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (C Dwell/ Timing Seque Action Force 3	OS) Add Tim Plan nce Plan Off 4 WB 24 D 0	SBLT 12 0 0 Misc. I Veh. P Split D	111.311.311.00 0 0 No.	7 WBLT 20 0	8 EB 18 0 0	Splits i Offsets 0 0 0 0 Vén. Pr	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Per	12 0 0 0	13 Q Q Q Orossing	0 0 0 0	0 0 0 0	16 0 0 0
Phase Omit Special Function Output Pattern - 36 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Wafs Resf Phase Reservice Max Select Split Preference Ph. Phase Description Splits (Split Pat 36) Preference 1 Preference 2 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp Split Sum 1000 Split Pattern Data Phase Coordinated Phases Coordinated Phases Coordinated Phases Vehicle Recalls Ped Recalls	36 130 30% Yes No No No None ases 1 1 0 0 0 0 7	2 S88 2 500 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (C Dwell/ Timing Seque Action Force 3	OS) Add Tim Plan nce Plan Off 4 WB 24 D 0	SBLT 12 0 0 Misc. I Veh. P Split D	111. 311. 0 0 1 1 0 No. No. No. No. No. No. No. No. No. No.	7 WBLT 20 0	8 EB 18 0 0	Splits i Offsets 9 0 0 0 Vén. Pr	10 0 0 0	Per	12	13 0 0 0	0 0 0 0	15 0 0 0	0 0 0 0

Pattern - 37 Split Pattern Cycle Offset Value Actuated Coord	37 130 49% Yes		3td (C	Add Tim	19	12- 313 0			Splits in		Pen	cent cent				
Actuated Walk Rest Phase Reservice Max Select	No No None		Seque Action Force	Plan		n No	ne									
Phase Description	_	LT SE		4 WB	5 SBLT	6 NB	7 WBLT	B	9	10	11	12	13	14	15	16
Splits (Split Pat 37) Preference 1 Preference 2		5 52 0 0 0 0	D	21 0 0	12 0 0	55 0 0	15 0 0	18 0 0	0 0	0	0	0	0	0 0	0 0	0
Ring 11 Ring Split Ext 0 Ring Disp. Split Sum 100	0	3 0 0 6 0%	0 0%			Data ermissiv ernand			Ven Pe Split De				/eh. Pe Crossing		e 2 Disp al Pat	o. 0 0
Split Pattern Data	Tr.	1 2	3	1 4	5	6	7	8	g	10	11	12	13	14	15	16
Coordinated Phases Vehicle Recalls	Ŧ	X	_			X										
Ped Recalls Max Recalls Phase Omit		X				Х			х	x	х	×	×	x	x	x
Pattern - 41 Split Pattern Cycle Offset Value Actuated Walk Rest Phase Reservice Max Select	41 150 38% Yes No No No		Std (C	Add Tim Plan Ince Plan	ne	13- 411 0 0 1 0 No			Splits in		Pen					
Pattern - 41 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference PI Phase Description Splits (Split Pat 41)	150 38% Yes No No None nases	LT SE	Std (O Dwell/ Timing Seque Action Force	Add Tirr p Plan ince Plan Off 4	5 SBLT 15	0 0 0 No No	ne 7 Welt 24	8 EB 18	Offsets 9	10 0	Peri	12 0	13 Q	14	15	16
Pattern - 41 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference PI Phase Description	150 38% Yes No No None	LT SE	Std (O Dwell/ Timing Seque Action Force	Add Tirr p Plan ince Plan Off	5 SBLT	0 0 1 0 No	ne 7	EB	Offsets	in 10	Peri	cent 12	. 1		1	
Pattern - 41 Split Pattern Cycle Offset Value Actuated Coord Actuated Walik Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 41) Preference 1 Preference 2 Ring 1 Ri	150 38% Yes No None nases	SE 5 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (C Dwelf/ Timing Seque Action Force	Add Tirr p Plan ince Plan Off 4 WB 24 D	5 SBLT 15 0 0 Misc. E Veh. P	6 NB 43 0	7 WBLT 24 0	EB 18 0 0	9 0	10 0 0	11 a a a a	12 0 0	0	0 0	0 0 0	0 0 0
Pattern - 41 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 41) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp. 1	150 38% Yes No None nases	SE 5 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (C Dwelf/ Timing Seque Action Force 3 EBL1 18 0 0	Add Tirr p Plan ince Plan Off 4 WB 24 D	5 SBLT 15 0 0 Misc. E Veh. P	411 0 0 1 0 No	7 WBLT 24 0	EB 18 0 0	9 0 0 0	10 0 0	11 a a a a	12 0 0	a a a	0 0	0 0 0	0 0 0
Pattern - 41 Split Pattern Cycle Offset Value Actuated Coord Actuated Walik Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 41) Preference 1 Preference 2 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Ped Recalls	150 38% Yes No None nases NE 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	S8 5 43 5 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (CD prelit) Seque Action Force 3 3 1 EBL1 18 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	OS) Add Tim g Plan ince Plan Off 4 WB 24 D 0	SBLT 15 0 0 Misc. E Veh. P Split D	6 NB 43 O O O O O O O O O O O O O O O O O O	7 WBLT 24 0	EB 0 0 0	9 0 0 0 Ven. Pe	10 0 0	Period Pe	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	a a a	0 0 0	0 0 0 0	0 0 0 0
Pattern - 41 Split Pettern Cycle Offset Value Actuated Coord Actuated Walik Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 41) Preference 1 Preference 2 Ring Ring Split Ext 0 Ring Disp Split Pattern Data Phase Coordinated Phases Vehicle Recalls	150 38% Yes No No None nases 1	SE 5 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (CD prelit) Seque Action Force 3 3 1 EBL1 18 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	OS) Add Tim g Plan ince Plan Off 4 WB 24 D 0	SBLT 15 0 0 Misc. E Veh. P Split D	411 0 0 0 No	7 WBLT 24 0	EB 0 0 0	9 0 0 0 Ven. Pe	10 0 0	Period Pe	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	a a a	0 0 0	0 0 0 0	0 0 0 0

Cycle Offset Value Actuated Coord Actuated Walk Rest	42 150 60% Yes No	Dv Tit Se	92 (Pat-Off) d (COS) yell/Add Tin ming Plan equence	79	13 42 0 0			Splits in Offsets			cent cent				
Phase Reservice Max Select	None -		tion Plan		No	ne									
Split Preference Ph		I a I		T =	-	-			40	- 44	40	40	44.	46	ac 1
Phase Description	1 NBLT	SB E	BLT WB	SBLT	NB NB	7 WBLT	EB B	9	10	11	12	13	14	15	16
Splits (Split Pat 42) Preference 1	15	42	21 22 D 0	16	41	0	21	0	0	0	0	0	0	0	0
Preference 2	0	0	0 0	0	0	0	0	0	0	0	0	0	0	0	0
Ring 1	2	3	4	Misc. C											
Ring Split Ext 0 Ring Disp.	0	0	0		ermissi ernand			Veh Pe Split De				/eh. Per Crossing		e 2 Disp al Pat). 0 0
Split Sum 100	100%		0%												
Split Pattern Data				_		_	_		_	_		_			
Phase Coordinated Phases	1	2 X	3 4	5	6 X	7	8	9	10	11	12	13	14	15	16
Vehicle Recalls Ped Recalls															
Max Recalls		х			х										
Phase Omit Special Function Output				-	-			Х	X	Х	X	X	X	Х	X
	None	Fo	rce Off												
Max Select Split Preference Ph					No	ne									
Split Preference Ph Phase	nases	2	3 4	5	6	7	8	9	10	tt.	12	13	14	15	16
Split Preference Ph Phase Description Splits (Split Pat 43)	NBLT	SB E	BLT WB 16 25	SBLT 16	6 NB 43	7 WBLT 25	EB 16	0	0	а	0	0	0	0	Ö
Split Preference Ph Phase Description	nases 1 NBLT	SB E	BLT WB	SBLT	6 NB	7 WELT	EB		7 71	T. 1		11	-		
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2	NBLT	SB E	BLT WB. 16 25 D D D 0	58LT 16 0	6 NB 43 0	7 WBLT 25 0	EB 16	0	0	O O	0	g g	0	0	0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5	1 NBLT 16 0 0 0 2 2 0	\$8 E	BLT WB. 16 25 D D D D O	SBLT 16 0 0 Misc. E Veh. P	6 NB 43 0 0	7 WBLT 25 0 0	68 16 0 0	0 0 0	Q Q Q Q	0 0	0 0	a a a	0 0 0	0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2	1 NBLT 16 0 0 0 2 0 0 0	\$8 E	BLT WB. 16 25 D D D 0	SBLT 16 0 0 Misc. E Veh. P	6 NB 43 0 0	7 WBLT 25 0 0	68 16 0 0	0 0	Q Q Q Q	0 0	0 0	a a a	0 0 0	0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp Split Sum 100	1 NBLT 16 0 0 0 2 0 0 0	\$8 E 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB 16 25 D D D 0 4 0 0	SBLT 16 0 0 Misc. E Veh. P	6 NB 43 0 0	7 WBLT 25 0 0	68 16 0 0	0 0 0	Q Q Q Q	0 0	0 0	a a a	0 0 0	0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring 91 Ring Split Ext 0 Ring Disp Split Sum 100 Split Pattern Data Phase	1 NBLT 16 0 0 0 2 0 0 0	\$8 E 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB 16 25 D D D 0	SBLT 16 0 0 Misc. E Veh. P	6 NB 43 0 0	7 WBLT 25 0 0	68 16 0 0	0 0 0	Q Q Q Q	0 0	0 0	a a a	0 0 0	0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring 9lit Ext 0 Ring Disp. 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	SB E 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0	7 WBLT 25 0 0	68 0 0	0 0 0 Véh. Pe	0 0 0 0 minissiv	0 0 0	0 0 0	a a a	0 0 0 mmissive g Arteria	0 0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0 0 Oata ermissiemand	7 WBLT 25 0 0	68 0 0	0 0 0 Véh. Pe	0 0 0 0 minissiv	0 0 0	0 0 0	a a a	0 0 0 mmissive g Arteria	0 0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring 5plit Ext 0 Ring Disp. 1 Split Pattern Data Phase Coordinated Phases Vehicle Recalls	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	\$8 E 43 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0	7 WBLT 25 0 0	68 0 0	0 0 0 Véh. Pe	0 0 0 0 minissiv	0 0 0	0 0 0	a a a	0 0 0 mmissive g Arteria	0 0 0 0	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Max Recalls	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0 0 Oata ermissiemand	7 WBLT 25 0 0	68 0 0	0 0 0 Vén. Pe Split De	0 0 0 ormissiv	0 0 0 0 2 2 2 4 1	0 0 0	a a a a a a a a a a a a a a a a a a a	0 0 0 mmissive g Arteria	0 0 0 0 2 Disp	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Ped Recalls Phase Omit	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0 0 Oata ermissiemand	7 WBLT 25 0 0	68 0 0	0 0 0 Vén. Pe Split De	0 0 0 ormissiv	0 0 0 0 2 2 2 4 1	0 0 0	a a a a a a a a a a a a a a a a a a a	0 0 0 mmissive g Arteria	0 0 0 0 2 Disp	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Ped Recalls Phase Omit	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0 0 Oata ermissiemand	7 WBLT 25 0 0	68 0 0	0 0 0 Vén. Pe Split De	0 0 0 ormissiv	0 0 0 0 2 2 2 4 1	0 0 0	a a a a a a a a a a a a a a a a a a a	0 0 0 mmissive g Arteria	0 0 0 0 2 Disp	0 0 0
Split Preference Ph Phase Description Splits (Split Pat 43) Preference 1 Preference 2 Ring 1 Ring 5plit Ext 0 Ring Disp. 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Ped Recalls Phase Omit	1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	BLT WB. 16 25 D D D D 0 4 0 0 0 0 6	SBLT 16 0 0 Misc. E Veh. P Split D	6 NB 43 0 0 0 Oata ermissiemand	7 WBLT 25 0 0	68 0 0	0 0 0 Vén. Pe Split De	0 0 0 ormissiv mand F	0 0 0 0 2 2 2 4 1	0 0 0	a a a a a a a a a a a a a a a a a a a	0 0 0 mmissive g Arteria	0 0 0 0 2 Disp	0 0 0

Pattern - 44 Spilt Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice	44 150 2% Yes No		Dw Tin Se	2 (Pat-Off I (COS) rell/Add Ti ning Plan quence tion Plan		14 44 0 0 2			Splits i			cent cent				
Max Select	None			rce Off			one									
Split Preference Ph Phase	ases	1 7 2	2	3 4	5	6	7	8	9	10	11	12	13	14	15	16
Description Splits (Split Pat 44)	NE	LT S	B E	BLT WE	SBLT	NB 48	WBLT 22		0	0	а	Q	a	0	0	0
Preference 1 Preference 2	-15-0		0	D 0 0 0	0	0	0	0	0	0	0	0	0	0	0	0
Inc.	1 2			-												
Ring 1 Ring Split Ext 0	0	0	91	0		ermissi		0	Veh Pe						2 Disp	
Ring Disp. : Split Sum : 100	% 100°	6 09		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Split D	ernand	Pat 1	0	Split De	imand f	Pat 2	0 (Orossing	g Arteria	d Pat	Ö.
Split Pattern Data																
Phase Coordinated Phases		_	_	3 4	5	6 X	-7	8	g	10	11	12	13	14	15	16
Coordinated Phases Vehicle Recalls		1	`	=		, A										
Ped Recalls Max Recalls	- 4-	+,	K.			×	-									
		_	_	_	_	-	-	_			-		-	_		
Phase Omit Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice	45 150 82% Yes No		Dw Tin Sec Act	2 (Pat-Off I (COS) ell/Add Ti ning Plan quence tion Plan		144 450 0 0	3		Splits i		Pen Pen	cent	(×)	X	Х	х
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select	45 150 82% Yes No No No	1	Dw Tin Sec Act	l (COS) rell/Add Ti ning Plan quence		45 0 0 1			Splits i	n	Per	cent		X	х	х
Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph	45 150 82% Yes No No None		Std Dw Tin Se Act For	d (COS) rell/Add Ti ning Plan quence tion Plan rce Off	me 5	45 0 0 1 0 No	one 7	8	Splits i	n	Per	cent	13	14	X 15	X 16
Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45)	H5 150 82% Yes No No None	LT S	Std Dw Tin Sei Act For	d (COS) rell/Add Ti ning Plan quence tion Plan ree Off 3 4 BLT WB	5 SBL7	45 0 0 1 0 No	7 WBLT	EB 17	Splits i	in 10	Per Per	cent cent	13 a	14	15	16
Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description	150 82% Yes No No None	LT S	Std Dw Tin Ser Act For 22 2 8 Es	d (COS) rell/Add Ti ning Plan quence tion Plan ree Off 3 4 BLT WE	5 SBLT	45 0 0 1 0 No	one 7	EB	Splits i	in	Per Per	cent cent	13	14	15	16
Special Function Output Pattern - 45 Split Pettern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2	H5 150 82% Yes No No None	5 5 5 0 0	Std Dw Tim Sec Act For	d (COS) rell/Add Ti ning Plan quence clon Plan ree Off 3 4 BLT WB 18 17 0 0 0 0	5 SBLT 10 0	45 0 0 1 0 No No No	7 WBLT 18	17 0	Splits i	10 0	Per Per	tent	13 a a	14	15	16 0 0
Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5	15 150 82% Yes No No None asses	5 5 5 0 C	Std Dw Tim Ser Act For	a (COS) ell/Add Ti ning Plan que de la	5 SBLT 10 0 0 Misc. I Veh F	45 0 0 1 0 No No No No	7 WBLT 18 0 0	68 17 0 0	Splits i Offsets	10 0 0	Per Per	12 0 0 0	13 0 0	14 0 0 0	15 0 0 0	16 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2	15 150 82% Yes No None None 1 () () () () () () () () () (3 0 0 0	Std Dw Tim Ser Act For	a (COS) ell/Add Ti ning Plan quence cion Plan ree Off 3 4 BLT WB 18 17 D D 0 0	5 SBLT 10 0 0 Misc. I Veh F	45 0 0 1 0 No No No No	7 WBLT 18 0 0	68 17 0 0	Splits i	10 0 0	Per Per	12 0 0 0	13 0 0	14 0 0	15 0 0 0	16 0 0
Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp Split Sum 100	15 150 82% Yes No None None 1 () () () () () () () () () (3 0 0 0	Std Dw Tim Ser Act For	a (COS) rell/Add Ti rell/Add Ti rell/Add Ti rell rell rell rell rell rell rell rel	5 SBLT 10 0 0 Misc. I Veh F	45 0 0 1 0 No No No No	7 WBLT 18 0 0	68 17 0 0	Splits i Offsets	10 0 0	Per Per	12 0 0 0	13 0 0	14 0 0 0	15 0 0 0	16 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Patt 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5 Ring 1 Ring 5 Ring 1 Ring	15 150 82% Yes No None None 1 () () () () () () () () () (SLT S 5 5 5 0 0 0 0 0 0 0	Sto Dwy Tin Sei-Act Foil	a (COS) rell/Add Ti rell/Add Ti rell/Add Ti rell rell rell rell rell rell rell rel	5 SBLT 10 0 0 Misc. I Veh F	450 0 0 No	7 WBLT 18 0 0	68 17 0 0	Splits i Offsets	10 0 0	Per Per	12 0 0 0	13 0 0	14 0 0 0	15 0 0 0	16 0 0
Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp Split Sum 100 Split Sum 100 Split Pattern Data	15 150 82% Yes No None None 1 () () () () () () () () () (SLT S 5 5 5 0 0 0 0 0 0 0	Sto Dwy Tin Sei-Act Foil	a (COS) ell/Add Tining Plan ing Plan querice clion Plan res Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	45000000000000000000000000000000000000	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 Véh. Pe	10 0 0 0	Per Per Der Der Der Der Der Der Der Der Der D	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 Q Q Q	0 0 0 0	15 0 0 0	16 0 0 0
Pattern - 45 Split Pattern Cycle Offset Value Actuated Value Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp Split Pattern Data Phase Coordinated Phases Coordinated Phases Vehicle Recalls Ped Rescalls	15 150 82% Yes No None None 1 () () () () () () () () () (SUT S 5 5 5 0 0 0 0 0 0 4 05	Sto Dww Tinn See Act For	a (COS) ell/Add Tining Plan ing Plan querice clion Plan res Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	450 0 0 1 0 No	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 Véh. Pe	10 0 0 0	Per Per Der Der Der Der Der Der Der Der Der D	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 Q Q Q	0 0 0 0	15 0 0 0	16 0 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp. Split Sym 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls	15 150 82% Yes No None None 1 () () () () () () () () () (SUT S 5 5 5 0 0 0 0 0 0 4 05	Sto Dwy Tin Sei-Act Foil	a (COS) ell/Add Tining Plan ing Plan querice clion Plan res Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	450 0 0 No	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 Véh. Pe	10 0 0 0	Per Per Der Der Der Der Der Der Der Der Der D	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	13 Q Q Q	0 0 0 0	15 0 0 0	16 0 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 9 Ring 1 Ring 10	15 150 82% Yes No None lases NE 1 () () () () () () () () () (SUT S 5 5 5 0 0 0 0 0 0 4 05	Sto Dww Tinn See Act For	a (COS) ell/Add Tining Plan ing Plan querice clion Plan res Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	450 0 0 1 0 No	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 0 Ven. Ps	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Per	12 0 0 0	13 g g g g	0 0 0 0 mmlssiv	15 0 0 0 0	16 0 0 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Walik Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 91 R	15 150 82% Yes No None lases NE 1 () () () () () () () () () (SUT S 5 5 5 0 0 0 0 0 0 4 05	Sto Dww Tinn See Act For	a (COS) ell/Add Tining Plan ing Plan querice clion Plan res Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	450 0 0 1 0 No	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 0 Ven. Ps	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Per	12 0 0 0	13 g g g g	0 0 0 0 mmlssiv	15 0 0 0 0	16 0 0 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Walik Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Ped Recalls Phase Omit	15 150 82% Yes No None lases NE 1 () () () () () () () () () (SUT S 5 5 5 0 0 0 0 0 0 4 05	Sto Dww Tinn See Act For	a (COS) ell/Add Tining Plan ing Plan querice clion Plan res Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	450 0 0 1 0 No	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 0 Ven. Ps	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Per	12 0 0 0	13 g g g g	0 0 0 0 mmlssiv	15 0 0 0 0	16 0 0 0 0
Special Function Output Pattern - 45 Split Pattern Cycle Offset Value Actuated Walik Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 45) Preference 1 Preference 2 Ring 1 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Ped Recalls Phase Omit	15 150 82% Yes No None lases NE 1 () () () () () () () () () (SUT S 5 5 5 0 0 0 0 0 0 4 05	Sto Dww Tinn See Act For	a (COS) ell/Add Tining Plan ing Plan querice clion Plan rece Off 3 4 BLT WB BLT WB 0 0 0 0	5 SBLT 10 0 0 Wisc. I	450 0 0 1 0 No	7 WBLT 18 0 0	EB 17 0 0	Splits i Offsets 0 0 0 0 Ven. Ps	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Per	12 0 0 0	13 g g g g	0 0 0 0 mmlssiv	15 0 0 0 0	16 0 0 0 0

Pattern - 51 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select	51 160 56% Yes No No		TS2 (Pat- Sid (COS) Dwell/Add Timing Pla Sequence Action Pla Force Off	Time in	0-6 51 0 0 1			Splits in Offsets i		Perc					
Split Preference Pl Phase Description Splits (Split Pat 51) Preference 1 Preference 2		2 7 S8 43	3 EBLT 1 20 D	4 5 WB SB1 20 12 0 0	6	7 WBLT 20 0	8 EB 20 0	9 0 0	10 0 0	11 0 0	0 0	0	0.	0 0 0	0 0 0
Ring 59lit Ext 0 Ring Disp. 5 Split Sum 100	0	3 0 0	0 0 0 0%		Data Permissi Demand			/eh. Per Split Der				n. Permi: ossing Ar			0
Split Pattern Data	I. 1	T 2	3	4 5	T 6	7	8	9	10	11	12	13 1	4	15	16
Coordinated Phases		X	,	3	X		,		10		12	10		10	
Vehicle Recalls Ped Recalls													1	_	
Max Recalls Phase Omit		X			X			х	х	х	х	× .	x	x	x
Special Function Output Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select	53 200 13% Yes No No None		TS2 (Pat- Std (COS) Dwell/Add Timing Pls Sequence Action Pla Force Off	Time an	0-0 15 0 0 1	1		Splits in Offsets i		Perc Perc					
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference PI Phase Description Splits (Split Pat 53)	53 200 13% Yes No No None nases	2 T SB 50	Sid (COS) Dwell/Add Timing Pla Sequence Action Pla Force Off 3 EBLT 1	Time an 4 5 N/B SBL1 26 8	15 0 0 1 0 No	1 7 WBLT 26	8 EB	Offsets 9	10	Perc 11	12 0	0	0	15	16
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference PI Phase Description Splits (Split Pat 53) Preference 1 Preference 2	53 200 13% Yes No. No. No. No. No. No. No. No. No. O. O. O. O. O. O. O. O. O. O. O. O. O.	2 T SB 50 0	Sid (COS) Dwell/Add Timing Pla Sequence Action Pla Force Off 3 EBLT 1 14 D	Time an 4 5 5 N/P SBL 28 8 0 0 0 0 0 Misc.	15 0 0 1 0 No	7 WELT 26 0	8 EB 14 0	9 0 0 0	10 0 0	Perc	12 0 0	0 0	0	0 0	0 0
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Walik Rest Phase Reservice Max Select Split Preference PI Phase Description Splits (Split Pat 53) Preference 1 Preference 2 Ring Ring Split Ext Ring Disp. Split Sum 100	53 200 13% Yes No. No. No. No. No. No. No. No. O o o	2 T SB 50	Std (COS) Dwell/Add Timing Pis Sequence Action Pla Force Off 3 EBLT 1 D D	Time an 4 5 5 8 8 0 0 0 0 0 Misc. Veh. 1	15 0 0 1 0 No No No	7 WBLT 26 0 0	8 EB 14 0 0 0	9 0	10 0 0 missive	11 a a a a a a a a a a a a a a a a a a	12 0 0 Vel	0	D D D D D D D D D D D D D D D D D D D	0 0 0	0
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Split Pat 53) Preference 1 Preference 2 Ring 1 Ring 5 Ring 5 Ring 1 Ring 5 Ring 1 Ring 5 Split Ext 6 Ring 5 Split Sum 100 Split Pattern Data Phase	53 200 13% Yes No. No. No. No. No. No. No. No. O o o	2 T SB 50 0 0 0	Std (COS) Dwell/Add Timing Pls Sequence Action Pla Force Off 3 EBLT 1 14 D D 0 0 0 0%	Time an 4 5 5 8 8 0 0 0 0 0 Misc. Veh. 1	15 0 0 1 0 No	7 WBLT 26 0 0	8 EB 14 0 0 0	9 0 0 0	10 0 0 missive	11 a a a a a a a a a a a a a a a a a a	12 0 0 Vel Circ	Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q	D D D D Ssive 2 Sterial F	0 0 0	0 0 0
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Walik Rest Phase Reservice Max Select Split Preference PI Phase Description Splits (Split Pat 53) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Split Ext 0 Ring Disp. 5 Split Pattern Data Phase Coordinated Phases Vehicle Recalls	53 200 13% Yes No No None 1 NBL 10 0 0 0 0 1 2 0 0 0 0 1 20 0 0 0 0 0 0	2 T SB 50 0 0	Std (COS) Dwell/Add Timing Pls Sequence Action Pla Force Off 3 EBLT 1 14 D D 0 0 0 0%	4 5 AMB SBL 28 8 0 0 0 0 0 Misc. Veh. I Split t	15 0 0 1 0 No	7 WBLT 26 0 0	8 EB 14 0 0	9 0 0 0 0	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 0 0 Vel Circ	Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q	D D D D Ssive 2 Sterial F	0 0 0 2 Disp.	0 0
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference PI Phase Description Splits (Split Pat 53) Preference 1 Preference 2 Ring 1 Ring 5plit Ext 6 Ring 5plit Ext 6 Ring 5plit Description Splits Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls Max Recalls	53 200 13% Yes No No None 1 NBL 10 0 0 0 0 1 2 0 0 0 0 1 20 0 0 0 0 0 0	2 T SB 50 0 0 0	Std (COS) Dwell/Add Timing Pls Sequence Action Pla Force Off 3 EBLT 1 14 D D 0 0 0 0%	4 5 AMB SBL 28 8 0 0 0 0 0 Misc. Veh. I Split t	15 0 0 1 0 No	7 WBLT 26 0 0	8 EB 14 0 0	9 0 0 0	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 0 0 0 Vel Dre	Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q	D D D D D D D D D D D D D D D D D D D	0 0 0 2 Disp.	0 0
Pattern - 53 Split Pattern Cycle Offset Value Actuated Coord Actuated Walik Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Split Pat 53) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5plit Ext 0 Ring Disp 5 Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls	53 200 13% Yes No. No. No. No. No. 0 0 0 0 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1	2 T SB 60 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Std (COS) Dwell/Add Timing Pls Sequence Action Pla Force Off 3 EBLT 1 14 D D 0 0 0 0%	4 5 AMB SBL 28 8 0 0 0 0 0 Misc. Veh. I Split t	15 0 0 0 1 0 No No Data Data Demand	7 WBLT 26 0 0	8 EB 14 0 0	9 0 0 0 0	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	11 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	12 0 0 Vel Circ	Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q Q	D D D D D D D D D D D D D D D D D D D	0 0 0 2 Disp.	0 0

Split Pattern Cycle Offset Value	57 200 97%		15	TS2 (Pa Std (CO Dwell/Ac	S)	ıq.	0-0 157			Splits i			cent:				
Actuated Coord Actuated Walk Rest Phase Reservice Max Select	Yes No No Non	6		Timing F Sequence Action P Force Of	Plan ce Plan		0 1 0 No	ne									
Split Preference Ph Phase Description		1 NBLT	2 58	3 EBLT	4 WB	5 SBLT	6 NB	7 WBLT	B	9	10	11	12	13	14	15	16
Splits (Split Pat 57) Preference 1 Preference 2		16	54 0 0	15 D	15 0 0	8 0	62 0	15 0 0	15 0	0	0	0	0	0	0 0 0	0 0	0 0
Ring 1 Ring Split Ext 0 Ring Disp. : Split Sum 100		0 00%	3 0 0	0 0 0%	1		Data Termissi ernand			Ven Pe Split De				Veh. Pe Crossin		e 2 Disp al Pat	, o
Split Pattern Data Phase		1.	2	3	4	5	6	7	8	g	10	11	12	13	14	15	16
Coordinated Phases Vehicle Recalls			Х				X										
Ped Recalls Max Recalls			Х				Х	-									
									_								
Phase Omit Special Function Outpu Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice	60 180 7% Yes No		1	TS2 (Pai Std (CO- Dwell/Ac Timing F Sequence Action P	S) dd Tim Plan ce Plan	e	0-0 64 0 0	•		X Splits i Offsets			cent	. *	X	X	х
Special Function Output Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest	60 180 7% Yes No No No	e	1	Std (CO Dwell/Ac Timing F Sequence	S) dd Tim Plan ce Plan	e 5 SBLT	0	•	8	Splits i	n	Per	cent	13	14	X 15	X 16
Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pt Phase	60 180 7% Yes No No No	e 1	2	Std (CO: Dwell/Ac Timing F Sequenc Action P Force O	S) dd Tim Plan ce Plan ff	5	64 0 0 1 0 No	ne 7		Splits i Offsets	n i in	Per Per	cent cent				
Special Function Output Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 60) Preference 1 Preference 2 Ring 1 Ring 5 Ring 6 Ring 7 Ring 8	60 180 7% Yes No No Non	1 NBLT 16 0 0	2 \$B 42 0	Std (CO Dwell/Ac Timing F Sequence Action P Force Of 3 EBLT 13	S) dd Tim Plan ce Plan ff 4 WB 29 0	5 SBLT 9 0 0	64 0 0 1 0 No No No	7 WELT 23 0	EB 19 0 0	Splits i	10 0 0 0	Per Per	12	13 a a	14 0 0 0	15 0 0 0	16 0 0
Special Function Output Pattern - 60 Split Pattern Cycle Offset Value Actuated Walk Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 60) Preference 1 Preference 2 Ring 1 Ring 5 Ring 6 Ring 7 Ring 8	60 180 7% Yes No No Non	1 NBLT 16 0 0	2 \$8 42 0 0	Std (CO) Dwell/Ac Timing F Sequence Action P Force O 3 EBLT 13 0 0	S) dd Tim Plan ce Plan ff 4 WB 29 0	5 SBLT 9 0 0	649 0 No No No No No No	7 WELT 23 0	EB 19 0 0	Splits i Offsets 9 0 0	10 0 0 0	Per Per	12	13 a a a a	14 0 0 0	15 0 0 0	16 0 0 0
Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 60) Preference 1 Preference 2 Ring 1 Ring Split Ext 0 Ring Disp. 5 Split Pattern Data Phase Coordinated Phases Vehicle Recalls	60 180 7% Yes No No Non	e 1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 SB 42 0 0	Std (CO) Dwell/Ac Timing F Sequence Action P Force O BBLT 13 0 0 4 0 0%	S) Add Tim Plan ce Plan ff 4 WB D 0	5 SBLT 9 0 0 Wisc. L Veh P Split D	649 0 No N	7 W8LT 23 0 0	68 19 0 0	Splits i Offsets 9 0 0 0 Ven. Pe	10 0 0 0	Per	12	13 Q Q Q Q	0 0 0 0	0 0 0 0	16 0 0 0
Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pt Phase Description Splits (Split Pat 60) Preference 1 Preference 2 Ring 1 Ring 1 Ring 5 Ring 1 Ring	60 180 7% Yes No No Non	e 1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 SB 42 0 0	Std (CO) Dwell/Ac Timing F Sequence Action P Force O BBLT 13 0 0 4 0 0%	S) Add Tim Plan ce Plan ff 4 WB D 0	5 SBLT 9 0 0 Wisc. L Veh P Split D	64 0 0 No	7 W8LT 23 0 0	68 19 0 0	Splits i Offsets 9 0 0 0 Ven. Pr	10 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Per	12	13 Q Q Q Q	0 0 0 0	0 0 0 0	16 0 0 0
Special Function Output Pattern - 60 Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Ph Phase Description Splits (Split Pat 60) Preference 1 Preference 2 Ring 1 Ring 1 Ring 1 Ring Split Ext 0 Ring Disp. Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls Ped Recalls	60 180 7% Yes No No Non	e 1 NBLT 16 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 SB 42 0 0 3 0 6 0%	Std (CO) Dwell/Ac Timing F Sequence Action P Force O BBLT 13 0 0 4 0 0%	S) Add Tim Plan ce Plan ff 4 WB D 0	5 SBLT 9 0 0 Wisc. L Veh P Split D	6 NB 49 0 O O O O O O O O O O O O O O O O O O	7 W8LT 23 0 0	68 19 0 0	Splits i Offsets 9 0 0 0 Ven. Pe	10 0 0 0	Per	12	13 Q Q Q Q	0 0 0 0	0 0 0 0	16 0 0 0

Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl	61 180 21% Yes No No No	6	S C T S	FS2 (Pat- std (COS Dwell/Add Firming Pla Sequence Action Pla Force Off	i) d Time an e an		0-0 611 0 0 1 0 None		Splits Offset			rcent: rcent				
Phase Description		1 NBLT	2 58		4 WB S	5 SBLT	_	7 E	_	10	11	12	13	14	15	16
Splits (Split Pat 61) Preference 1 Preference 2		13	49 0 0	16 D 0	22 0 0	13 0 0	0 0	0 0 0 0	0	0	0	0	0	0 0	0	0
Ring Split Ext C Ring Disp. Split Sum 100	0 0 0% 10	0 00%	3 0 0	4 0 0 0%	V	lisc. Dat eli. Perr plit Dern	mssive			ermissiv emand (Veh. Pe Crossin		e 2 Disp al Pat	, 0
Split Pattern Data Phase		1	2	3	4	5	6	7 8	g	10	11	12	13	14	15	16
Coordinated Phases Vehicle Recalls			Х				Х									
Ped Recalls Max Recalls			х				х									
Phase Omit Special Function Output	al	-	\rightarrow	-	-	-	-	-	X	X	X	X	×	X	X	X
Pattern - 62 Split Pattern. Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice	62 180 24% Yes No		5 T S	FS2 (Pat- Std (COS Dwell/Add Firming Pla Sequence Action Pla	i) d Time an		0-0 622 0 0		Splits Offset			rcent rcent				
Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Split Pat 62) Preference 1	180 24% Yes No No Non	1 NBLT 13 0	2 SB 52 0	Std (COS Owell/Add Firming Pla Sequence Action Pla Force Off 3 EBLT	d Time an an www.s	10	6 NB M	7 8 BLT E 19 10 0 0	Offset	10 0	Per di	12 0	13	0 0	0 0	0 0 0
Split Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Split Pat 62) Preference 1 Preference 2 Ring Ring Split Ext Ring Disp. Split Sum 100	180 24% Yes No No Non Non	e 1 NBLT 13	2 SB 52 0 0	ed (COS) Owell/Add Fining Pla Fining Pla Orace Off BELT 18 0 0	d Time an 4 WB S 17 D 0	10 10	6 Nome NB W 55 0 1 0 Nome	BLT E 19 10 0 0	Offset	10 0	111 a a a	12 0 0 0	a a	0 0	0 0 0	0 0 0
Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Split Pat 62) Preference 1 Preference 2 Ring Ring Ring Split Ext (Ring Disp. Split Sum 100 Split Pattern Data Phase	180 24% Yes No No Non Non	1 NBLT 13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 SB F2 O O O O M S	ed (COS) Owell/Add Fining Pla Fining Pla Orace Off BELT 18 0 0	d Time an 4 WB S 17 D 0	SBLT 10 0 0 0 isc. Det	622 0 0 1 0 None 6 NB W 555 0 0	BLT E 19 10 0 0 0 0	Offset	10	111 a a a	12 0 0 0	a a a	0 0	0 0 0	0 0 0
Split Pattern Cycle Offset Value Actuated Coord Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Split Pat 62) Preference 1 Preference 2 Ring Ring Split Ext (Ring Disp. Split Sum 100 Split Pattern Data	180 24% Yes No No Non Non	1 NBLT 13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 SB 52 0 0 0 5 0 0 0	Std (COS Owell/Add Timing Pic Sequence Action Pla Force Off 3 EBLT 18 0 0 0 0 0 %	4 4 4 5 17 D D M VV	SBLT 10 0 0 0 iisc. Dati eh. Perm plit Dem	6 Nome Nome NB W 55 0 0 Name NB W Amissive	BLT E 19 14 0 0 0 0	Offset	10 0 0 0	Per Per 2	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Q Q Q Veh. Pe	0 0 0	0 0 0 0	0 0 0
Spilt Pattern Cycle Offset Value Actuated Coord Actuated Walk Rest Phase Reservice Max Select Split Preference Pl Phase Description Splits (Spilt Pat 62) Preference 1 Preference 2 Ring Ring Split Ext Ring Split Ext Ring Split Sum 100 Split Pattern Data Phase Coordinated Phases Vehicle Recalls	180 24% Yes No No Non Non	1 NBLT 13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 SB F2 O O O O M S	Std (COS Owell/Add Timing Pic Sequence Action Pla Force Off 3 EBLT 18 0 0 4 0 0 0%	4 4 4 5 17 D D M VV	SBLT 10 0 0 0 iisc. Dati eh. Perm plit Dem	622 0 0 1 0 None 6 NB W 555 0 0	BLT E 19 14 0 0 0 0	Offset	10 0 0 0	Per Per 2	12 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Q Q Q Veh. Pe	0 0 0	0 0 0 0	0 0 0

Page 1 of 1

Lee County, FL

409 - US 41 & Coconut Rd - - Econolite Type - ASC3

Controller Timing Plan (MM)2-1 Plan 1

Phase	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Direction	NBLT	SB	EBLT	WB	SBLT	NB	WBLT	EB								
Min Green	7	26	7	10	7	26	7	10	0	0	0	0	0	0	0:	0
BK Min Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
CS Min Green	0	0	0	0	0	0	0.	0	0.	0	0	0	0	0	0	0
Delay Green	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Walk	0	7	0	7	0	7	0	7	0	0	0	0	0	0	0	0
Walk 2	0	0	0	0	0	0	Ò	0	0	0	0	0	0	0	0	0
Walk Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.	0
Ped Clear	0	26	0	38	0	28	0	40	0	0	0	0	0	0	0	0
Ped Clear 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped Clear Max	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Ped CO	0.	0	0	0	0	0	0	0	0	0	0	0	0	0	O.	0
Vehicle Ext	3.0	5.0	3.0	3.0	3.0	5.0	3.0	3.0	0.0	0.0	0.0	0,0	0,0	0.0	0.0	0.0
Vehicle Ext 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max 1	25	90-	25	25	20	90	30	25	. 0	0	0	0	0	0	0	0
Max 2	0	0	0	0	0	0.	0	0	0	0	0	0	0.	0	0	0
Max 3	0	0	0	0	0.	0	0 -	0	0	0	0	0	O.	0.	0.	0
DYM Max	0	0	0	0	0	0	b .	0	. 0	10	0	0	0	O.	0	0
DYM Stp	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	5.1	5.1	4.0	4.7	5.1	5.1	4.0	4.7	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Red Clear	2.5	2.0	2.0	2.5	2.5	2.0	2.0	2.5	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0
Red Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0,0	0.0	0.0	0.0
Red Revert	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
ACT B4	0	0	0	0	0	O.	0	0	0	0	0	0	0	0	0	0
SEC/ACT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Max Int	30	30	30	30	30	30	30	30	30	30	30	30	0	0	O.	0
Time B4	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Cars Wt	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
STPT Duc	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Time To Reduce	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Min Gap	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

file:///C:/Users/padgetms/AppData/Roaming/Econolite/0/TimingPlan.xml

2/18/2016

Appendix M: Intersection Analysis - HCS 2010 Printouts (9 Sheets)	Coconat Roda Trajjic Study – June 2016	
	Appendix M: Intersection	Analysis - HCS 2010 Printouts

Assigned Phase 3 8 7 4 1 6 5 Case Number 2.0 3.0 2.0 3.0 2.0 3.0 2.0			1.00 2.	103	igiiaii	zeu i	mers	CHOI	i ne:	sults S	иши	ary				
Agency	Ganaral Informati	ion							T	Interce	tion les	orm of	NP	7	nd design	-1
Analysis		IOII			_		_	_	\rightarrow			-) II		11111	
Jurisdiction		-	M	_	Analisa	ic Data	Mara	1 2016	-	_		-				
Unban Street	THE RESERVE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TW		avi.		-	-	ivieti 2	1, 2010	_			-		- 165	ah-	7
Demand Information		0.0	Q /1 (QD //5)				2016		\rightarrow		Darind	1000	nn	- 10		- 5
Project Description		\rightarrow		_	_		-	AM DIVE	_	_		17-40	00	-8		1
Demand Information		_			File Na	arne	2010	SIVI PK I	11 - 03	-22-2010	xus	_		- 9	11111	
Approach Movement L T R L T	Project Description	1 2	UTO AIM PK HF		_	-		-			-					
Approach Movement L T R L T	Demand Informat	tion		_		EB			W	3		NB			SB	_
Demand (v), veh/h	Approach Moveme	ent			L	_	R	L	-		L	1. 644	I R	L	I T	R
Signal Information		_				_	_	_	-		_	_	_		_	197
Cycle, s 130.0 Reference Phase 2	7 - 71 - 154W						1 11	120					1	1		
Color Colo	Signal Informatio	n				1		124	1	of [12]	2	84			4	5
Discoordinated No Simult Gap EW On Yellow 51 0.0 5.1 4.0	Cycle, s 13	30.0 R	Reference Phase	2		180	St	0	2	8	8		1 4	-	1	1
Discriminated No Simult Gap E/W On Red 2.5 0.0 2.	Offset, s				Green	6.5	11	62.4	8.2	4.0	13 8	3		-	2	
Timer Results	Uncoordinated 1				-	-								t	/	100
Assigned Phase	Force Mode Fi				Red	2.5	0.0	2.0	2.0	2.0	2.5		100	1	7	V
Assigned Phase 3 8 7 4 1 6 5 Case Number 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.1 1.0 7.0 0.1 7.0 0.1 7.0 0.1 7.0 0.0 0.0 3.0 3.1 3.1 3.0 0.0 0.0 3.						-,-										
Case Number 2.0 3.0 2.0 3.0 2.0 3.0 2.0 3.0 2.0 Phase Duration, s 14.2 21.0 24.3 31.1 15.2 70.6 14.1 Change Penod, (Y+R □), s 6.0 7.2 6.0 7.2 7.6 7.1 7.6 Max Allow Headway (MAH), s 3.1 3.1 3.0 3.1 3.0 0.0 3.0 Queue Clearance Time (g □), s 8.1 13.4 18.1 9.7 8.0 4.8 Green Extension Time (g □), s 0.2 0.4 0.1 0.7 0.0 0.0 0.1 Phase Call Probability 1.00 1.00 1.00 1.00 0.00 0.18 0.00 0.0 Movement Group Results EB WB WB NB SB SB Approach Movement L T R L T R L T R L T R L T R L	100 51 110 5000				-		-	_	L		_		2000			SBT
Phase Duration, s 14.2 21.0 24.3 31.1 15.2 70.6 14.1 Change Penod, (Y+R∞), s 6.0 7.2 6.0 7.2 7.6 7.1 7.6 Max Allow Headway (MAH), s 3.1 3.1 3.0 3.1 3.0 0.0 3.0 Queue Clearance Time (g∞), s 8.1 13.4 18.1 9.7 8.0 4.8 Green Extension Time (g∞), s 0.2 0.4 0.1 0.7 0.0 0.0 0.1 Phase Call Probability 1.00 1.00 1.00 1.00 0.00 0.18 0.00 Movement Group Results EB WB NB NB 0.00 0.00 Movement Group Results EB WB NB NB L T R L T R L T R L T R L T R L T R L T R L T R L					_				41				-			2
Change Period, (Y+Rv), s 6.0 7.2 6.0 7.2 7.6 7.1 7.6 Max Allow Headway (MAH), s 3.1 3.1 3.0 3.1 3.0 0.0 3.0 Queue Clearance Time (g s), s 8.1 13.4 18.1 9.7 8.0 4.8 Green Extension Time (g s), s 0.2 0.4 0.1 0.7 0.0 0.0 0.1 Phase Call Probability 1.00 1.00 1.00 0.095 0.93 0.93 Max Out Probability 0.03 0.25 1.00 0.00 0.18 0.00 0.00 Movement Group Results EB WB WB NB SB Approach Movement L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T	Case Number				2.0	-		2.0		3.0	2.0		3.0	2.0	_	3.0
Max Allow Headway (MAH), s 3.1 3.0 3.1 3.0 0.0 3.0 Queue Clearance Time (g ₃), s 8.1 13.4 18.1 9.7 8.0 4.8 Green Extension Time (g ∘), s 0.2 0.4 0.1 0.7 0.0 0.0 0.1 Phase Call Probability 1.00 1.00 1.00 1.00 0.95 0.93 0.93 Max Out Probability 0.03 0.25 1.00 0.00 0.18 0.00 0.00 Movement Group Results EB WB NB SB Approach Movement L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T<	Phase Duration, s				14.2	2	21.0	24.3	3	31.1	15.2	2	70.6	14.1	1	69.5
Queue Clearance Time ($g \circ$), s 8.1 13.4 18.1 9.7 8.0 4.8 Green Extension Time ($g \circ$), s 0.2 0.4 0.1 0.7 0.0 0.0 0.1 Phase Call Probability 1.00 1.00 1.00 0.00 0.01 0.00 0.03 Max Out Probability 0.03 0.25 1.00 0.00 0.18 0.00 0.00 Movement Group Results EB WB NB SB Approach Movement L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T A L T R L T A L L T A L L <td></td> <td></td> <td></td> <td></td> <td>6.0</td> <td></td> <td>7.2</td> <td>6.0</td> <td></td> <td>7.2</td> <td>7.6</td> <td></td> <td>7.1</td> <td>7.6</td> <td></td> <td>7.1</td>					6.0		7.2	6.0		7.2	7.6		7.1	7.6		7.1
Green Extension Time (g ∘), s 0.2 0.4 0.1 0.7 0.0 0.0 0.1 Phase Call Probability 1.00 1.00 1.00 1.00 1.00 0.95 • 0.93 • Max Out Probability 0.03 0.25 1.00 0.00 0.18 • 0.00 • Movement Group Results Image: Company of the properties of the propert							_	_			_	-	0.0		\rightarrow	0.0
Phase Call Probability 1.00 1.00 1.00 1.00 0.95 0.93 0.93 0.25 1.00 0.00 0.18 0.00 0	Queue Clearance	Time (gs), s			-	_	_	_		_	-		_	-	
Movement Group Results EB VB VB VB VB SB SB Approach Movement Assigned Movement L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T R L T A 14 14 16 6 16 4 4 15 16 16 16 14 4 15 17 169 1449 1723 1689 1610 17.7 4 2 6.0 14.9 10.2 2.8 45.4 Cycle Queue Clearance Time (g s), s 6.1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 </td <td>Green Extension T</td> <td>Time (g</td> <td>g e), s</td> <td></td> <td>0.2</td> <td>_</td> <td></td> <td></td> <td>_</td> <td>0.7</td> <td>0.0</td> <td></td> <td>0.0</td> <td>_</td> <td>_</td> <td>0.0</td>	Green Extension T	Time (g	g e), s		0.2	_			_	0.7	0.0		0.0	_	_	0.0
Movement Group Results EB	Phase Call Probab	oility			1.00		1.00	1.00)	1.00	0.98	5		0.93	3	
Approach Movement L T R L T B 2 Adjusted Flow Rate (v), veh/h 171 1863 1610 1723 1863 1610 1723 1863 1610 1722 1863 1810 1810 180 <td< td=""><td>Max Out Probabilit</td><td>ty</td><td></td><td>-</td><td>0.03</td><td>3-</td><td>0.25</td><td>1.00</td><td>)</td><td>0.00</td><td>0.18</td><td>3</td><td></td><td>0.00</td><td>)</td><td></td></td<>	Max Out Probabilit	ty		-	0.03	3-	0.25	1.00)	0.00	0.18	3		0.00)	
Approach Movement L T R L T R L T R L T R L T R Signed Movement 3 8 18 7 4 14 1 6 16 16 5 2 Adjusted Flow Rate (v), veh/h 164 96 144 435 127 60 83 927 192 76 2038 Adjusted Saturation Flow Rate (s), veh/h/n 1723 1863 1610 1723 1863 1579 1774 1691 1449 1723 1691 Queue Service Time (g s), s 6 1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Cycle Queue Clearance Time (g s), s 6 1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Cycle Queue Clearance Time (g s), s 6 1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Green Ratio (g/C) 0.06 0.11 0.11 0.14 0.18 0.18 0.06 0.49 0.49 0.05 0.48 Capacity (c), veh/h 218 198 171 484 342 290 104 2477 708 173 2436 Volume-to-Capacity Ratio (X) 0.753 0.484 0.840 0.899 0.370 0.208 0.796 0.374 0.271 0.436 0.837 Back of Queue (Q), tt/ln (95 th percentile) 122.9 135.3 225.3 321.7 162.1 73.5 126.5 242.8 154.5 54.6 628.3 Back of Queue (Q), veh/ln (95 th percentile) 4.8 5.3 9.0 12.7 6.4 2.9 5.0 9.6 6.2 2.1 24.7 Queue Storage Ratio (RQ) (95 th percentile) 0.20 0.00 0.56 0.71 0.00 0.11 0.26 0.00 0.21 0.09 0.00 Uniform Delay (d 1), s/veh 59.9 54.7 57.0 55.0 46.5 45.1 60.4 20.8 19.6 59.9 29.4 10.00 0.00 0.00 0.00 0.00 0.00 0.00 0		р				CH			114.5			1100			CO.	
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Adjusted Saturation Flow Rate (s), veh/h/ln 1723 1863 1610 1723 1863 1579 1774 1691 1449 1723 1691 Queue Service Time (g s), s 6.1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Cycle Queue Clearance Time (g s), s 6.1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Green Ratio (g/C) 0.06 0.11 0.11 0.14 0.18 0.18 0.06 0.49 0.49 0.05 0.48 Capacity (c), veh/h 218 198 171 484 342 290 104 2477 708 173 2436 Volume-to-Capacity Ratio (X) 0.753 0.484 0.840 0.899 0.370 0.208 0.796 0.374 0.271 0.436 0.837 Back of Queue (Q), tt/ln (95 th percentile) 122.9 135.3 225.3 321.7 162.1 73.5 126.5 242.8 154.5 54.6 628.3 Back of Queue (Q), veh/ln (95 th percentile) 4.8 5.3 9.0 12.7 6.4 2.9 5.0 9.6 6.2 2.1 24.7 Queue Storage Ratio (RQ) (95 th percentile) 0.20 0.00 0.56 0.71 0.00 0.11 0.26 0.00 0.21 0.09 0.00 Uniform Delay (d 1), s/veh 59.9 54.7 57.0 55.0 46.5 45.1 60.4 20.8 19.6 59.9 29.4 Incremental Delay (d 2), s/veh 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.		_	2 in la Ha		_		_		_	-		-	_	_		12
Queue Service Time (g s), s 6.1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Cycle Queue Clearance Time (g s), s 6.1 6.3 11.4 16.1 7.7 4.2 6.0 14.9 10.2 2.8 45.4 Green Ratio (g/C) 0.06 0.11 0.11 0.14 0.18 0.18 0.06 0.49 0.05 0.48 Capacity (c), veh/h 218 198 171 484 342 290 104 2477 708 173 2436 Volume-to-Capacity Ratio (X) 0.753 0.484 0.840 0.899 0.370 0.208 0.796 0.374 0.271 0.436 0.837 Back of Queue (Q), tt/ln (95 th percentile) 122.9 135.3 225.3 321.7 162.1 73.5 126.5 242.8 154.5 54.6 628.3 Back of Queue (Q), veh/ln (95 th percentile) 4.8 5.3 9.0 12.7 6.4 2.9 5.0 9.6 6.2 2.1 24.7 Queue Storage Ratio (RQ) (95 th percentile) <td></td> <td></td> <td></td> <td></td> <td>_</td> <td>-</td> <td>-</td> <td></td> <td>_</td> <td>_</td> <td>_</td> <td>-</td> <td>_</td> <td></td> <td>-</td> <td>201</td>					_	-	-		_	_	_	-	_		-	201
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Green Ratio (g/C) 0.06 0.11 0.11 0.14 0.18 0.18 0.06 0.49 0.49 0.05 0.48 Capacity (c), veh/h 218 198 171 484 342 290 104 2477 708 173 2436 Volume-to-Capacity Ratio (X) 0.753 0.484 0.840 0.899 0.370 0.208 0.796 0.374 0.271 0.436 0.837 Back of Queue (Q), tel/ln (95 th percentile) 122.9 135.3 225.3 321.7 162.1 73.5 126.5 242.8 154.5 54.6 628.3 Back of Queue (Q), veh/ln (95 th percentile) 4.8 5.3 9.0 12.7 6.4 2.9 5.0 9.6 6.2 2.1 24.7 Queue Storage Ratio (RQ) (95 th percentile) 0.20 0.00 0.56 0.71 0.00 0.11 0.26 0.00 0.21 0.09 0.00 Uniform Delay (d ·), s/veh 59.9 54.7 57.0 55.0 46.					_		-	-	_	-		-	_		_	9.9
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Back of Queue (Q), veh/lin (95 th percentile) 4.8 5.3 9.0 12.7 6.4 2.9 5.0 9.6 6.2 2.1 24.7 Queue Storage Ratio (RQ) (95 th percentile) 0.20 0.00 0.56 0.71 0.00 0.11 0.26 0.00 0.21 0.09 0.00 Uniform Delay (d₁), s/veh 59.9 54.7 57.0 55.0 46.5 45.1 60.4 20.8 19.6 59.9 29.4 Incremental Delay (d₂), s/veh 2.0 0.7 14.4 17.8 0.2 0.1 5.1 0.4 0.9 0.6 3.6 Initial Queue Delay (d₃), s/veh 0.0 <td></td> <td></td> <td>THE RESERVE TO THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN</td> <td></td> <td>-</td> <td>and to be desired in the</td> <td>-</td> <td>-</td> <td>_</td> <td>-</td> <td>-</td> <td>-</td> <td>the latest designation of the latest designa</td> <td>_</td> <td>-</td> <td>0.26</td>			THE RESERVE TO THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN		-	and to be desired in the	-	-	_	-	-	-	the latest designation of the latest designa	_	-	0.26
Queue Storage Ratio (RQ) (95 th percentile) 0.20 0.00 0.56 0.71 0.00 0.11 0.26 0.00 0.21 0.09 0.00 Uniform Delay (d 1), s/veh 59.9 54.7 57.0 55.0 46.5 45.1 60.4 20.8 19.6 59.9 29.4 Incremental Delay (d 2), s/veh 2.0 0.7 14.4 17.8 0.2 0.1 5.1 0.4 0.9 0.6 3.6 Initial Queue Delay (d 3), s/veh 0.0 <td< td=""><td></td><td></td><td>The second second second</td><td>_</td><td>2000000</td><td>-</td><td>_</td><td>-</td><td>_</td><td></td><td>-</td><td>-</td><td></td><td></td><td>100,000,000</td><td>163,</td></td<>			The second second second	_	2000000	-	_	-	_		-	-			100,000,000	163,
Uniform Delay (d i), s/veh 59.9 54.7 57.0 55.0 46.5 45.1 60.4 20.8 19.6 59.9 29.4 Incremental Delay (d z), s/veh 2.0 0.7 14.4 17.8 0.2 0.1 5.1 0.4 0.9 0.6 3.6 Initial Queue Delay (d s), s/veh 0.0					_	-	-	-	-	THE RESERVE	_	-	-	-	-	6.5
Incremental Delay (d z), s/veh 2.0 0.7 14.4 17.8 0.2 0.1 5.1 0.4 0.9 0.6 3.6 Initial Queue Delay (d s), s/veh 0.0				ne)	-	-	-	-	-	-		-	-	-	-	0.59
Initial Queue Delay (d s), s/veh 0.0 <th< td=""><td>The second secon</td><td>-</td><td>777</td><td></td><td>_</td><td>_</td><td>_</td><td>-</td><td>_</td><td></td><td>_</td><td>-</td><td>-</td><td></td><td>-</td><td>20.1</td></th<>	The second secon	-	777		_	_	_	-	_		_	-	-		-	20.1
Control Delay (d), s/veh 61.9 55.4 71.4 72.7 46.7 45.2 65.6 21.3 20.6 60.6 33.0 Level of Service (LOS) E E E E D D E C C E C Approach Delay, s/veh / LOS 63.7 E 64.8 E 24.2 C 32.8		-			_	_	_	-	-	-		_	-	_	_	0.9
Level of Service (LOS) E E E E E D D E C C E C Approach Delay, s/veh / LOS 63.7 E 64.8 E 24.2 C 32.8					-	-		-	-	-	_	_	_		-	0.0
Approach Delay, s/veh / LOS 63.7 E 64.8 E 24.2 C 32.8	The second secon				_	-	-	_	-	_	_	-	-	_	-	21.0
		THE RESERVE	00		-	_	_				-		_	_		C
Intersection Delay, siven / LOS 37.7 D					63.7		_			E	24.2	4				С
	intersection Delay,	s/veh	1108				3/	.1						ט		
Multimodal Results EB WB NB SB	Multimodal Pooul	lte		-		ED			\A/D			NID			CD.	
	CONTRACTOR OF THE PARTY OF THE	_	os		2.4	_	0	2.0	_		20	_	0	20	-	В
Pedestrian LOS Score / LOS 3.4 C 3.5 C 2.9 C 2.6 Bicycle LOS Score / LOS 1.2 A 1.5 A 1.1 A 1.8	Total Control of the					_		-		_		$\overline{}$			$\overline{}$	A

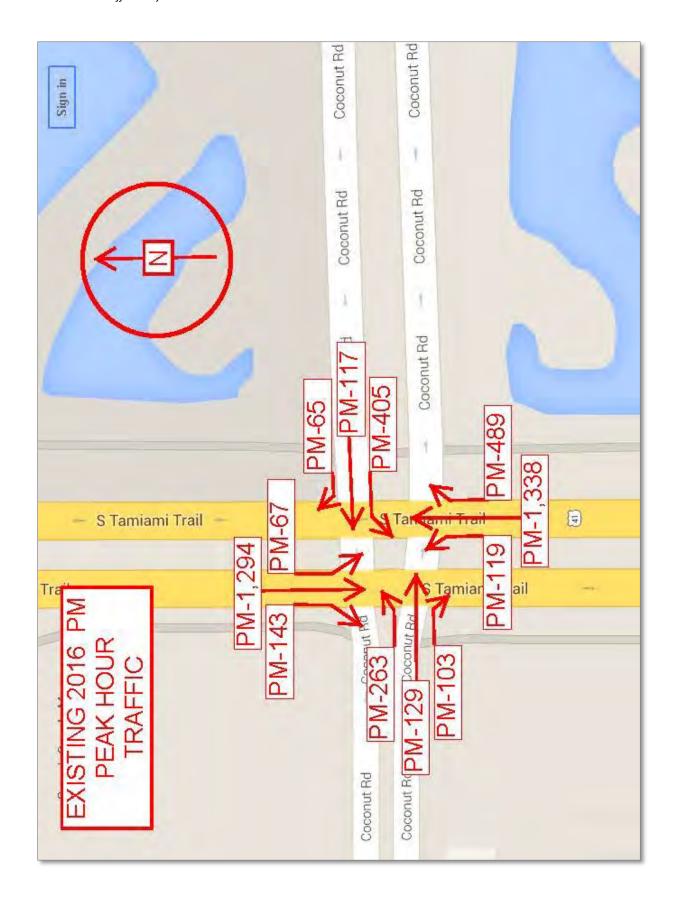
		HCS 2	0 10 3	ignan	zeu i	nierse	ction	res	suits 5	umm	ary	_			
Canagal Inform	antin m			-					luterose	tion luf				of the 1	-1
General Inform	lation		_		_			\rightarrow	Intersec		0.25	on		JIIII	
Agency Analyst		CM	_	LAnaka	io Doto	Mar 2	2 2016	-	Duration Area Typ	_	Other	_	- 13 (19)		
Jurisdiction		Civi		Time F	-	IVIAI 2	2, 2010	_	PHF	e	0.98		- 勝二	eT.	7
Urban Street		LIC 44 (CD 45)	_			2018		-		Doring	1> 4:	20	-11-		Ţ
		US 41 (SR 45)	_	-	sis Year	-	DM DG		Analysis		12.40	00	- 8	W06.5	
Intersection	07.	Coconut Road		File N	ame	2016	PIVI PK	Hr - U3	-22-2016	.xus	_		- 4	1111	7
Project Descrip	tion	2016 PM Pk Hr		-	-	-								(STREET)	
Demand Inform	nation		_		EB	_	1	WE	3	T	NB	_		SB	_
Approach Move	77.75			L	T	R	L	IT	R	L	T	R	IL	T	I R
Demand (v), v				268	129	103	405	-		119	1338	_	67	1294	-
Demand (V), V	enn	_	-	200	120	100	405	117	00	113	1000	408	07	1294	140
Signal Informa	tion					1	121				8		1	1	5
Cycle, s	180.0	Reference Phase	2	1	18.	8.6		10	8	F-	-	1	_	1	-
Offset, s	0	Reference Point	End	-	100	100		C		3		H X	2	4	
Uncoordinated	No	Simult. Gap E/W	On	Green	-	0.0	5.1	7 16.5	0.0	15.2			40	_	
Force Mode	_		_	Red	2.5	0.0	2.0	2.0	2.0	2.5		-		-	V
	ce Mode Fixed Simult. Gap N/S On														100
Timer Results	ner Results					EBT	WB	L	WBT	NBI		NBT	SB		SBT
Assigned Phase	e			3		8	7	11 1	4	1		6	5		2
Case Number				2.0		3.0	2.0		3.0	2.0		3.0	2.0		3.0
Phase Duration			_	22.5	-	22.4	25.9	_	25.8	16.8	\rightarrow	117.3	14.4	-	114.8
Change Period		.1 =		6.0	\rightarrow	7.2	2.0		7.2	2.5	\rightarrow	7.1	7.6	-	7.1
AND DESCRIPTION OF PERSONS ASSESSMENT	-	with the same		3.1	_	3.1	3.0	_	-	3.0	_	0.0	3.0	_	0.0
Max Allow Head				-	\rightarrow	_			3.1	_		0.0	_	_	0.0
Queue Clearan				16.1	-	14.5	23.3		13.1	14.2	-	0.0	5.5	-	0.0
Green Extension	-	(ge), s		0.4	_	0.7	0.7	_	0.7	0.2	_	0.0	0.1	_	0.0
Phase Call Pro				1.00	_	1.00	1.00	-	1.00	1.00	_		0.97	_	
Max Out Proba	bility			0.00) -	0.00	0.01		0.00	0.00)	_	0.00)	
Movement Gro	un Ras	culte	_		EB	_		WB	_		NB	_		SB	
Approach Move		ourta .	_	L	T	R	L	T	IR	L	T	R	T.	T	R
Assigned Move	-		-	3	8	18	7	4	14	1	6	16	5	2	12
	_	V Cobile			_	_		_	-	_	_	_	_		_
Adjusted Flow F	_			273	132	105	413	119	66	121	1365	499	68	1320	146
		ow Rate (s), veh/h/l	11	1723	1863	1610	1723	1863	-	1774	1691	1449	1723	1691	1579
Queue Service				14.1	12.5	11.5	21.3	11.1	7.1	12.2	25.7	36.7	3.5	25.4	7.4
		e Time (g ⊧), s		14.1	12.5	11.5	21.3	11.1	7.1	12.2	25.7	36.7	3.5	25.4	7.4
Green Ratio (g				0.09	0.08	0.08	0.13	0.10	_	0.08	0.61	0.61	0.04	0.60	0.60
Capacity (c), v		6 - 6 3/3		317	157	136	458	193	163	141	3106	887	130	3037	945
Volume-to-Cap		The second secon		0.864	-	0.773		-	-	0.861	0.440	0.563	0.528	0.435	0.15
		/In (95 th percentile)		271.1	-	213.3	387.9	-	-	245.2	381	458	71.7	380.1	121,
		eh/ln (95 th percent		10.7	10.3	8.5	15.3	9.1	5.2	9.7	15.0	18.3	2.8	15.0	4.9
NY NY TENY		RQ) (95 th percent	tile)		0.00	7771	0.86	1		0.51	1000	1		0.00	
Uniform Delay				80.6		80.7	76.9	77.3	-	81.9	18.5	20.7	85.0	19.6	16.0
Incremental De				5.7	4.4	3,5	11.5	1,2	0.6	5.8	0.5	2.6	1,2	0.5	0.3
Initial Queue De				0.0	0.0	0,0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0,0	0.0
Control Delay (86.3	85.6	84.2	88.3	78.5	-	87.7	19.0	23.2	86.3	20.1	16.3
Level of Service	(LOS)			F	F	I F	F	E	E	F	В	C	JF.	С	В
Approach Delay	y, s/veh	/LOS		85.7	7	F	85.0		F	24.3	3	C	22.7	7	С
Intersection De	lay, s/ve	eh/LOS				38	3.4						D		
Land Street							-				- 116		,	- 1	
Multimodal Re	-				EB			WB			NB	_		SB	
Pedestrian LOS	Score	/LOS		3.4		C	3.5		D	2.9	$\overline{}$	C	2.6		В
r caestran Loc		os		1.3		Α	1.5		A	1.6		A	1.3		Α

	HCS 20	10 5	ignali	zea II	nerse	cuor	Res	suits S	umm	ary				
General Information								Intersec	tion Inf	ormatic	n	1	44-1	e.l.
Agency	T	_					\rightarrow	Duration,	_	0.25			11111	
Analyst	СМ		Analys	is Date	Mar 2	2 2016	\rightarrow	Area Typ	_	Other		18 .		
Jurisdiction	July	_	Time F	_	Iviai Z	-, 2010	-	PHF	_	0.98	_	- 15	-16	7
Urban Street	US 41 (SR 45)			sis Year	2016		\rightarrow	Analysis	Period	1> 4:0	00	1		
Intersection	Coconut Road		File Na		-	DM Dk I	-	-23-2016		132.000		79	****	
Project Description	2021 PM Pk Hr		I HE IVE	40)V	12021	MEKI	03	20-2010						200
Project Description	ZOZIFWIEKTI						-							
Demand Information	1	=		EB			VVE	3	1	NB	-		SB	
Approach Movement			L	Т	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h			263	129	103	426	12	3 69	126	1407	514	71	1361	151
												1	-	
Signal Information				5		11	1	U Tax	2	8.				5
Cycle, s 180.	Reference Phase	2		18	50		2	8	6	-	1	-	1	100
Offset, s 0				6.8	0.6	105.9	16.	3 2.8	15.2	2		-	2	
Uncoordinated No			Yellow	-	0.0	5.1	4.0		4.7	- 1		t	1	
Force Mode Fixed	Simult. Gap N/S	On	Red	2.5	2.5	2.0	2.0	2.0	2.5		18	1	= 1	Z
-				40									- ,-	
Timer Results			EBI		EBT	WB	L	WBT	NBI		NBT	SB		SBT
Assigned Phase			3		8	7	41	4	1	- 1	6	- 5	- 1	2
Case Number			2.0	-	3.0	2.0		3.0	2.0		3.0	2.0		3.0
Phase Duration, s			22.3	3	22.4	27.0		27.2	17.5	5	116.1	14.4	4	113.0
Change Period, (Y+	CHARLES WINE		6.0		7.2	2.0		7.2	2.5		7.1	7.6		7.1
Max Allow Headway			3.1		3.1	3.0	-	3.1	3.0		0.0	3.0	10	0.0
Queue Clearance Tin	ne (gs), s		15.8	3	14.5	24.4	1	13.6	14.9	9		5.7		.0
Green Extension Tim	e (g .), s		0.4		0.7	0.7		0.7	0.2		0.0	0.1	12	0.0
Phase Call Probabilit	у		1.00		1.00	1.00		1.00	1.00			0.97	7	
Max Out Probability		- 1	0.00)	0.00	0.03	3	0.00	0.00)		0.00)	
				-										
Movement Group R			-	EB	-	-	WB	1 -	-	NB	-	-	SB	-
Approach Movement			a L	T	R	L	Т	R	L	T	R	L	T	R
Assigned Movement	20 Y 40 A W		3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow Rate (The latest death of the latest death death of the latest death of the latest death of the latest death death of the latest death death of the latest death dea		268	132	105	435	126	70	129	1436	524	72	1389	154
	low Rate (s), veh/h/h	1	1723	1863	1610	1723	1863	-	1774	1691	1449	1723	1691	1579
Queue Service Time			13.8	12,5	11.5	22.4	11.6	-	12.9	28.0	40.3	3.7	27.9	8.0
Cycle Queue Clearar	ice Time (g =), s		13.8	12.5	11.5	22.4	11.6	-	12.9	28.0	40.3	3.7	27.9	8.0
Green Ratio (g/C)			0.09	0.08	0.08	0.14	0.11	0.11	0.08	0.61	0.61	0.04	0.59	0.59
Capacity (c), veh/h			311	157	136	479	207	175	148	3074	878	130	2986	929
Volume-to-Capacity F			0.862	de la contraction de la contra	0.773	0.907	0.607	-	0.868	0.467	0.598	0.556	0.465	0.166
	ft/In (95 th percentile)		2000	261.2	213.3	407.5	239.7	_	256.7	410.2	498	76.1	412.6	133,3
	veh/ln (95 th percentil		10.5	10.3	8.5	16.0	9.4	5.5	10.1	16.1	19.9	3.0	16.2	5.3
	(RQ) (95 th percent	le)	0.44	0.00	0.53	0.91	0.00	-	0.53	0.00	0.68	0.13	0.00	0.48
Uniform Delay (d 1),			80.8	81.2	80.7	76.3	76.3	_	81.5	19.5	21.9	85.1	21.0	16.9
Incremental Delay (5,1	4.4	3,5	13.0	1,1	0.6	5.8	0.5	3.0	1.4	0.5	0.4
Initial Queue Delay (0.0	0.0	0,0	0.0	0,0	0.0	0.0	0.0	0.0	0.0	0,0	0.0
Control Delay (d), s			85.8	85.6	84.2	89.3	77.3	_	87.3	20.0	24.9	86.5	21.5	17.3
Level of Service (LOS	·		F	F	F	F	E	E	F	С	С	F	С	В
Approach Delay, s/ve			85.4	1	F	85.3	3	F	25.4	1	C	24.0		С
Intersection Delay, s/	veh / LOS				38	9.0						D		
				- 1			100	_				7		
Multimodal Results			-	EB		-	WB	-	-	NB		-	SB	_
Pedestrian LOS Scor			3.4	$\overline{}$	0	3.5	_	D	2.9	$\overline{}$	C	2.6		В
Bicycle LOS Score /	OS		1.3		A	1.5		A	1.6		A	1.4	15/10-	A

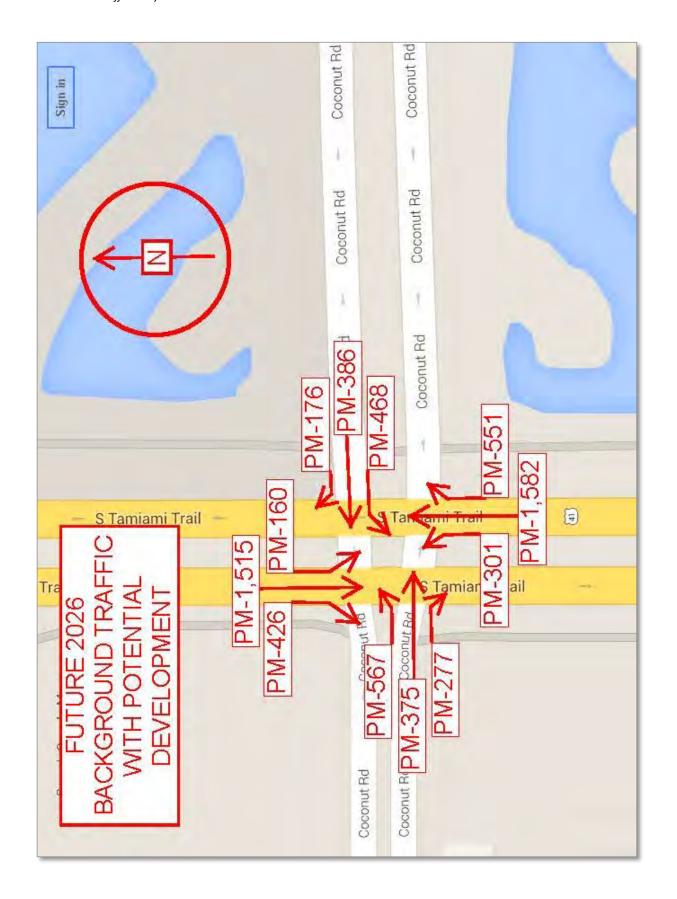
		HCS 2	010 S	ignali	zed II	nterse	ection	1 Res	suits S	umm	ary				
Canaral Inform	otion								Interces	tion luf	odi		4	7 d July 2 1	-1
General Inform	ation	_			_			-	Intersection.	-	-	on		11111	
Agency		CM	_	LAmaka	ia Data	IM 2	2 2046	-			0.25		- 0		
Analyst	_	LIVI	_	-	-	Mar 2	2, 2016	_	Area Typ PHF	e	Other		- 185	-Ti-	-
Jurisdiction	_	110 W (OD 40)	_	Time F		2040		\rightarrow		Destruction	0.98	20	- 11 -		- 2
Urban Street	_	US 41 (SR 45)		_	sis Year	-	D14 D1 1	_	Analysis		1> 4:	00	- 8		
Intersection	07.	Coconut Road		File Na	ame	2026	PIVI PK	Hr - U3	-23-2016	xus.	_		-1 - 9	1111	7
Project Descrip	tion	2026 PM Pk Hr		_	_	_	_	_	_	_	_		-	(laurae a)	
Demand Inform	nation		_		EB			W	2	T.	NB	-	1	SB	
Approach Move	1000			L	T	R	L	T	_	L	IT	R	L	T	R
Demand (v), v	_			263	129	103	448	-		132	4	_	75	1430	158
Demand (V), V	CIVII	_	-	200	120	100	440	10	0 12	102	1470	5 541	7.5	1400	100
Signal Informa	tion				1	T	11	1	- 1		8.		1	- 1	5
Cycle, s	180.0	Reference Phase	2	1	100	2.5		-2	6	7	-	\ \	_	1	-
Offset, s	0	Reference Point	End	1	0.0	112	403	120	0 00	3			4	- 1	-
Uncoordinated	No	Simult. Gap E/W	On	Green Yellow		0.0	104. ⁴	1 16.		15.2			**	-	
Force Mode	Fixed	Simult. Gap N/S	On	Red	2.5	2.5	2.0	2.0		2.5		-		1	V
		- Allen Sale (108)				dill'e									
Timer Results				EBI		EBT	WB	L	WBT	NBI		NBT	SBI		SBT
Assigned Phase	9			3		8	7	7-1	4	1		6	5		2
Case Number				2.0		3.0	2.0		3.0	2.0		3.0	2.0		3.0
Phase Duration	S			22.3	-	22.4	28.2	_	28.3	18.2	-	115.0	14.4		111.2
Change Period,	-	e1 s		6.0	\rightarrow	7.2	2.0	_	7.2	2.5	$\overline{}$	7.1	7.6		7.1
Max Allow Head	-	WATER WATER	$\overline{}$	3.1	_	3.1	3.0	_	3.1	3.0	_	0.0	3.0	_	0.0
Queue Clearan				15.8		14.5	25.5		14.2	15.5		0.0	5.9	_	0.0
Green Extensio			_	0.4	-	0.7	0.7	-	0.7	0.2	-	0.0	0.1	$\overline{}$	0.0
Phase Call Prol	-	19-7-5		1.00	_	1.00	1.00		1.00	1.00	_	0.0	0.98	_	
Max Out Probal	-		_	0.00	_	0.00	0.07	-	0.00	0.00	_		0.00	_	
Wax Out Froba	Dinty			0.00		0.00	0.01		0.00	0.00		_	0.00		
Movement Gro	up Re	sults			EB			WB	0.0		NB			SB	
Approach Move	ment			L	T	R	L	T	R	L	T	R	TL.	T	R
Assigned Move	ment			3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow F), veh/h		268	132	105	457	133	73	135	1508	552	77	1459	161
Control of the last of the las	_	ow Rate (s), veh/h/	n	1723	1863	1610	1723	1863	_	1774	1691	1449	1723	1691	1579
Queue Service				13.8	12.5	11.5	23.5	12.2	the local division in which the	13.5	30.5	44.4	3.9	30.6	8.6
		e Time (g =), s		13.8	12.5	11.5	23.5	12.2	_	13.5	30.5	44.4	3.9	30.6	8.6
Green Ratio (g.		A.W. T. J. T. T.		0.09	0.08	0.08	0.15	0.12	_	0.09	0.60	0.60	0.04	0.58	0.58
Capacity (c), v				311	157	136	501	219	_	154	3041	868	131	2936	913
Volume-to-Capa		atio (X)		0.862	_	0.773	0.912	0.607	-	-	0.496	0.636	0.584	0.497	_
	-	/In (95 th percentile)		266.3	and the local division in which the	213.3	428	249.6	-	266.2	_	544.4	80.5	447.8	144.4
		eh/ln (95 th percent	_	10.5	10.3	8.5	16.8	9.8	5.7	10.5	17.4	21.8	3.2	17.6	5.8
Name and Address of the Owner, where the Owner, which is the O		RQ) (95 th percent	-	0.44	0.00	0.53	0.95	0.00	-	0.55	0.00	0.74	0.14	0.00	0.52
Uniform Delay (100000000000000000000000000000000000000			80.8	81.2	80.7	75.8	75.5	-	81.2	-	23.4	85.2	22.4	17.8
Incremental De	_	MI TOTAL		5.1	4.4	3.5	14.5	1.0	0.5	5.8	0.6	3.5	1.5	0.6	0.4
Initial Queue De	and the latest designation of			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (85.8	85.6	84.2	90.3	76.5	-	87.0	21.2	26.9	86.7	23.0	18.2
Level of Service				F	F	F	90.5	70.5	E	F	C	C C	F	C .	B
Approach Delay	CARL SHAPE			85.4	_	F	85.8		F	26.6		C	25.5		C
Intersection De				03,1		_	0.9			2010			D 20,0		
	,,				3	30								-	
Multimodal Re	sults			1	EB			WB			NB			SB	
Pedestrian LOS	Score	/LOS		3.4		C	3.5	71	D	2.9		C	2.6		В
	ore / L	-12-		1.3		Α	1.6		Α	1.7		A	1.4		A

General Informatio	n						- 1	Intersec	tion Inf	ormatio	on		THE R.	
Agency	1						-	Duration	h	0.25				No.
Analyst	CM		Analys	sis Date	e Mar 2	5, 2016		Area Typ	e	Other		B-:		- 5
Jurisdiction			Time F	Period				PHF		0.98		8 -		Ē
Urban Street	US 41 (SR 45)		Analys	sis Year	2016	1 1		Analysis	-	1> 4:	_	18		. 1
Intersection	Coconut Road		File N	ame	2021	PM Pk I	Hr w D	evelopm	ents - 0	3-25-20	16.xus		1111	7
Project Description	2021 PM Pk Hr w [)evelop	ments		-							1	Netters.	86
							-		-					
Demand Informatio	**			EB	-	-	WE	_		NB	-		SB	-
Approach Movemen		_	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h		_	503	339	238	446	308	173	240	1511	524	156	1446	296
Signal Information		=			_	121	_	7	7	2	-		Y	
Cycle, s 180	0 Reference Phase	2	1	71	1 22	24	10	12	12	₽,	(1	-
Offset, s 0	Reference Point	End		1	1	1		3	1		11	- 1	1	
Uncoordinated No		On	Green		3.7	86.9	26.	-	22.8	3		200	_	
	d Simult, Gap N/S	On	Yellow	2.5	2.5	5.1	2.0	0.0	2.5		*	Y		V
Torce wode Fixe	a Joinfuit, Gap IV/S	Oll	Neu	12.0	14.0	12.0	12.0	10.0	12.0			-		100
Timer Results			EBI	900	EBT	WB		WBT	NBI		NBT	SB		SBT
Assigned Phase			3		8	7		4	1		6	5		2
Case Number			2.0		3.0	2.0		3.0	2.0		3.0	2.0		3.0
Phase Duration, s			32.0	-	33.9	28.	_	30.0	24.0	-	100.2	17.8	_	94.0
Change Period, (Y+	Ra) s		6.0	\rightarrow	7.2	2.0		7.2	2.5	\rightarrow	7.1	7.6	-	7.1
Max Allow Headway			3.1	_	3.1	3.0	_	3.1	3.0		0.0	3.0	-	0.0
Queue Clearance Ti			28.0		28.7	25.4	_	24.8	23.5	-	0.0	10.5	_	0.0
Green Extension Tin			0.0	-	0.0	0.7	-	0.0	0.0	-	0.0	0.0	\rightarrow	0.0
Phase Call Probabili			1.00	_	1.00	1.00		1.00	1.00	_	0.0	1.00	_	0,0
Max Out Probability	17	_	1.00	_	1.00	0.0	-	1.00	1.00	_		1.00	_	
Wax Out Frobability		_	1.00		1,00	0.0	-	1.00	1.00		_	1.00		_
Movement Group F	tesults			EB			WB	0 - 0		NB			SB	
Approach Movemen			a L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement			3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow Rate			513	346	243	455	314	177	245	1542	535	159	1476	302
	Flow Rate (s), veh/h/	n	1723	1863	1610	1723	1863	-	1774	1691	1449	1723	1691	1579
Queue Service Time			26.0	26.7	26,7	23.4	22.8	19.8	21.5	37.9	50.8	8.2	38.2	22.0
Cycle Queue Cleara			26.0	26.7	26.7	23.4	22.8	19.8	21.5	37.9	50.8	8.2	38.2	22.0
Green Ratio (g/C)			0.14	0.15	0.15	0.14	0.13	0.13	0.12	0.52	0.52	0.06	0.48	0.48
Capacity (c), veh/h			498	276	239	499	236	200	212	2624	749	196	2450	762
Volume-to-Capacity	Ratio (X)		1.031	_	1.016	0.912	-	-	_	0.588	0.714	0.813	0.602	0.396
The second secon	ft/ln (95 th percentile		545	881.7	-	426.1	_	-	615	550.6	637.1	190.8	560.1	-
	veh/ln (95 th percent	-	21.5	34.7	21.7	16.8	33.8	A CONTRACTOR OF THE PARTY OF TH	24.2	21.7	25.5	7.5	22.1	13.4
Name and Address of the Owner, where the Owner, which is the Owne	(RQ) (95 th percen	1200	0.90	_	-	0.95	The Person Name of Street, or other Designation of the Person of the Per	and the local division in the local division	1.27	-	0.87	0.32	0.00	_
Uniform Delay (d +)			77.0	100000	76.6	75.8	78.6	-	79.3	30.2	33.3	83.9	33.9	29.8
Incremental Delay (48.6	200000000000000000000000000000000000000	_	14.4	-	-	110.2	1.0	5.7	20.4	1.1	1.5
Initial Queue Delay (0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s			-	_	139.1	90.2	-	-	189.4	31.1	39.0	104.3	35.1	31.3
Level of Service (LO			F	F	F.	F	E	F	F	С	D	F	D	C
Approach Delay, s/v	The state of the s		156.	_	F	148.	_	F	49.6		D	40.3		D
Intersection Delay, s).3						F		
	144													
THE RESERVE OF THE PERSON NAMED IN	t			EB			WB			NB			SB	
Multimodal Results			3.4	_	C	3.5	_	D	2.9	_	C	2.6	_	В
Multimodal Results Pedestrian LOS Sco	re / LOS													

No.	HCS 20	. 10 0	gnan	LUU II	110130	-6101	. 1,65	uno o	SALULUI)	ur y				
General Information								ntersec	tion Infe	ormatic	on	1 1	Auto 1	
Agency							_	Duration		0.25			11111	W.
	СМ		Analys	is Date	Mar 2	5, 2016	\rightarrow	Area Typ		Other		- B		
Jurisdiction			Time F	-		.,	_	PHF		0.98		- M -:	-10-	- 5
	US 41 (SR 45)		-	is Year	2016		_	Analysis	Period	1> 4:0	מכ	- N		-
	Coconut Road		File Na		-	PM Pk I	_	evelopm		-		-	****	, –
The state of the s	2026 PM Pk Hr w D	evelop	-		12020						Joines	1	IIIII	20
roject becompaint	EGEO! MITKIN W.D.	Стегор	TICITES				- 44						-20	
Demand Information				EB			WE	3	1	NB	-		SB	
Approach Movement			L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h			567	375	277	468	386	176	301	1582	551	160	1515	426
										-	7	1		
Signal Information			1	5		1	7	2	11/6	S			14.	4
	Reference Phase	2		5	50	1	2	F	13		1		1	
	Reference Point	End	Green	10.4	3.5	86.9	27.2	2 2.8	22.8					
Name and Address of the Owner, where the Owner, which is the Owner, where the Owner, which is the Owner, whic	Simult. Gap E/W	On	Yellow	5.1	0.0	5.1	0.0	0.0	4.7			t	1	100
Force Mode Fixed	Simult. Gap N/S	On	Red	2.5	2.5	2.0	2.0	0.0	2.5		18	1	7	Y
				-					-			-	-	
Timer Results			EBI		EBT	WB	L	WBT	NBI		NBT	SB		SBT
Assigned Phase			3		8	7	71	4	1		6	5		2
Case Number			2.0	-	3.0	2.0	_	3.0	2.0	-	3.0	2.0	-	3.0
Phase Duration, s			32.0	\rightarrow	32.8	29.2		30.0	24.0	\rightarrow	100.0	18.0	-	94.0
Change Period, (Y+R a	To be the		6.0		7.2	2.0	_	7.2	2.5		7.1	7.6	_	7.1
Max Allow Headway (M			3.1		3,1	3.0	_	3.1	3.0		0.0	3.0	\rightarrow	0.0
Queue Clearance Time			28.0	-	27.6	26.6	-	24.8	23.5	-		10.4	\rightarrow	-
Green Extension Time (g .), s		0.0	_	0.0	0.6	_	0.0	0.0	_	0.0	0.0	_	0.0
Phase Call Probability			1.00	_	1.00	1.00	_	1.00	1.00	_		1.00	_	
Max Out Probability		- 1	1.00		1.00	0.17	1	1.00	1.00	(+)		1.00)	
Movement Group Bass	ılte		-	EB			WB		-	NB			SB	
Approach Movement	una		L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement			3	8	18	7	4	14		6	16	5	2	12
Assigned Movement	woh/h		_	383	283	478	394	-	307	1614	562	163	1546	435
Adjusted Flow Rate (v)			579 1723	1863	1610	1723	1863	180	1774	1691	1449	1723	1691	1579
Adjusted Saturation Flor				-	-	24.6	22.8	20.2	-	40.6	55.2		40.8	35.4
Queue Service Time (g			26.0 26.0	25.6 25.6	25.6	24.6	22.8	20.2	21.5	40.6	55.2	8.4	40.8	35.4
Cycle Queue Clearance Green Ratio (g/C)	Time (gr), s		0.14	0.14	0.14	0.15	0.13	0.13	0.12	0.52	0.52	0.06	0.48	0.48
Capacity (c), veh/h			498	265	229	521	236	200	212	2619	748	199	2450	762
Volume-to-Capacity Rat	io (V)		1.163	1.445	1.235	2000000	1.669	100000000000000000000000000000000000000	1.449	0.616	0.752	0.820	0.631	0.570
Back of Queue (Q), ft/l			-	1094.	726.5	446.7	1244.	-	901	584.6	688.3	196	593.1	500.3
back of Queue (Q), TVI	in (95 tri percentile)		070.5	9	720.5	440./	8	303.6	901	304.0	000.3	190	393.1	500,3
Back of Queue (Q), ve	h/ln (95 th percentil	e)	26.6	43.1	29.1	17.6	49.0	15.4	35.5	23.0	27.5	7.7	23.4	20.0
Queue Storage Ratio (/			1.12	0.00	1.79	0.99	0.00	_	1.86	0.00	0.94	0.33	0.00	1.82
Uniform Delay (d +), s/			_	77.2	77.2	75.3	78.6	_	79.3	30.9	34.4	83.9	34.6	33.2
Incremental Delay (d 2)			93.4	220.2	137.3	15.9	319.2	-	226.7	1.1	6.9	21.8	1.2	3.1
Initial Queue Delay (d =			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d), s/ve				297.4	214.6	91.2	397.8	-	306,0	32.0	41.3	105.6	35.9	36.3
Level of Service (LOS)	_		F	F	F	F	F	F	F	С	D	F	D	D
Approach Delay, s/veh /	LOS		219.	_	F	209.	_	F	68.0	_	E	41.3	_	D
Intersection Delay, s/vel						8.5						F		
				EB			WB			NB			SB	
Multimodal Results			3.4		C	3.5		D	2.9		С	2.6	-	В
Multimodal Results Pedestrian LOS Score /	LOS		3.4		~	0.0		-	2.0	_				_







Appendix N: Intersection Alternative - HCS 2010 Analysis

(2 Sheets)

		HCS 2	0 10 5	ignali	zea I	nterse	cuor	Res	suits S	umm	ary				
General Inform	ation							T	Intersec	tion Inf	ormati	nn .	1	44-1	-V
Agency	duon		_					\rightarrow	Duration		0.25	VIII		11111	
Analyst		CM	_	Analie	sie Data	Mar 2	5 2016	-	Area Typ		Other		- 1		
Jurisdiction		I SIVI		Time F	W. O. S. Co., Lot, Lot, Lot, Lot, Lot, Lot, Lot, Lot	ivieti Z	0, 2010		PHF	~	0.98		- 15	36	7
Urban Street		US 41 (SR 45)		-	sis Year	2016		\rightarrow	Analysis	Derind	1> 4:	nn	- 1		1
Intersection				File N		-	DM Db I	_	evelopm	-	_		- 1		
Project Descript	ion	Coconut Road)avalar	-	-	-	-W PK	H W D	evelopm	ent- (m	proved	- 03-2			(
Fioject Descript	lion	2021 PM Pk Hr w E	revelopi	nents -	mprov	su									
Demand Inform	ation		_		EB			VVE	3	T	NB			SB	
Approach Move	777			L	T	R	L	T	I R	L	LT	I R	L	T	R
Demand (v), v	_			503	339	238	446	4	-	_	_	_	_	1446	296
_ 3118413 (¥); V	-1111		100	- 500	500	200	440	00	170	240	101	324	100	140	200
Signal Informa	tion				1-1	JUL	JU	1	5	5 8-			1		5
Cycle, s	165.0	Reference Phase	2	1	38	1		7	7	E		14		1	1
Offset, s	0	Reference Point	End	Green		0.9	68.5	24.	4 3.8	28.5	1	1	4	2	-
Uncoordinated	No	Simult. Gap E/W	Off	Yellow	-	0.0	5.1	0.0		4.0		L	ta	-	
Force Mode		Simult, Gap N/S	Off	Red	2.5	0.0	2.0	2.0		2.0	-	- W		T+	7
						Mile									
Timer Results				EBI		EBT	WB	L	WBT	NBI		NBT	SBI		SBT
Assigned Phase				3		8	7	74	4	1	-	6	5		2
Case Number				2.0		3.0	2.0		3.0	2.0		3.0	2.0		3.0
Phase Duration	s			34.5	-	45.5	26.4	-	37.4	16.6	$\overline{}$	75.6	17.4	_	76.5
Change Period,	-	e), s		7.2	\rightarrow	7.2	2.0		7.2	2.5	\rightarrow	7.1	7.6	-	7.1
Max Allow Head	-			3.1	\rightarrow	3.1	3.0		3.0	3.0	_	0.0	3.0	-	0.0
Queue Clearan				26.1	\rightarrow	30.9	23.4		29.4	13.5	-	36.7	9.5	_	-
Green Extensio				1.2	-	1.1	1.0	-	0.9	0.5		0.0	0.3	-	0.0
Phase Call Prob	-			1.00	_	1.00	1.00		1.00	1.00			1.00		
Max Out Probal				0.00		0.00	0.00	-	0.00	0.00	$\overline{}$		0.00	-	
		-		A COLUMN						سنما				1	
Movement Gro	up Re	sults			EB			WB	0		NB			SB	
Approach Move	ment			a Lin	T	R	L	T	R	L	Т	R	TE:	T	R
Assigned Move	ment			3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow F	Rate (v), veh/h		513	346	243	455	314	177	245	1542	535	159	1476	302
Adjusted Satura	tion Flo	ow Rate (s), veh/h/	n	1723	1863	1610	1723	1863	1579	1723	1691	1449	1723	1691	1579
Queue Service	Time (g s), s		24.1	28.9	20.0	21.4	27.4	15,7	11.5	42.1	42.1	7,5	39.2	16.2
Cycle Queue Cl	earanc	e Time (g =), s		24.1	28.9	20.0	21.4	27.4	15.7	11.5	42.1	42.1	7.5	39.2	16.2
Green Ratio (g.	/C)			0.17	0.23	0.32	0.15	0.18	0.24	0.09	0.42	0.56	0.06	0.42	0.59
Capacity (c), v	eh/h			570	432	511	510	341	383	294	2108	817	205	2134	925
Volume-to-Capa	acity Ra	atio (X)		0.900	0.800	0.475	0.892	0.921	0.461	0.834	0.732	0.655	0.775	0.691	0.326
Back of Queue	(Q), ft	/In (95 th percentile)		407.9	501.8	316.8	368.3	494.3	201.1	224.6	616.3	523,7	153.1	578	120
Back of Queue	(Q), v	eh/ln (95 th percent	ile)	16.1	19.8	12.7	14.5	19.5	8.0	8.8	24.3	20.9	6.0	22.8	4.8
Queue Storage	Ratio (RQ) (95 th percent	tile)	0.67	0.00	0.62	0.65	0.00	0.31	0.31	0.00	0.71	0.26	0.00	0.28
Uniform Delay (67.5	59.7	45.3	69.0	66.2	14.6	74.3	40.5	24.9	76.5	39.0	10.3
Incremental Del	lay (d.	z), s/veh		2.2	1.3	0.3	2.2	6.8	0.3	2.4	2.3	4.1	2.4	1.9	0.9
Initial Queue De	elay (d	3), s/veh		0.0	0.0	0,0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0,0	0.0
Control Delay (d), s/v	eh		69.7	61.1	45.5	71.2	73.0	14.9	76.7	42.8	29.0	78.9	40.9	11.2
Level of Service				E	E	D	Е	E	В	Е	D.	С	E	D	В
Approach Delay	, s/veh	/LOS	Î	61.6	3	E	61.3	3	E	43.2	2	D	39.4	4	D
Intersection Del						48	3.0						D		
COLUMN TO SERVICE	1	4400			3,-						-				
Multimodal Re	sults				EB			WB			NB			SB	
Pedestrian LOS	Score	/LOS		3.5		C	3.5	g il li	D	3.0		C	3.0		C
	-	os		2.3		В	2.0		В	1.8		A	1.6		A

	HCS 2	010 S	ignali	zed l	nterse	ection	Res	sults S	umm	ary				
S				300										
General Informatio	n						-	Intersec	-	-	on		TO ST	
Agency					_		\rightarrow	Duration		0.25		1		
Analyst	CM		-		Mar 2	5, 2016		Area Typ	e	Other				25
Jurisdiction			Time F				-	PHF		0.98		- 18 = 1		2
Urban Street	US 41 (SR 45)		_	is Year	-		_	Analysis	_	1> 4:0		2		
Intersection	Coconut Road		File Na	and the latest designation of the latest des	-	PM Pk l	Hr w D	evelopm	ent - Im	proved	- 03-2		1111C	1
Project Description	2026 PM Pk Hrw D	evelopr	nents - I	mprove	d			_				- 7	NI STATE	87
Demand Information	in			EB			W	R	F	NB			SB	_
Approach Movemen		_	L	T	R	L	T	_	L	T	R	L	T	R
Demand (v), veh/h	,		567	375	277	468	-	_	301	1582	_	160	1515	426
Bernand (v), venin			001	010	Lit	400	00	0 170	001	1002	. 001	100	1010	420
Signal Information	Section 1985	5-34		100	J	11	Т	5	28		5 M 18			5
Cycle, s 165	0 Reference Phase	2		3.6	250		7	7		_	14	2	1	1
Offset, s 0	Reference Point	End	Green	10.0	2.0	56.2	25.	5 9.7	31.7	7	+	4	2	
Uncoordinated No	Simult. Gap E/W	Off	Yellow		0.0	5.1	0.0		4.0	- (_	t	1	
Force Mode Fixe	ed Simult. Gap N/S	Off	Red	2.5	0.0	2.0	2.0		2.0		(k)	6		Z
The second second				1		-								
Timer Results			EBI		EBT	WB	L	WBT	NBI		NBT	SBI	-	SBT
Assigned Phase			3		8	7		4	1		6	5		2
Case Number			2.0		3.0	2.0	_	3.0	2.0	\rightarrow	3.0	2.0	-	3.0
Phase Duration, s		_	37.7	\rightarrow	54.6	27.5	_	44.4	19.6	\rightarrow	65.3	17.6	-	63.3
Change Period, (Y	PERSONAL PROPERTY.		7.2		7.2	2.0	_	7.2	2.5		7.1	7.6	_	7.1
Max Allow Headway			3.1		3.1	3.0		3,0	3.0		0.0	3.0		0.0
Queue Clearance Ti			29.2	$\overline{}$	32.4	24.4	_	36.3	16.5	-		9.7	\rightarrow	0.6
Green Extension Tir			1.3	_	1.3	1.1	_	1.0	0.6	_	0.0	0.3	_	0.0
Phase Call Probabil	ty		1.00	_	1.00	1.00	-	1.00	1.00	_		1.00	_	
Max Out Probability			0.00	1	0.00	0.00)	0.00	0.00) -		0.00) -	
Movement Group F	Results			EB			WB			NB			SB	
Approach Movemen			L	T	R	L	T	R	L	T	R	TE.	T	R
Assigned Movement			3	8	18	7	4	14	1	6	16	5	2	12
Adjusted Flow Rate			579	383	283	478	394	-	307	1614	562	163	1546	435
	Flow Rate (s), veh/h/	ln .	1723	1863	1610	1723	1863	-	1723	1691	1449	1723	1691	1579
Queue Service Time			27.2	30.4	21.4	22.4	34.3	-	14.5	49.8	51.5	7.7	47.7	29.8
Cycle Queue Cleara	THE RESERVE TO THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN		27.2	30.4	21.4	22.4	34.3	-	14.5	49.8	51.5	7.7	47.7	29.8
Green Ratio (g/C)	ne inne (gr), e		0.18	0.29	0.39	0.15	0.23	_	0.10	0.35	0.51	0.06	0.34	0.53
Capacity (c), veh/h			636	535	629	533	420		357	1790	735	209	1728	829
Volume-to-Capacity	Ratio (X)		0.910		0.449	0.896	-	-	0.859	100000	0.765	0.781	0.895	0.52
	ft/ln (95 th percentile)	463.2	-	330.7	382.8	626.3	-	268.1	746.3	646.9	156.9	719.2	197.
	veh/ln (95 th percent		18.2	20.5	13.2	15.1	24.7	_	10.6	29.4	25.9	6.2	28.3	7.9
NAME AND ADDRESS OF TAXABLE PARTY.	o (RQ) (95 th percen	and the last of	0.77	0.00	0.64	0.67	0.00	-	0.36	0.00	0.88	0.26	0.00	0.46
Uniform Delay (d +)			65.9	52.8	37.1	68.4	62.7	-	72.8	50.7	32.7	76.4	51.6	11.4
Incremental Delay (6.9	1.4	0.2	2.2	15.5	_	2.4	7.9	7.4	2.4	7.6	2.4
Initial Queue Delay			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Control Delay (d),			72.9	54.1	37.3	70.6	78.2	_	75.1	58.6	40.1	78.8	59.2	13.8
Level of Service (LC			E	D	D	E	E	В	E	E	D	E	E	В
Approach Delay, s/v	The same of the sa		59.0	_	E	63.9		E	56.4	_	E	51.5		D
Intersection Delay, s					_	3.5						E		
Multimodal Results				EB			WB			NB			SB	
Pedestrian LOS Sco	re / LOS		3.5		C	3.5	71.	С	3.0		С	3.0		С
	LOS		2.5		В	2.2	2011	В	1.9	7-1	A	1.7		Α

Appendix O: Crash Data - West of US 41

(1 Sheet)

EventCrash Date	EventCrash EventCrash Date	EventOnStreet	EventCrossStreet	EventCrash Node	Event	EventCltyCode	EventAddress	Event RoadwayID	EventMP	Event RoadwaylD EventMP EventDD X	EventDD Y	EventDD Y EventNodeDescription
11/13/2015		14.17.00 NORTH COMMONS DR	COCONUTRD	12.2041	E	BONITA SPRINGS	No Data	NoData	0	-81.81602706	26.39797636	-81.81602706 26.39797638 COCONUT RD @ NORTH COMMONS DR
10/22/2015		23:00:00 ELDORADO BLVD	COCONUT RD	12,2039	到	BONITA SPRINGS	No Data	NoData	0	-81.82856346	26.39798352	-81.82856346 26.39798352 COCONUT RD @ EL DORADO BLVD
9/19/2015	4	7.42:00 COCCONUT ROAD	VIA VENETO AT THE COLONY ENTRANCE	12 2035	H	BONITASPRINGS	No Data	No Data	0	-81.8327298	26.3979703	26.3979703 GOCONUT RD @ VIA VENETO BLVD
5/17/2015		8:10:00 EL DORADO BLVD	COCONUTRD	12_2039	387	BONITA SPRINGS	No Data	No Data	0	-81.82856346	26.39798352	-81.82&56346 26.39798352 COCONUT RD @ EL DORADO BLVD
2/7/2015		2:45:00 CGCGNUT RD	SPRING CREEK DR	12,2008	H	BONITASPRINGS	No Date	No Data	0	-81.82616265	26.39797925	-81.82616265 26.39797925 COCONUT RD @ SPRING CREEK RD
10/8/2014	00:00:62	COCONUT RD	EL DORADO BLVDVD	12 2039	EE	BONITASPRINGS	No Data	NoData	0	-81.82856346	26.39798352	-81.82856346 26.39738352 COCONUT RD @ EL DORADO BLVD
7722/2014		900.00 COCCONUT RD	SAND FLY CT	12_2035	931	FORT MYERS	5001 COCONUT RD	No Data	0	-81 8627298		26.3979703 COCCONUT RD @ VIA VENETØ BLVD
11.9/2013		2:00:00 ELDORADO BLYDVD	COCCONUT RD	12 2039	LEE	FORT MYERS	No Date	No Data	0	-81.82856346	26.39798352	-81.82866346 26.39738352 COCONUT RD @ EL DORADO BLVD
211212013		6:55:00 COCONUT RD	OLD MEADOWBROOK CIR	12,2037	H	BONITASPRINGS	No Date	No Data	0	-81.82355981	26.39797461	26.39797461 COCONUT RD @ OLDE MEADOWBROOK BLVD
8/15/2012		18:19:00 COCONUT RD	NORTH COMMONS DR.	12,2041	337	UNINCORPORATED	No Data	NoData	O	-81,81599		26.39789 COCONUT RD @ NORTH COMMONS DR
12/31/2011	1701	COCCONUT RD	OLDE MEADOWBROOK CIRCLE	DATA ENTRY LEE	331	No Data	No Data	No Data	0	-81,823605	26,39795335	-81 82:305 26:39795333 COCONUT RD @ OLDE MEADOWBROOK CIR

Appendix P: Crash Data - East of US 41

(1 Sheet)

EventID Da	Event Crash Event Crash Date Time	Event Crash Time	EventOnStreet	EventCrossStreet	Event Crash Event Node Count	>	EventCityCode	EveniAddress	Event RoadwaylD	Event MP E	EventDD X	EventDD Y	EventNodeDescription
86103219	12/30/2015	15:34:00	COCONUTRD	IMPERIAL PKWY	12 2023 LEE	Ī	UNINCORPORATED	No Data	No Data	0	-81.78456793	26.39613154	THREE GAKS PKWY @ COCONUT RD.
86102652	12/8/2015	15:47:00	COCONUT RD	HEALTH CENTER BLVD	12_2045 LEE		UNINCORPORATED	No Data	No Data	0	-81.80814806	26,39810394	COCONUT RD @ WAVILLAGIO
86102546	12/3/2015	13:54:00	13:54:00 COCONUT RD	VIA VILLAGIO PKWY	-		UNINCORPORATED	No Data	No Data	0		26,39810394	COCONUT RD @ VIA VILLAGIO
86102441	11/25/2015		9:22:00 COCONUT RD	IMPERIAL PKWY	12_2023_LEE		FORT MYERS	No Data	NoData	0	81.78456793	26,39613154	THREE OAKS PKWY @ COCONUT RD
86101905	11/9/2015	Ė	6:31:00 IMPERIAL BLVD	COCONUT DR	12_2023_LEE	П	FORT MYERS	No Data	No Data	0	:793	26.39613154	THREE DAKS PKWY @ COCONUT RD
85188983	10/31/2015	11,00:00	THREE OAKS PW	COCONUTRD	12_2023 LEF	E BC	BONITA SPRINGS	No Data	NoData	0	-81.78431702	26,39620972	THREE DAKS PKWY @ COCONUT RD
20	10/28/2015		7:40:00 IMPERIAL PKWY	COCONUT RD			BONITASPRINGS	No Data	NoData	0	-81.78456793	26,39613154	THREE OAKS PKWY @ COCONUT RD
86101368	10/19/2015		COCONUT RD	OAKWILDEBLYD	12_2053 LEE		FORT MYERS	No Data	NoData	0	.81.79877218	26.39876629	COCONUT RD @ OAKWILDE BLVD
36100537	9/14/2015	14:47:00	VIA COCONUT POINT	COCONUTRD	12 2054 LEE		UNINCORPORATED	No Data	NoData	0	-81.80574015	26.39874023	COCONUT RD @ WACOCONUT POINT
86100546	9/14/2015	19:24:00	IMPERIAL PKWY	COCONUT RD	12_2023 LEE	П	BONITASPRINGS	No Data	No Data	0	-81,78456793	26,39613154	THREE GAKS PKWY @ COCONUT RD
86100131	8/28/2015	16:30:00	16:30:00 GDCONUT RD	THREE DAKS PKWY	12_2023_LEE		BONITASPRINGS	No Data	NoData	0	-81,78456793	26,39613154	THREE DAKS PKWY @ COCONUT RD
08/66098	8/14/2015	Ę	7.50:00 THREE DAKS PKWY	COCONUT RD	12 2023 LEE	Ĩ	UNINCORPORATED	No Data	NoData	0	-81.78456793	26,39613154	THREE DAKS PKWY @ COCONUT RD
33659	7772015		0:45:00 COCONUT RD	THREE DAKS PKWY	12, 2023 LEE	Ī	UNINCORPORATED	No Data	No Data	0	-81.78456793	26.39613154	THREE DAKS PKWY @ COCONUT RD
5883099	6/12/2015	12:40:00	COCONUT RD	VIACOCONUT PT	12_2054 LEE		FORT MYERS	No Data	NoData	0	-81.80574015	26,39874023	COCONUT RD @ VIA COCONUT POINT
5882790	5/29/2015	20:44:00	20:44:00 GOCONUT RD	SPRING RUN BLVD	12_2019_LEE	Г	UNINCORPORATED	No Data	No Data	0	52	-	COCONUT RD @ SPRING RUNBLVD
35118465	5/28/2015	14:01:00	14:01:00 THREE DAKS PW	COCONUT RD	12_2023 LEE		BONITASPRINGS	No Data	NoData	0	-81.78450775	26.39582253	THREE DAKS PKWY @ COCONUT RD
5882711	5/26/2015	16:47:00	VIA COCONUT PT	COCONUTRD	12_2054_LEE		UNINCORPORATED	No Data	No Data	0	-81.80574015	26.39874023	COCONUT RD @ VIA COCONUT POINT
32715	5/26/2015		IMPERIAL PKWY	COCONUT RD	12_2023 LEE		UNINCORPORATED	No Data	No Data	0	-81.78456798	26,39613154	THREE DAKS PKWY @ COCONUT RD
32351	5/11/2015		10.45.00 THREE DAKS PKWY	COCONUTRD	12_2023 LEE	Г	UNINCORPORATED	No Data	NoData	0	-81,78456793	26,39613154	THREE DAKS PKWY @ COCONUT RD
76473	4/6/2015		COCONUT RD	THREE OAKS PKWY	12_2023_LEE	0	NINCORPORATED	10020 COCONUT RD	NoData	0	81.78456793	26,39613154	THREE DAKS PKWY @ COCONUT RD
35775612	3/10/2015	Ĺ	12:40:00 THREE GAKS PKWY	COCONUT RD	12_2023_LEE		FORT MYERS	No Data	NoData	0	-81.78456793	26.39613154	THREE GAKS PKWY @ COCONUT RD
5832	3/2/2015		SPRING RUN BLVD	COCONUT RD	12_2019 LEE		BONITASPRINGS	9520 SPRING RUN BLVD	No Data	0	-81.79493925	26,39571431	COCONUT RD @ SPRING RUN BLVD
85615128	1/20/2015		16:15:00 SANDY CREEK TERRACE	SPRINGRUN BLVD			BONITASPRINGS	23506 SANDY OREEK TERRACE	NoData	0	_		SPRING RUN BLVD @ SANDYCREEK TER
85614965	1/14/2015		17,29:00 THREE OAKS PKWY	COCONUTRD	12_2023 LEE	1	BONITASPRINGS	No Data	No Data	0	200	26,39613154	THREE DAKS PKWY @ COCONUT RD
85614893	1/12/2015	1	15:25:00 COCONUT RD	SPRING RUN BLVD	12_2019 LEE		UNINCORPORATED	No Data	No Data	0	-81.79493925	26.39571431	COCONUT RD @ SPRING RUN BLVD
85614112	12/15/2014	3	11:25:00 COCONUT DR	SPRING RUN BLVD	12_2019 LEE	П	BONITASPRINGS	No Data	No Data	0	-81,79493925	26,39571431	COCONUT RD @ SPRING RUN BLVD
10	11/12/2014		17.24.00 GOCONUT RD	SPRING RUN BLYDVD	_		BONITASPRINGS	No Data	No Data	0	_	26,39571431	COCONUT RD @ SPRING RUNBLVD
84996239	11/10/2014		4:01:00 CR 881	COCONUTRD	12_2023 LEE		BONITASPRINGS	No Data	NoData	0	81.78456793	26.39613154	THREE DAKS PKWY @ COCONUT RD
84996252	11/10/2014		16:15:00 VIA COCONUT PT	COCONUT RD	12_2054_LEE		FORT MYERS	No Data	NoData	o	.81.80574015	26.39874023	COCCINUT RD @ VIA COCCINUT POINT
84995889	10/26/2014		18:50:00 IMPERIAL PKWY	COCONUT RD	12_2023_LEE		BONITASPRINGS	No Data	NoData	0	-81,78456793	26,39613154	THREE OAKS PKWY @ COCONUT RD
	10/10/2014		0:08:00 COCONUT RD	MACGCONUT PT		8	BONITASPRINGS	No Data	No Data	0		26.39874023	COCONUT RD @ VIA COCONUT POINT
84794116	772/2014		12:14:00 THREE DAKS PKWY	COCONUTRD	12_2023 LEE	\sim	NINCORPORATED	No Data	No Data	0	-81.78456793	26,39613154	THREE GAKS PKWY @ COCONUT RD
84793592	6/3/2014		18:31:00 IMPERIAL PKWY	COCONUTRD	12_2023 LEE		BONITASPRINGS	No Data	No Data	0	-81.78456793	26.39613154	THREE CAKS PIXMY @ COCONUT RD
84152318	3/4/2014	15,58:00	15:58:00 IMPERIAL PKWY	COCONUT RD	12_2023 LEE		BONITASPRINGS	No Data	NoData	0	-81.78456793	26,39613154	THREE DAKS PKWY @ COCONUT RD
3482355	1/29/2014	13:50:00	13:50:00 COCNUT RD	=	12_2054 LEE	3	UNINCORPORATED	No Data	No Data	0	S	26,39874023	COCCINUT RD @ WA COCCINUT POINT
81585676	12/9/2013	Ţ	0:34:00 COCONUT RD	VIA COCONUT RD	12_2064 LEE	2	NINCORPORATED	No Data	NoData	0	\$1,80574015	26.39874023	COCONUT RD @ WA COCONUT POINT
81588597	10/3/2013		15:27:00 COCONUT RD	VIA COCONUT PT	12_2054_LEE		BONITA SPRINGS	No Data	NoData	0	81.80574015	26.39874023	COCONUT RD @ VIA COCONUT POINT
81586001	9/24/2013	19,24:00	19:24:00 IMPERIAL PKWY	COCONUT RD	12_2023 LEE		UNINCORPORATED	No Data	No Data	0	-81,78456793	26,39613154	THREE OAKS PKMY @ COCONUT RD
9506440	7722/2013	8/30/00	8:30:00 CIR 881	COCONUT RD	12_2023_LEE		FORT MYERS	No Data	NoData	0	-	26.39613154	THREE DAKS PKWY @ COCONUT RD
81593383	6/16/2013	15:45:00	15:45:00 COCONUT BLVD		12_2019 LEE		BONITASPRINGS	No Data	No Data	0	-	26,39571431	COCONUT RD @ SPRING RUN BLVD
81590289	5/16/2013	8:50:00	IE IMPERIAL PARKWAY IE THREE GAKS	COCONUTROAD	12_2023 LEE		UNINCORPORATED	No Data	No Data	0	-81.78456793	26.39613154	THREE CAKS PKWY @ COCONUT RD
9499169	4/28/2013	17:40:00	THRE DAKS PKWY	COCONUTRD	12_2023 LEE		UNINCORPORATED	No Data	No Data	0	-81.78456793	26,39613154	THREE OAKS PKWY @ COCONUT RD
81589663	3/18/2013		14:58:00 GOCONUT RD	HEALTH CENTER BLYDVD	12_2045 LEE	Ī	BONITASPRINGS	No Data	NoData	0	-	26,39810394	COCONUT RD @ WAVILLAGIO
83276176	3/14/2013	16,50,00	COCONUT RD	THREE OAKS PKWY	12_2023 LEE		NINCORPORATED	No Data	NoData	0	-81.78456793	26.39613154	THREE OAKS PKWY @ COCONUT RD
81591774	3/12/2013	8.11.00	IMPERIAL PKWY	COCONUT RD	12_2023_LEE		BONITASPRINGS	No Data	No Data	0	-81.78456793	26.39613154	THREE DAKS PKWY @ COCONUT RD
9602305	2/28/2013	18:04:00	18:04:00 8200 HEALTH CENTER BLVDVD		12_2045 LEE		FORT MYERS	8200 HEALTH CENTER BLVD	NoData	0	-81,80814806	26,39810394	COCONUT RD @ VIA VILLAGIO
81590065	2/24/2013		14:08:00 COCONUT RD				UNINCORPORATED	No Data	NoData	0	_		COCONUT RD @ VIA VILLAGIO
81591908	2/13/2013		19:17:00 COCONUT RD	HEALTH CENTER BLVDVD	12_2045 LEE		BONITASPRINGS	No Data	No Data	0	\$1,80814806	26,39810394	COCONUT RD @ VIA VILLAGIO
81586566	1/16/2013	18:19:00	18:19:00 COCGNUT RD	₩.	12_2054_LEE		UNINCORPORATED	No Data	No Data	0	0 -81.80574015	26:39874023 COCONUT RD @ MACDCONUT POINT	COCONUT RD @ VIA COCONUT POINT
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