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FDOT STANDARD PLANS FOR BRIDGE CONSTRUCTION

- 415-001 BAR BENDING DETAILS (STEEL)
- 455-001 SQUARE PRESTRESSED CONCRETE PILES TYPICAL DETAILS & NOTES
- 455-002 SQUARE PRESTRESSED CONCRETE PILE SPLICES
- 455-003 SQUARE PRESTRESSED CONCRETE PILES EDC INSTRUMENTATION
- 455-018 18" SQUARE PRESTRESSED CONCRETE PILE

STANDARD ABBREVIATIONS

APPROX.	APPROXIMATE	LT.	LEFT
ASTM	AMERICAN SOCIETY FOR	MIN.	MINIMUM
	TESTING MATERIALS	N/A	NOT APPLICABLE
BSW	BACK OF SIDEWALK	NTS	NOT TO SCALE
BOT.	ВОТТОМ	P.C.	POINT OF CURVATURE
CIP	CAST IN PLACE	P.I.	POINT OF INTERSECTION
CL.	CLEARANCE	PREST.	PRESTRESSED
Ç	CENTERLINE	PROP.	PROPOSED
ĒONC.	CONCRETE	P.T.	POINT OF TANGENCY OR
СО.	COVER		PRESSURE TREATED
CF	CUBIC FEET	REINF.	REINFORCING
CFS	CUBIC FEET PER SECOND	RT.	RIGHT
СМИ	CONCRETE MASONRY UNIT	R/W	RIGHT OF WAY
СҮ	CUBIC YARDS	S.F.	SQUARE FOOT
DHW	DESIGN HIGH WATER	SPA.	SPACE OR SPACES
DIA.	DIAMETER		OR SPACED
DIM.	DIMENSION	SP	SPAN
EA.	EACH	<i>S.S.</i>	STAINLESS STEEL
EL.	ELEVATION	S.Y.	SQUARE YARD
EXIST.	EXISTING	STA.	STATION
EXP.	EXPANSION	TYP.	TYPICAL
FSW	FRONT OF SIDEWALK	UNO	UNLESS NOTED OTHERWISE
INV.	INVERT	VERT.	VERTICAL
INT.	INTERMEDIATE		

GENERAL NOTES

- A. DESIGN SPECIFICATIONS:
 - 1. FDOT STRUCTURES MANUAL DATED JANUARY 2023. 2. AMERICAN ASSOCIATION OF STATE HIGHWAY OF TRANSPORTATION
 - OFFICIALS (AASHTO) LOAD AND RESISTANCE FACTOR (LRFD) BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION.
 - 3 AASHTO LRFD DESIGN SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES 2009 WITH 2015 INTERIMS.
 - 4 FDOT DESIGN MANUAL DATED JANUARY 2023 AND SUBSEQUENT ROADWAY DESIGN BULLETIN.
- B. GOVERNING STANDARDS AND CONSTRUCTION SPECIFICATIONS: FLORIDA DEPARTMENT OF TRANSPORTATION, FY 2023-24 STANDARD PLANS AND JULY 2023-24 STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AS AMENDED BY CONTRACT DOCUMENTS.
- C. VERTICAL DATUM:
 - 1. ALL ELEVATIONS ARE IN FEET AND BASED ON NATIONAL GEODETIC VERTICAL DATUM OF 1929 (NGVD29)
- D. ENVIRONMENT:
 - 1. SUPERSTRUCTURE, SLIGHTLY AGGRESSIVE
 - 2. SUBSTRUCTURE, MODERATELY AGGRESSIVE
- E. CONTROLLING CRITERIA: 1. FRESH WATER
- F. DESIGN METHODOLOGY: 1. LRFD METHOD USING STRENGTH AND SERVICE LIMIT STATES.
- G. DESIGN LOADINGS:
 - LIVE LOADS: PEDESTRIAN LIVE LOAD (90 PSF). 1.
 - LIVE LOADS: NO VEHICLE LOAD HAS BEEN INCLUDED IN THE DESIGN. 2
 - LIVE LOADS: RAIL AND POST LIVE LOAD (200 LB. (VERTICAL OR HORIZONTAL) 3
 - PLUS 50 PLF (VERTICAL AND HORIZONTAL, ACTING SIMULTANEOUSLY)).
 - 4 DEAD LOADS:
 - REINFORCED CONCRETE (150 PCF) a.
 - b. 2" NO. 9 GAGE FENCE (2.35 LB/FT)
 - 5. DEAD LOADS: PREFABRICATED STEEL BRIDGE (83,200 LB).
 - WIND LOADS: END BENT DESIGN WIND LOADS ARE IN ACCORDANCE WITH AASHTO, 6. SECTION 3.8.31, AND WITH STRUCTURES DESIGN GUIDELINES SECTION 2.4.
- H. CONSTRUCTION LOADING:
 - 1. IT IS THE CONSTRUCTION CONTRACTOR'S RESPONSIBILITY TO PROVIDE FOR SUPPORTING CONSTRUCTION LOADS.

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			NOT FOR CONSTRUCTION	800 WATERFORD WAY, SUITE 700	CLIENT	PROJECT #	Village of
			NOT FOR CONSTRUCTION	MIAMI, FLORIDA 33126 FBPE CERTIFICATE OF AUTHORIZATION NO. 24 RUDOLF P.G. PEIN, P.E. #56805	VILLAGE OF ESTERO	CN 2022-02	ESTER
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GENERAL NOTES PEDESTRIAN BRIDGE (1 OF 2)

SHEET NO. B-1

GENERAL NOTES (CONT.)

- I. MATERIALS:
 - 1. CONCRETE:
 - a. SUBSTRUCTURE CLASS IV 5500 PSI, MINIMUM 28 DAY COMPRESSIVE STRENGTH.
 - CONCRETE SHALL BE IN ACCORDANCE WITH SPECIFICATION SECTION 346. b.
 - PROVIDE $\frac{3}{4}$ " CHAMFERS ON ALL EXPOSED EDGES, UNLESS С.
 - OTHERWISE NOTED.
 - CONCRETE COVER SHOWN IN PLANS DOES NOT INCLUDE PLACEMENT d. AND FABRICATION TOLERANCES UNLESS SHOWN AS "MINIMUM COVER". SEE SPECIFICATIONS FOR ALLOWABLE TOLERANCES.
 - 2. ANCHOR BOLTS, NUTS AND WASHERS:
 - ANCHOR BOLTS: ASTM F1554 GRADE 105.
 - NUTS: ASTM A563 GRADE A HEAVY HEX (5 PER ANCHOR BOLT). b
 - PLATE WASHERS: ASTM A36 (2 PER BOLT). C.
 - 3. REINFORCING STEEL: REINFORCING STEEL SHALL BE ASTM A615, GRADE 60, UNLESS NOTED OTHERWISE.
 - ALL DIMENSIONS PERTAINING TO LOCATIONS OF REINFORCING ARE TO CENTERLINE OF BARS EXCEPT WHERE THE CLEAR DIMENSION IS SHOWN TO FACE OF CONCRETE. 4. NEOPRENE PAD:
 - NEOPRENE PAD PER SPECIFICATION SECTION 932.
 - 5 PRESTRESSING STRANDS:
 - STRANDS FOR PRESTRESSED PILES SHALL MEET THE REQUIREMENTS DETAILED IN INDEX NOS. 455-001 AND 455-018.
 - 6. DIMENSIONAL LUMBER:
 - COMPOSITE DECKING MATERIAL SHALL BE IN ACCORDANCE WITH TECHNICAL PROVISIONS T952
 - a. MANUFACTURER: MOISTURESHIELD INC. OR EQUAL
 - SERIES: VISION b.
 - с. SANDSTONE
- J. UTILITIES:
 - THE UTILITIES SHOWN IN THE BRIDGE PLANS ARE AT APPROXIMATE LOCATIONS.
 - CONTRACTOR SHALL ENSURE THAT ANY EXISTING UTILITIES ARE NOT 2 ENDANGERED OR DISTURBED DURING CONSTRUCTION AND THAT ACTIVE UTILITIES WITHIN THE PROJECT LIMITS ARE PROPERLY MAINTAINED DURING CONSTRUCTION.
 - UTILITY LINE TO BE MOUNTED ALONG THE OUTSIDE OF THE ABUTMENT(S) З. AND THE UNDERSIDE OF THE PREFABRICATED STEEL BRIDGE. THE GENERAL LOCATION OF THE FORCE MAIN AND IRRIGATION LINE IS SHOWN ON THE "IRRIGATION PLAN" SHEETS IN THE LANDSCAPE PACKAGE. FINAL LOCATION OF UTILITIES IS SUBJECT TO CHANGE TO ALLOW THE CONTRACTOR ADJUSTMENT IN THE FIELD. UTILITY LINES AND CONNECTIONS SHALL ALLOW THE BRIDGE TO EXPAND AND CONTRACT DURING THERMAL CHANGES.
- K. PILES:
 - 1. PRESTRESSED CONCRETE PILES SHALL USE CLASS V 6500 PSI MINIMUM 28 DAY COMPRESSIVE STRENGTH.

- L. PLAN DIMENSIONS: ALL DIMENSIONS IN THESE PLANS ARE MEASURED IN FEET EITHER HORIZONTALLY OR VERTICALLY UNLESS OTHERWISE NOTED.
- M. EXISTING STRUCTURE: CONSTRUCTION OCCURS IN CLOSE PROXIMITY TO EXISTING STRUCTURES. THE CONTRACTOR IS TO TAKE ALL REASONABLE PRECAUTIONS TO PREVENT DAMAGE TO SUCH STRUCTURES IN ACCORDANCE WITH THE PROVISIONS OF SECTION 455 OF THE STANDARD SPECIFICATIONS.
- N. GEOTECHNICAL DATA: REFER TO GEOTECHNICAL ENGINEERING SERVICE REPORT FOR SOIL BORING LOCATION MAP AND SUBSURFACE PROFILES.
- PREFABRICATED STEEL BRIDGE: 0.
 - THE PEDESTRIAN PREFABRICATED STEEL BRIDGE SHOWN IS FOR REFERENCE ONLY AND WAS USED TO DESIGN THE FOUNDATIONS. THE CONTRACTOR SHALL PROVIDE A PREFABRICATED BRIDGE THAT MEETS OR EXCEEDS THE DESIGN SPECIFICATIONS AND DESIGN LOADINGS SHOWN ON THIS SHEET. THE CONTRACTOR SHALL SUBMIT LOAD RATING. SHOP DRAWINGS AND DESIGN CALCULATIONS TO THE ENGINEER FOR REVIEW. THE MANUFACTURER SHALL BE RESPONSIBLE FOR ALL MEMBERS, CONNECTIONS, ANCHOR BOLT LOCATIONS AND DETAILS WITHIN THE PREFABRICATED STEEL BRIDGE. FOUNDATION LOADS ASSUMED IN DESIGN ARE BASED ON THE CONCEPT SHOWN ON SHEET "PLAN AND ELEVATION PEDESTRIAN BRIDGE". ANY MODIFICATIONS TO THE SUBSTRUCTURE OR FOUNDATION DUE TO INCREASED PREFABRICATED STEEL BRIDGE LOADINGS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. ALTERNATIVE SUPERSTRUCTURE TYPE SHALL BE SUBMITTED AS SHOP DRAWINGS TO THE ENGINEER FOR CONSIDERATION

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DATE	DESCRIPTION	DATE	DESCRIPTION		PEDESTRIAN IMPR	OVEMENTS	
			NOT FOR CONSTRUCTION	800 WATERFORD WAY, SUITE 700	CLIENT	PROJECT #	
			NOT FOR CONSTRUCTION	MIAMI, FLORIDA 33126 EBPE CERTIFICATE OF AUTHORIZATION NO 24	VIU ACE OF ESTERO	CN 2022.02	ST
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GENERAL NOTES PEDESTRIAN BRIDGE (2 OF 2)

SHEET NO. B-2



THIS ITEM HAS BEEN DIGITALLY SIGNED AND SEALED BY:

ON THE DATE ADJACENT TO THE SEAL.

PRINTED COPIES OF THIS DOCUMENT ARE NOT CONSIDERED SIGNED AND SEALED AND THE SIGNATURE MUST BE VERIFIED ON ANY ELECTRONIC COPIES.

TIERRA 7351 TEMPLE TERRACE HIGHWAY TAMPA, FLORIDA 33637 THOMAS E. MUSGRAVE, JR., P.E. NO. 81669

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

SHEET DESCRIPTION

SIGNATORY SHEET REPORT OF CORE BORINGS

SIGNATORY SHEET

SHEET NO.

B-3



<u>L</u>	.E	GEND		
IT GRAY TO TAI	N SAN	ID TO SAND WITH SILT	(SP/SP-SM)	
IT GRAY TO BR	OWN	SILTY SAND (SM)		
IT GRAY TO TAI	N CLA	YEY SAND (SC)		
E GREEN SILT (I	ML/M	H)		
THERED LIMES	τονι	E/CAPROCK		
ITH CAPROCK	LIME/	STONE FRAGMENT		
UNDWATER LE	VEL		INVESTIGATION	
N-VALUE IN BL ETRATION (UNL	OWS/ _ESS	FOOT FOR 12 INCHES (OTHERWISE NOTED)	DF	
TED SOIL CLAS	SIFIC VISU S FOI	ATION SYSTEM (ASTM I JAL REVIEW AND LABO R CONFIRMATION OF VI	D 2488) GROUP SYMBOL RATORY TESTING ON SUAL REVIEW	
UNDWATER NO	ОТ АР	PARENT DUE TO DRILL	ING METHOD USED	
IBER OF BLOW	S FOF	4 INCHES OF PENETRA	TION	
D AUGERED TO) VER	IFY UTILITY CLEARANC	ES	
S OF CIRCULAT		OF DRILLING FLUID (100	J%)	
ING				
CENT PASSING URAL MOISTUR	8 #200 RE CO	SIEVE NTENT (%)		
JID LIMIT (%)				
STICITY INDEX ((%)			
PLASTIC				
ULAR MATERIA	LS-	SPT N-VALUE	SPT N-VALUE	
ATIVE DENSITY		(BLOWS/FT.)	(BLOWS/FT.)	
		4 to 10 10 to 30	3 to 8 8 to 24	
E DENSE		30 to 50 GREATER THAN 50	24 to 40 GREATER THAN 40	
S AND CLAYS NSISTENCY		SPT N-VALUE (BLOWS/FT.)	SPT N-VALUE (BLOWS/FT.)	
SOFT		LESS THAN 2	LESS THAN 1	
		4 to 8 8 to 15	3 to 6 6 to 12	
STIFF		15 to 30 GREATER THAN 30	12 to 24 GREATER THAN 24	
	= BC	RINGS		
			$\overline{\gamma}$	
			_	
OADWAY E	IM	PROVEMENTS	B-5	
OUNTY, FL	ORI	DA		ノ
				-
LA.	Ø	FPORT OF C	ORE ROPINICS	SHEET NO.
ERO	м	LICAL OF C	UMA DUMANUD	B-4





1. UP TO 6 FEET OF TOE MAY BE REQUIRED. QUANTITIES ARE BASED ON 5 FEET. THE CONTRACTOR TO DETERMINE THE DEPTH OF THE TOE IN THE CHANNEL DUE TO CONSTRICTIONS. ADDITIONAL ROCK MAY BE REQUIRED. REMOVAL OF EXISTING RIPRAP RUBBLE DITCH LINING, AS REQUIRED TO CONSTRUCT THE PROJECT, IS INCLUDED IN PAY ITEM 110-1-1. THE CONTRACTOR SHALL AVOID BLOCKING THE EXISTING AND PROPOSED PIPES INVERT ELEVATIONS DURING RIPRAP RUBBLE BANK AND SHORE INSTALLATION. FOR THE PROPOSED PIPE TO BE INSTALLED ALONG THE SOUTH BANK, PROTECT EXISTING WING WALL AND USE EXISTING OPENING FORMED BY REMOVED PIPE. LARGER PIPE TO BE ACCOMMODATED WITH MINOR CHIPPING OF GRINDING. ADD GROUT/COLLAR BETWEEN PIPE AND WALL TO RETAIN EARTH. BANK AND SHORE RIPRAP AROUND END BENT 2 SHOULD BE PLACED BEFORE PEDESTRIAN BRIDGE IS INSTALLED. RIPRAP RUBBLE BANK & SHORE MATERIALS, CONSTRUCTION AND INSTALLATION PER FDOT STANDARD SPECIFICATIONS SECTION 530. LEGEND: EXISTING RIPRAP TO BE REMOVED EXISTING CONCRETE SLOPE TO BE REMOVED AND REPLACED WITH SLOPE PAVEMENT DI PROPOSED RIPRAP RUBBLE BANK AND SHORE SHEET NO. SLOPE PROTECTION

B-6

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PLAN AND ELEVATION



TABLE	OF RIPR	AP STAKING	POINTS
POINT ID	ELEVATION (TOP OF RIPRAP)	X	Ŷ
1	4.75	720015.1251	763717.9833
2	4.75	720024.2884	763718.8771
3	10.00	720024.6432	763708.7181
4	10.00	720036.3025	763709.0113
5	1.00	720035.5353	763740.8001
6	1.00	720014.8754	763729.6473
7	1.00	720014.1905	763764.8611
8	1.00	720024.8605	763767.4584
9	1.00	720029.3996	763773.8798
10	1.00	720034.0756	763784.6500
11	1.00	720037.7745	763789.0618
12	1.00	720041.3539	763790.9553
13	10.00	720034.5666	763809.5628
14	10.00	720032.0614	763807.6762
15	10.00	720030.2267	763796.0193
16	10.00	720031.2265	763796.0400
17	7.50	720031.4546	763785.0424
18	7.50	720025.6992	763784.9017
19	4.00	720026.3012	763781.7217
20	3.75	720022.8832	763773.4587
21	3.75	720013.9514	763773.2106



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			SUMMARY OF STRUCTUR	ES QUANTITIES	- PEDESTRIAN E	BRIDGE				
CECTION			10047101		QUAN	NTITY	ТО	TAL		
SECTION	PAY ITEM NO.	DESCRIPTION	LOCATION	UNIT	Р	F	Р	F	DESIGN NOTES	CONSTRUCTION NOTES
	400 4 5		END BENT 1	CY.	9.1		9.1			
CURCTRUCTURE	400-4-5	CONCRETE CLASS IV, BRIDGE SUBSTRUCTURE	END BENT 2	C1	9.0		9.0			
SUBSTRUCTURE	A1E 1 E		END BENT 1	1 P	1133		1133			
	415-1-5	KEINFORCING STEEL - BRIDGE SUBSTRUCTURE	END BENT 2	LD	1246		1246			
FOUNDATIONS	455-34-3	PRESTRESSED CONCRETE PILING, 18" SQ	END BENTS 1 AND 2	LF	320.0		320.0			
SUPERSTRUCTURE	460- 7	PREFABRICATED STEEL TRUSS PEDESTRIAN BRIDGE	-	SF	962.0		962.0			
CONCRETE SLOPE PAVEMENT	524-2-29	CONCRETE SLOPE PAVEMENT, 4", REINFORCED	END BENT 1	SY	11.0		11.0			
	530-3-3	RIPRAP- RUBBLE, BANK AND SHORE	END BENTS 1 AND 2	TN	163.0		163.0			
SLOPE PROTECTION	530-74	BEDDING STONE	END BENTS 1 AND 2	ΤN	46.0		46.0			

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				800 WATERFORD WAY, SUITE 700	CLIENT	PROJECT #		SUMMARY OF		
			NOT FOR CONSTRUCTION	MIAMI, FLORIDA ['] 33126 FBPE CERTIFICATE OF AUTHORIZATION NO. 24 RUDOLF P.G. PEIN, P.E. #56805	VILLAGE OF ESTERO	CN 2022-02	STERO	STRUCTURES QUANTITIES	B-8	- UE
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						PILE DA	ATA TABLE						
		INSTALLATIC	N CRITERIA							DESIGN CRITER	RIA		
STATION	PILE SIZE (IN.)	NOMINAL BEARING RESISTENCE (KIPS)	MINIMUM TIP ELEVATION (FT.)	PILE ORDER LENGTH (FT.)	TEST PILE LENGTH (FT.)*	REQUIRED PREFORM ELEVATION (FT.)	FACTORED DESIGN LOADS (KIPS)	DOWN DRAG (TONS)	TOTAL SCOUR RESISTENCE (TONS)	NET SCOUR RESISTENCE (TONS)	100 YR. SCOUR ELEVATION (FT.)	LONG TERM SCOUR ELEVATION (FT.)	○ COMPRESSION
ALL	18	163	- 24 . 00	80	N/A	- 15.00	122	N/A	N/A	N/A	N/A	N/A	0.75

FACTORED DESIGN LOAD + NET SCOUR RESISTANCE + DOWN DRAG Ø

− ≤ NOMINAL BEARING RESISTANCE

TENSION RESISTANCE: THE ULTIMATE SIDE FRICTION CAPACITY THAT MUST BE OBTAINED BELOW THE 100 YEAR SCOUR ELEVATION TO RESIST PULLOUT OF THE PILE (SPECIFY ONLY WHEN DESIGN REQUIRES TENSION CAPACITY).

TOTAL SCOUR RESISTANCE: AN ESTIMATE OF THE ULTIMATE STATIC SIDE FRICTION RESISTANCE PROVIDED BY THE SCOURABLE SOIL.

NET SCOUR RESISTANCE: AN ESTIMATE OF THE ULTIMATE STATIC SIDE FRICTION RESISTANCE PROVIDED BY THE SOIL FROM THE REQUIRED PREFORMED OR JETTING ELEVATION TO THE SCOUR ELEVATION.

100 YEAR SCOUR ELEVATION: ESTIMATED ELEVATION OF SCOUR DUE TO THE 100 YEAR STORM EVENT.

LONG TERM SCOUR ELEVATION: ESTIMATED ELEVATION OF SCOUR USED IN DESIGN FOR EXTREME EVENT LOADING.

PILE NOTES:

A PILE INSTALLATION PLAN DETAILING THE PROPOSED PILE INSTALLATION METHODS SHALL BE PROVIDED TO THE ENGINEER FOR APPROVAL PRIOR TO THE INSTALLATION OF ANY PILES. THE PILE INSTALLATION PLAN SHOULD INCLUDE AT A MINIMUM: THE PILE HAMMER TYPE AND SPECIFICATIONS, AN ANALYSIS SHOWING THE PILE HAMMER IS CAPABLE OF PRODUCING REQUIRED DRIVING RESISTANCE, AND PILE INSTALLATION SEQUENCE.

PILE INSTALLATION NOTES:

- 1. MINIMUM PILE TIP ELEVATIONS ARE REQUIRED FOR LATERAL STABILITY.
- 2. NO JETTING WILL BE ALLOWED WITHOUT APPROVAL OF THE ENGINEER.
- 3. VIBRATION MONITORING PER FDOT STANDARD SPECIFICATION 455.
- 4. ALL PILES SHALL BE DYNAMICALLY MONITORED IN ACCORDANCE WITH FDOT STANDARD SPECIFICATIONS.
- PILE REBOUND POTENTIAL SHALL BE CONSIDERED DURING PILE HAMMER SELECTION. 5. ANTICIPATE SET-CHECKS AND/OR RE-DRIVES DURING PILE DRIVING OPERATIONS TO ACHIEVE THE REQUIRED NOMINAL BEARING RESISTANCE. ANTICIPATE SET-CHECKS AND/OR REDRIVES WHEN DEVELOPING THE PILE DRIVING SCHEDULE.
- DO NOT ADVANCE PREFORMED PILE HOLES DEEPER THAN THE PREFORM ELEVATIONS 6. SHOWN ON THE PILE DATA TABLES WITHOUT THE APPROVAL OF THE ENGINEER. IF ACTUAL PREFORMING ELEVATIONS DIFFER FROM THOSE SHOWN ON THE PILE DATA TABLES, THE ENGINEER SHALL DETERMINE THE REQUIRED DRIVING RESISTANCE.
- ANTICIPATE THE USE OF SPECIALIZED EQUIPMENT AND/OR METHODS INCLUDING, BUT NOT 7. LIMITED TO, CORE BARRELS, ROCK AUGERS, PUNCHES, DRILL BITS, ETC. TO COMPLETE PREFORMING. IF DRILLING EQUIPMENT WITH A TAPERED END IS USED TO CONSTRUCT THE PREFORMED PILE HOLES, THE MAXIMUM DIAMETER OF THE DRILLING EQUIPMENT MUST REACH THE REQUIRED PREFORM ELEVATION.
- 8. ALL PILES ARE 18" SQUARE PRESTRESSED CONCRETE PILES AND SHALL BE DRIVEN PLUMB.
- 9. VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO ANY PILE DRIVING.

PILE CUT-C	DFF ELEVA	TIONS
LOCATION	1	2
END BENT 1	8.53	8.53
END BENT 2	8.29	8.29

	REVI	SIONS			SANDY LANE B	ICYCLE/	
DATE	DESCRIPTION	DATE	DESCRIPTION		PEDESTRIAN IMPR	OVEMENTS	
				800 WATERFORD WAY, SUITE 700	CLIENT	PROJECT #	
			NOT FOR CONSTRUCTION	MIAMI, FLORIDA 33126 FBPE CERTIFICATE OF AUTHORIZATION NO. 24 RUDOLF P.G. PEIN, P.E. #56805	VILLAGE OF ESTERO	CN 2022-02	
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PILE DATA TABLE	SHEET NO.	
AND NOIES PEDESTRIAN BRIDGE	B1-4	
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ZE DES FT	$\Gamma H \mid N \cap$	TYPF	STY		В			C			Л			F		F			Н			.1			К		N	Φ
CATION END PENT 1	IN BARS	BAR	A G	FT	IN	FR	FT		FR	FT	IN	FR	FT		R FT	, 	FR	FT	IN	FR	FT	IN	FR	FT	IN	FR	 NO	ANG
ALIUN END DENT I	NO	REQUIF	RED = 1																									
6 E01 15	6 5	1		15	6																							
5 E02 11	4 1	18	1 1	10	0											_												
5 E03 4	6 36	1		4	6																							
4 E04 10	7 16	4	4 4	2	5		2	6								_												
4 E05 8	0 2	5		2	5		2	6			4			4														
4 E06 15	6 12	1		15	6																							
5 E07 11	6 10	10		10	6		1	0																				
5 E08 9	6 12	10		8	6		1	0																				
Vary	/ 2 Sets			5	6																							
5 EU9 5	7 of 8			5	7																							
6 E10 3	4 6	10		2	4		1	0																				
ATION END BENT 2	NO	. REQUI	RED = 1																									
5 E01 15	6 7	1		15	6																							
5 E02 10	1 3	18	1 1	8	9																							
5 E03 4	6 40	1		4	6																							
4 E04 10	7 18	4	4 4	2	5		2	6																				
4 E05 8	0 2	5		2	5		2	6			4			4														
E06 15	6 12	1		15	6																							
5 E07 11	6 10	10		10	6		1	0																				
5 E08 9	6 12	10		8	6		1	0																				
5 E09	2 3015	1		5	3											_												
5	5 of 8	10		5	7		1	0																				
EIU 3	4 0	10		2	4		1	0																				

REVISIONS				SANDY LANE BIO	-		
DATE	DESCRIPTION	DATE	DESCRIPTION	ΛΤΚΙΝΣ	PEDESTRIAN IMPRO	OVEMENTS	
			NOT TOD CONTEMPTOR	800 WATERFORD WAY, SUITE 700	CLIENT	PROJECT #	
			NOT FOR CONSTRUCTION	MIAMI, FLORIDA 33126 FBPE CERTIFICATE OF AUTHORIZATION NO. 24 RUDOLF P.G. PEIN, P.E. #56805	VILLAGE OF ESTERO	CN 2022-02	

SHEET

NO.

B1-7

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PRESTRESSED CONCRETE PILE NOTES:

- the Structures Plans.
- 2. Concrete:
 - (Index 455-031).
 - В. High Capacity Splice Collar: Class V. С.
- the use of Highly Reactive Pozzolans is required. 3. Concrete strength at time of prestress transfer:
- A. Piles: 4,000 psi minimum. Β.
- 4. Carbon-Steel Reinforcing:
 - Α. В.
 - С.
- 5. Spiral Ties:
- One full turn required for spiral splices. В.
- Compound or an Epoxy Mortar as recommended by the Manufacturer.



TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS								
	D = .	Square	e Pile	Size	(inches)	Required Storage and	Diek Un Deteil	
	12	14	18	24	30	Transportation Detail	PICK-OP Delan	
Maximum	48	52	59	68	87	2, 3, or 4 point	1 Point	
Pile Length	69	75	85	98	124	2, 3, or 4 point	2 Point	
(Feet)	99	107	121	140	178	3 or 4 point	3 Point	



Spiral Ties -W4.0 (30" Piles) W3.4 (All others)

DETAIL SHOWING TYPICAL COVER

DESCRIPTION: LAST REVISION 11/01/22



FY 2023-24 STANDARD PLANS

SQUARE PRESTRESSED CONC - TYPICAL DETAILS & N

1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in

A. Piles: Class V, except use Class VI for High Moment Capacity Pile

See "GENERAL NOTES" in the Structures Plans for locations where

High Moment Capacity Piles: 6,500 psi minimum.

Bars: Meet the requirements of Specification Section 415. Prestressing Strands: Meet the requirements of Specification Section 933. Protect all strands permanently exposed to the environment and not embedded under final conditions in accordance with Specification Section 450.

A. Tie each wrap of the spiral strand to a minimum of two corner strands. 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 962. Use an Epoxy Bonding

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- Mechanical Pile Splices on the Approved Products List (APL).
- holes; use either removable preforming material or stay-in-place corrugated galvanized steel ducts meeting ASTM Specification A653, Coating Designation G90, 26 gauge. Use 2" diameter ducts with a minimum corrugation (rib) height of 0.12 in. fabricated with either welded or interlocked seams. Galvanizing of welded seams is not required.
- development as approved by the Engineer.













** See Note 4 on Index 455-002



ALTERNATE STRAND PATTERNS

- 12 ~ 0.6" Ø, Grade 270 LRS, at 35 kips
- $12 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips
- 16 ~ $\frac{1}{2}$ " Ø, Grade 270 LRS, at 26 kips
- $20 \sim \frac{7}{16''}$ Ø, Grade 270 LRS, at 21 kips
- 24 ~ $\frac{3}{8}$ "Ø, Grade 270 LRS, at 17 kips

NOTES:

- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 – Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows: Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.

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FY 2023-24 STANDARD PLANS

18" SQUARE PRESTRESSED CONCRETE PILE



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